

## GENERAL NOTES

1.2 MATERIALS

1 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER STANDARD PRACTICES 2.6 THE BUYER/FIND LISE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

ASTM DESIGNATION MIN. YIELD STRENGTH

HOT ROLLED STEEL SHAPES (W, & C) HOT ROLLED STEEL ANGLES (L) STEEL PIPES STRUCTURAL TUBING STRUCTURAL STEEL WEB PLATE STRUCTURAL STEEL FLANGE PLATES/BARS COLD FORMED LIGHT GAGE ROOF & WALL SHEETS CABLE BRACE	A529/A572 A653/A1011 A792/A653	
ROD BRACE	A36	Fy = 36 KSI
		MIN. TENSILE STRENGTH
MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1"ø & LESS) HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)	A325-TYPE 1	

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF MPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.

(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).

NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE - 5/8" ø x 0'-1 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION)

C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS - 1/2"ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325—N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325—N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRÉ TENSIONING

.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS
OUTSIDE — BETWEEN ROOF PANELS & RIDGE CAP
— BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:

ALL BRACINE,

BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS

REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE

STRUCTURE, IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

## BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY COUNTY STATE OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS, M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHISTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/FRD USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE ONCE THE BUTER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

THE BUTERY AND USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COUNTINATION.
ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS
CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE
CONSIDERED AND CORONINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIAL
CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,

2.7 IT IS THE RESPONSIBILITY OF THE BLIYER/FND LISE CLISTOMER TO INSURE THAT M.B.S. PLANS IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH MB.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED. FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD DRAWINGS). PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE

UNIESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS M.B.S. DOES NOT DESIGN AND UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANIAL) SYSTEMS MANUAL)

2.10 NORMAL FRECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISEITS BY MODERATE J NORMAL ERECTION OPERATIONS INCLUDE THE CURRECTIONS OF MINOR MISTIS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BI REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD DRACTICE LATEST EDITION) PRACTICE LATEST EDITION)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS, (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even RNN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE

2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. WERREGRICY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME

2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

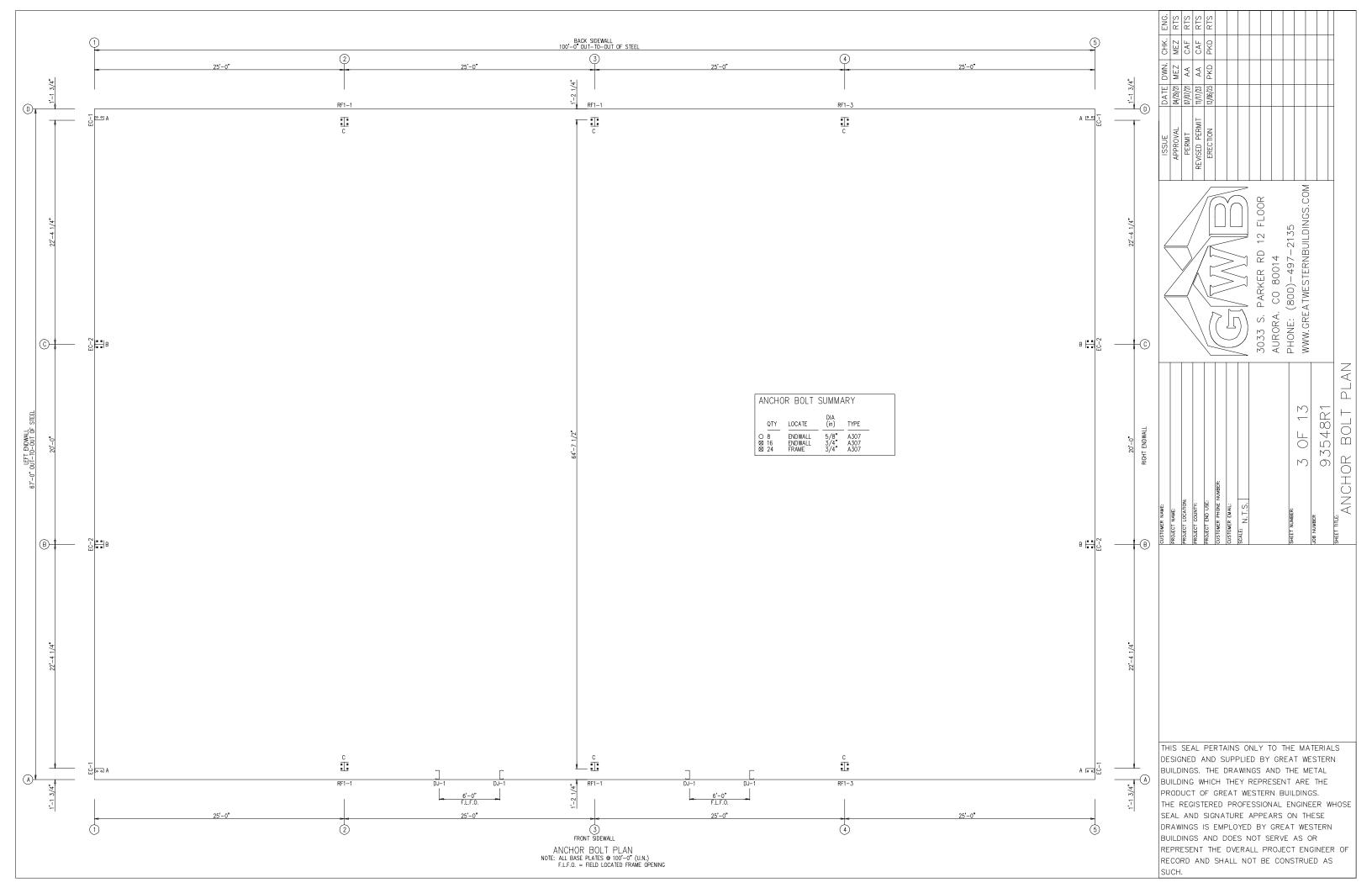
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

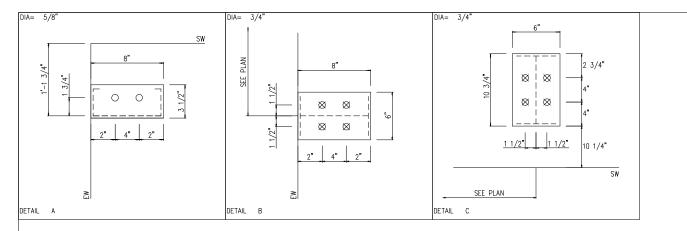
FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002

## BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: **DESIGN LOADS:** DESIGN CODE / WIND CODE : IBC-21 OCCUPANCY / RISK CATEGORY : II-Normal **ENCLOSURE** · Fnclosed ROOF DEAD LOAD (D) (PSF) : 2.10 ROOF COLLATERAL LOAD (C) (PSF) .1.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) :115.00 WIND EXPOSURE CATEGORY : C INTERNAL PRESSURE COEFFICIENT. (GCDi) : 0.18/-0.18 WALL PANEL DESIGN WIND PRESSURE (PSF) : 25.73/-27.87 WIND ENCLOSURE CLASSIFICATION : Enclosed LIVE LOAD PRIMARY FRAMING (PSF) : 20.00 TWESTERNBUILDINGS. TRIB. AREA REDUCTION : No SECONDARY FRAMING (PSF) : 20.00 2135 SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) : 40.00 8 80014 497-ROOF SNOW LOAD, (Pf) (PSF) : 30.00 SNOW EXPOSURE FACTOR, (Ce) : 1.00 ARKER SNOW IMPORTANCE FACTOR, (Is) :1.00 (800)THERMAL FACTOR, (Ct) : 1.00 00 SEISMIC LOAD PHONE: (8' WWW.GREAT SEISMIC IMPORTANCE FACTOR, (Ie) : 1.00 Ś SITE CLASSIFICATION : D AUROR, ш 3033 SPECTRAL RESPONSE ACCELERATION : Ss = 0.440 : S1 = 0.149TS. SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.425 : Sd1 = 0.229SEISMIC DESIGN CATEGORY : D $\alpha$ BASIC SEISMIC FORCE RESISTING SYSTEM :STEEL SYSTEM NOT SPECIFICALLY $\mathbb{R}$ DETAILED FOR RESISTANCE 0: RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) :LONGITUDINAL = 2.63 TOTAL DESIGN BASE SHEAR, (V) (KIPS) $\bigcirc$ :TRANSVERSE = 5.29 $\infty$ :RIGID FRAMES = 3.25RESPONSE MODIFICATION FACTORS, (R) $\Omega = 3.00$ 4 $\Box$ SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.1308 $\sim$ 0 $\overline{Z}$ ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS COUNTY: END USE: R PHONE $\bar{m}$ THIS BUILDING IS DESIGNED USING PANEL SHEAR BRACING. NO ADDITIONAL OPENINGS ARE TO BE ADDED WITHOUT CONSULTING THE ORIGINAL DESIGN ENGINEER OR A LOCAL ENGINEERING PROFESSIONAL. **BUILDING DESCRIPTION:** WIDTH (FT) : 67.00 LENGTH (FT) : 100.00 EAVE HEIGHT AT BSW (FT): 17.00 EAVE HEIGHT AT FSW (FT): 17.00 ROOF SLOPE AT BSW : 2.0:12 ROOF SLOPE AT FSW : 2.0:12 BAY SPACING (FT) : 4 AT 25.00 COVERING AND TRIMS: **ROOF PANELS & TRIMS** PANEL TYPE · 26 GA PRE PANEL COLOR : GALVALUME TRIM COLORS : CHARCOAL GRAY GABLE/EAVE EAVE GUTTER : N / A WALL PANELS & TRIMS PANEL TYPE :26 GA. PBR PANEL COLOR :ASH GRAY THIS SEAL PERTAINS ONLY TO THE MATERIALS TRIM COLORS DESIGNED AND SUPPLIED BY GREAT WESTERN : CHARCOAL GRAY CORNER BUILDINGS. THE DRAWINGS AND THE METAL : CHARCOAL GRAY FRAMED OPENING BUILDING WHICH THEY REPRESENT ARE THE DOWNSPOUTS : N/A PRODUCT OF GREAT WESTERN BUILDINGS. : ASH GRAY BASE THE REGISTERED PROFESSIONAL ENGINEER WHOSE INSULATION SEAL AND SIGNATURE APPEARS ON THESE ROOF INSULATION : N/A DRAWINGS IS EMPLOYED BY GREAT WESTERN WALL INSULATION : N/A

BUILDINGS AND DOES NOT SERVE AS OR

REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS





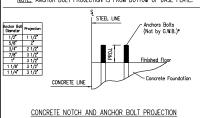
## NOTE:

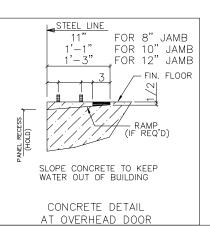
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

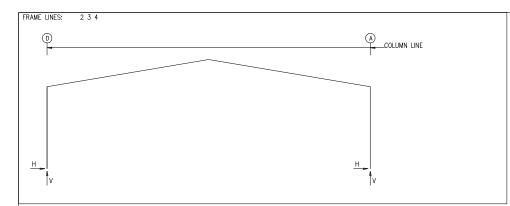
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS, ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





CUSTOMER NAME:	<b>←</b>	ISSNE	DATE DWN. CHK.	CHK.	ENG.
PROJECT NAME:		APPROVAL	04/29/21 MEZ	MEZ	RTS
PROJECT LOCATION:		PERMIT	07/07/21 AA	CAF	RTS
PROJECT COUNTY:		REVISED PERMIT	11/17/23 AA CAF	CAF	RTS
PROJECT END USE:		ERECTION	12/06/23 PKD PKD	PKD	RTS
CUSTOMER PHONE NUMBER:					
CUSTOMER EMAIL:					
SOALE: N.T.S.					
	3 5 5 0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				
	JUJJ J. PARNER RD 12 FLUUR				
	AURORA. CO 80014				
SHEET NUMBER:	PHONE: (800)-497-2135				
4 OF 13	WWW GREATWESTERNBUILDINGS COM				
JOB NUMBER					
93548R1					
SHEET TILE:					
ANCHOR BOLT DETAILS	$\mathcal{S}_{\perp}$				



RIGID	FRAME:		MAXIMUM	REACTION	S, ANC	HOR BOL	TS, & BASE	PLATE	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_React V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	1	19.5	28.9	2	-6.6	-8.3	4	0.750	6.000	10.75	0.375	0.0
2*	А	3 1	6.6 -19.5	-8.3 28.9	1 3	-19.5 6.6	28.9 -8.3	4	0.750	6.000	10.75	0.375	0.0
2*	FRAME li	nes:	2 3 4										

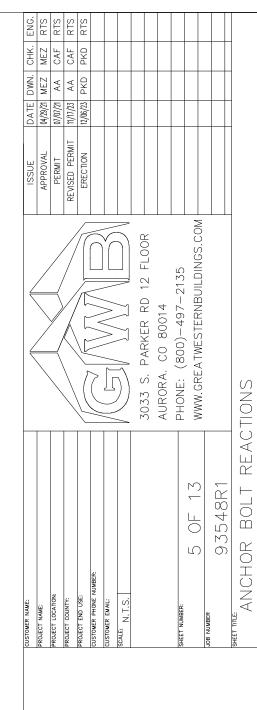
L		TTO MILE TITIES. 2 5 T	
H	NOTEC	FOR DEACTIONS	
ľ	MOTE2	FOR REACTIONS	
	Buil the	ding reactions are based on following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Live Load (psf) Snow Load (psf) Live Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Glosed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= 67.00 = 100.00 = 17.00/17.00 = 2.0:12/2.0:12 = 2.10 = 1.00 = 20.00 = 30.00 = 115.00 = 115.00 = 160-21 c C E Enclosed = 1.00 = 0.64
	ID	Description	
			GWind_Long1L D.6Wind_Long1L Vind_Suction Low Unid_Suction D.6Wind_Long2L R GWind_Long2L L

		±	Reacti	ons(k_)	Panel_	Shear
——Wall Loc l	— Col Line Line			- — Seis Horz	(lb/ Wind	ft) Seis
L_EW F_SW	1 Δ				37 68	17 39
R_EW	A 5				37	17 34
B_SW	D				59	34

ANCHU	K BOLI	20MM	NK I	
QTY	LOCATE ENDWALL ENDWALL FRAME	DIA (in) 5/8" 3/4" 3/4"	A307 A307 A307 A307	-

RIGIE	FRAN	ΛE:	BASI	COLUM	IN REACT	ONS (k )	)										
FRAME Line 2* 2*	Column Line D A	 Horiz 1.7 -1.7	Dead Vert 2.9 2.9	Collo Horiz 0.6 -0.6	oteral- Vert 0.8 0.8	Horiz 11.5 –11.5	-Live Vert 16.8 16.7	Horiz 17.2 –17.2	Snow Vert 25.1 25.1	Wind Horiz -12.8 4.9		-Wind_ Horiz -4.9 12.8	Right1- Vert -12.0 -16.8				
FRAME Line 2* 2*	Column Line D A	Wind_ Horiz -10.2 2.4	Left2- Vert -9.6 -4.9	-Wind_ Horiz -2.4 10.2	Right2- Vert -4.9 -9.6	Wind Horiz -5.5 6.5	_Long1- Vert -15.5 -12.8	Wind Horiz -6.5 5.5	_Long2- Vert -12.8 -15.5	-Seismi Horiz -0.8 -0.8		Seismic Horiz 0.8 0.8	_Right Vert 0.4 -0.4				
FRAME Line 2* 2*	Column Line D A	-MIN_SN Horiz 11.5 -11.5	NOW Vert 16.7 16.7	F1UNB_ Horiz 14.4 -14.4	SL_L- Vert 24.5 14.3	F1UNB_ Horiz 14.4 –14.4	SL_R- Vert 14.3 24.5										
2*	FRAME lir	nes:	2 3 4														
ENDV	VALL (	COLUMI	N:	ВА	SIC COLU	MN REAC	TIONS (k	)									
Frm Line 1 1 1	Line D C B	Dead Vert 0.5 1.2 1.2	Collat Vert 0.1 0.3 0.3		.0 .0	Snow Vert 3.8 9.0 9.0 3.8	Wind Left1 Vert -3.4 -8.4 -5.1 -2.2		2 1 4	Wind Left2 Vert -2.1 -6.0 -2.8 -0.8	Wind Right2 Vert -0.8 -2.8 -6.0 -2.1	? Pr	.3 .3	Wind Suct Horz 2.2 4.7 4.7 2.2	Wind Long1 Vert -3.8 -8.0 -4.5 -2.4	Wind Long2 Vert -2.4 -4.5 -8.0 -3.8	

ENDV	VALL	COL	JMN:		BASIC CO	LUMN R	EACTIONS	` '								
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.5 1.2 1.2 0.5		Collat Vert 0.1 0.3 0.3 0.1	Live Vert 2.5 6.0 6.0 2.5	Snov Vert 3.8 9.0 9.0 3.8		t1 ! t - 4 - 1 -	Wind Right1 Vert 2.2 5.1 8.4 3.4	Wind Left2 Vert -2.1 -6.0 -2.8 -0.8	Wir Ri <u>c</u> Ver -0.1 -2.1 -6.1 -2.	ht2   rt   B - B -	Wind Press Horz 2.0 4.3 4.3 2.0	Wind Suct Horz 2.2 4.7 4.7 2.2	Wind Long1 Vert -3.8 -8.0 -4.5 -2.4	Wind Long2 Vert -2.4 -4.5 -8.0 -3.8
Frm Line 1 1 1	Col Line D C B A	Seis Left Vert 0.0 0.0 -0.2 0.2	-	Seis Right Vert 0.2 -0.2 0.0	Seis Long Horz 0.0 0.1 0.1	-MII Horz 0.0 0.0 0.0 0.0	N_SNOW Vert 2.5 6.0 6.0 2.5	E1UNI Horz 0.0 0.0 0.0 0.0	B_SL_L- Ver 3.7 11.6 3.3	t Hor 0.0 0.0	1.1 3.3 11.6	rt 3				
Frm Line 5 5 5	Col Line A B C D	Dead Vert 0.5 1.2 1.2 0.5		Collat Vert 0.1 0.3 0.3 0.1	Live Vert 2.5 6.0 6.0 2.5	Snov Vert 3.8 9.0 9.0 3.8		t1   t - 4 - 1 -	Wind Right1 Vert 2.2 5.1 8.4 3.4	Wind Left2 Vert -2.1 -6.0 -2.8 -0.8	Wir Ric Ve -0.: -2.: -6.: -2.	ht2   rt   8 - 8 - 0 -	Wind Press Horz 2.0 4.3 4.3 2.0	Wind Suct Horz 2.2 4.7 4.7 2.2	Wind Long1 Vert -3.8 -8.0 -4.5 -2.4	Wind Long2 Vert -2.4 -4.5 -8.0 -3.8
Frm Line 5 5 5	Col Line A B C D	Seis Left Vert 0.0 0.0 -0.2 0.2	-	Seis Right Vert 0.2 -0.2 0.0	Seis Long Horz 0.0 0.1 0.1	- MII Horz 0.0 0.0 0.0 0.0	N_SNOW Vert 2.5 6.0 6.0 2.5	E2UN Horz 0.0 0.0 0.0 0.0	B_SL_L Ver 3.7 11.6 3.3 1.1	t Hor 0.0 0.0	1.1 3.3 11.6	rt 3				
ENDV	VALL	COLI	JMN:		MAXIMUM	REACTIO	NS, ANCHO	OR BOLTS,	& BAS	E PLATES						
Frm Lin			Load Id	Hmax H	umn_React V Vmax	ions(k ) Load Id	Hmin H	V Vmin	Bolt QTY	(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)	-	
1	0		4 1	1.3 0.0	-2.0 4.4	5 4	-1.2 1.3	-2.0 -2.0	2	0.625	3.500	8.000	0.250	0.0		
1	C		6	2.8	-4.3 13.1	5	-2.6 2.8	-4.1 -4.3	4	0.750	6.000	8.000	0.375	0.0		
1	Е		8 10	2.8	-4.3 13.1	9	-2.6 2.8	-4.1 -4.3	4	0.750	6.000	8.000	0.375	0.0		
1	P	٨	11 1	1.3	-2.0 4.4	9 11	-1.2 1.3	-2.0 -2.0	2	0.625	3.500	8.000	0.250	0.0		
5	A	١.	4	1.3	-2.0 4.4	5 4	-1.2 1.3	-2.0 -2.0	2	0.625	3.500	8.000	0.250	0.0		
5	Е		6 12	2.8 0.0	-4.3 13.1	5	-2.6 2.8	-4.1 -4.3	4	0.750	6.000	8.000	0.375	0.0		
5	C	:	8 13	2.8 0.0	-4.3 13.1	9	-2.6 2.8	-4.1 -4.3	4	0.750	6.000	8.000	0.375	0.0		
5	0		11 1	1.3	-2.0 4.4	9 11	-1.2 1.3	-2.0 -2.0	2	0.625	3.500	8.000	0.250	0.0		

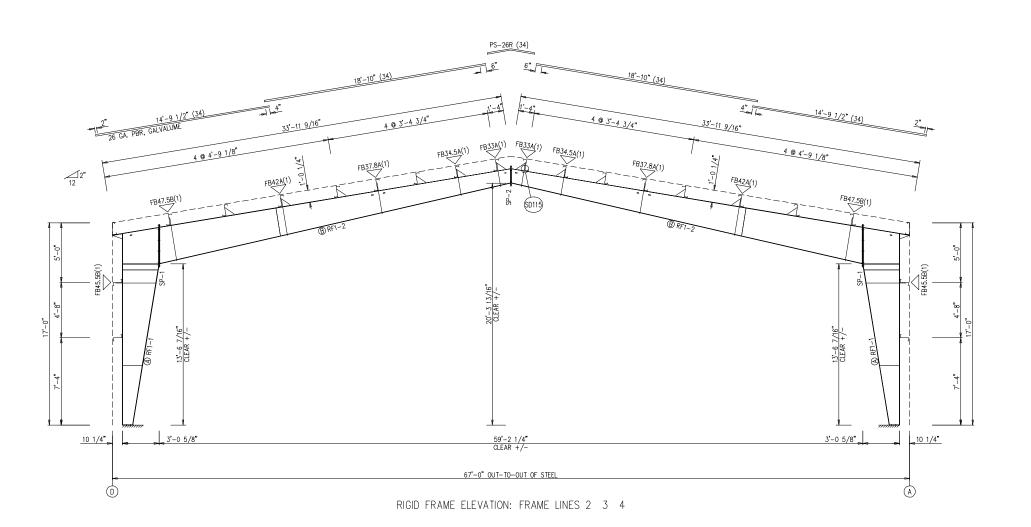


SPL	JCE BOL	T TAB	LE					
Mar	k	Qty Top	Bot	Int	Туре	Dia	Length	
SP- SP-	-1 -2	4 4	4 4	4 0	A325 A325	3/4" 5/8"	2 1/4" 1 3/4"	

ALTERNAT	Е МЕМ	BER
Frame Line	OID	Mark
4	Α	RF1-3
4	В	RF1-4

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
B - 2X2X1/8
A - 2X2X14Ga

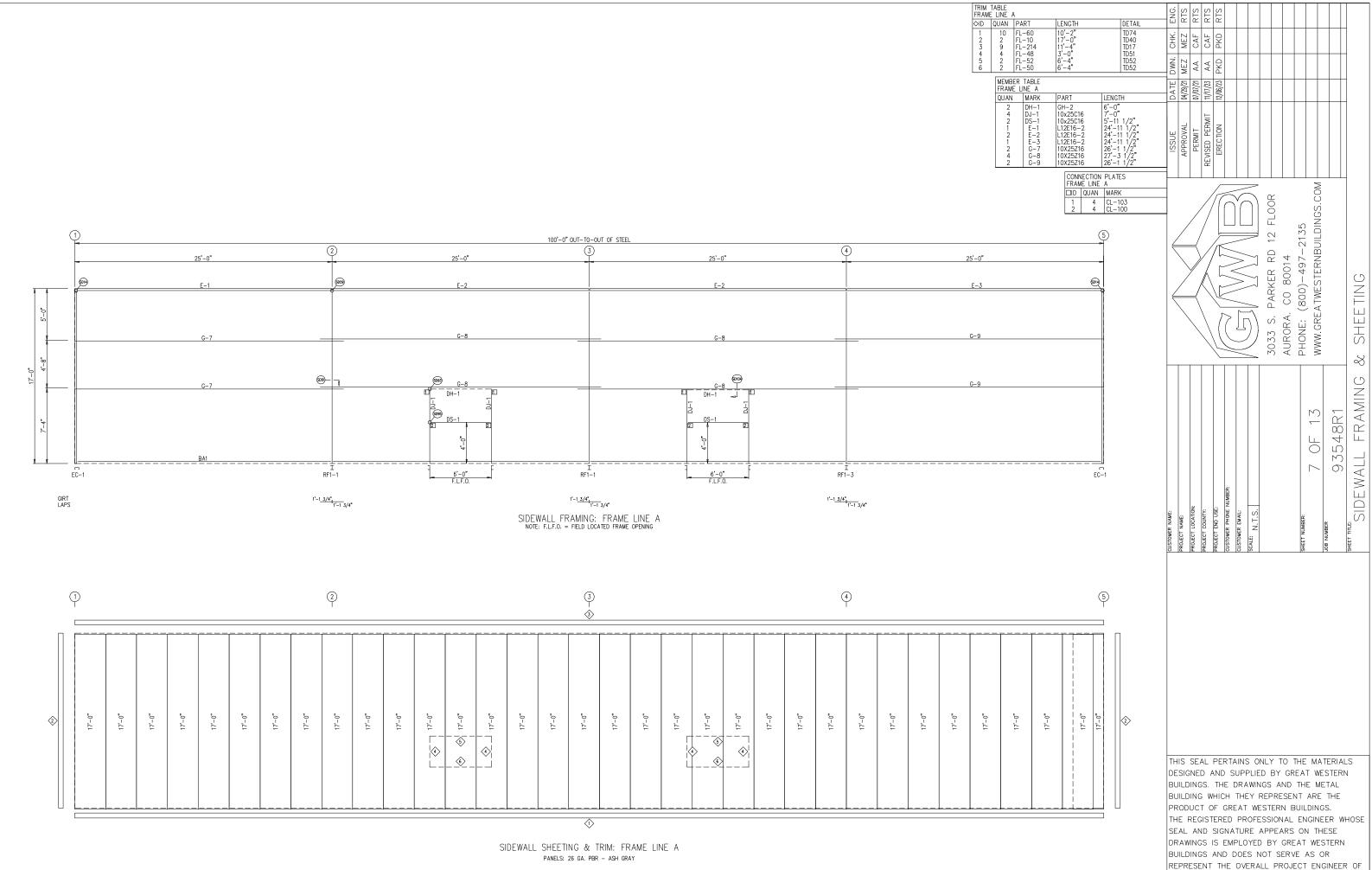
MEMBER 17					
	Web Depth	Web P	late	Outside Flange	Inside Flange
Mark	Start/Énd	Thick	Length		W x Thk x Length
RF1-1	10.0/19.8	0.135	5'-0"	6 x 1/4" x 16'-0 11/16"	6 x 3/8" x 13'-5 3/16"
	19.8/35.5	0.164		6 x 1/4" x 3'-11 1/8"	,
	35.5/36.0	0.250	3'-6 9'/16"	· '	
RF1-2	36.0/28.4	0.164	10'-4 3/4"	6 x 1/4" x 29'-10 11/16"	6 x 3/8" x 10'-9"
		A 175	20' 0"	1 ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '	C v 1//4" v 10' E 1/16"



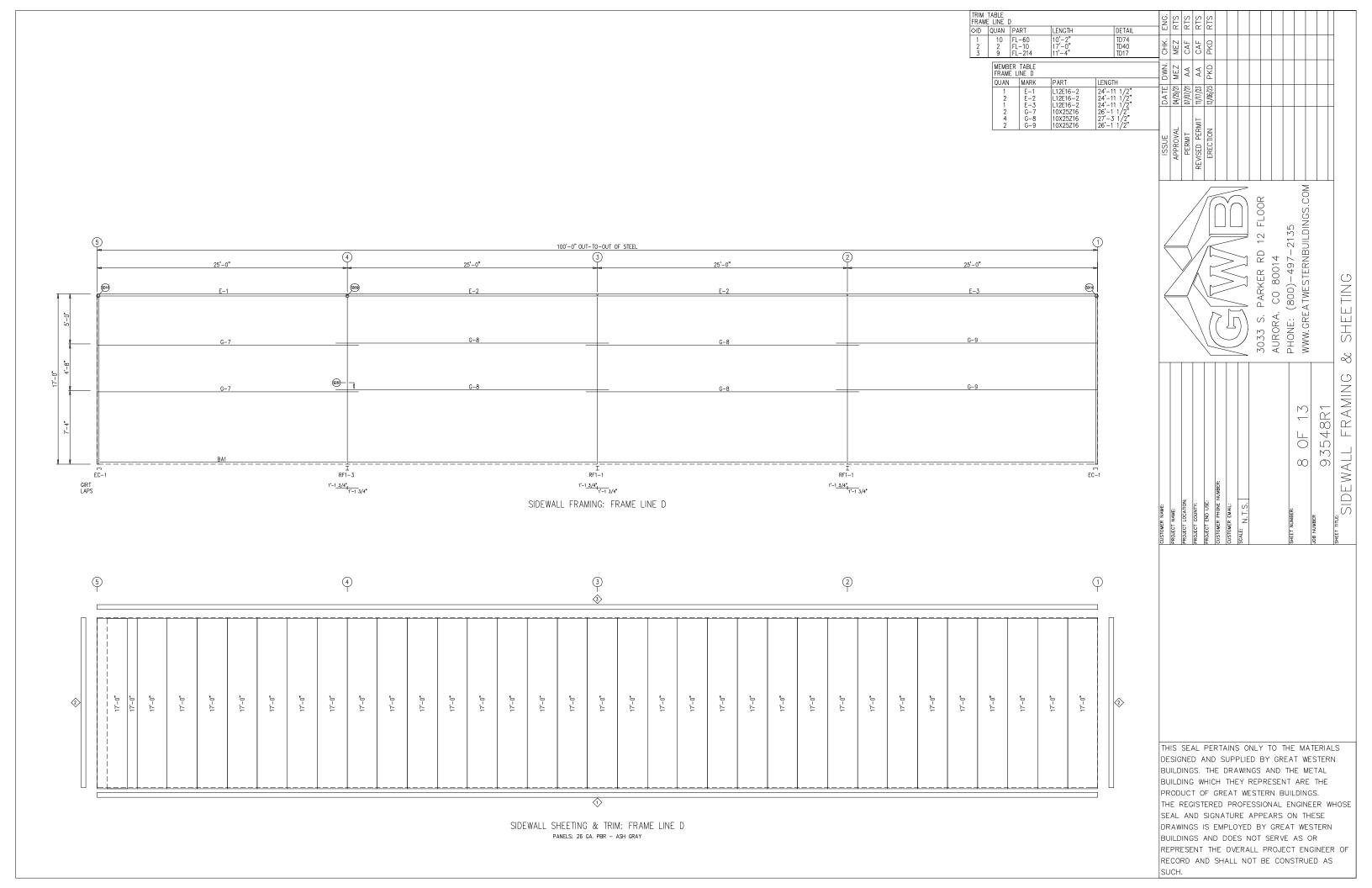
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT COUNTY:
PROJECT END USE:

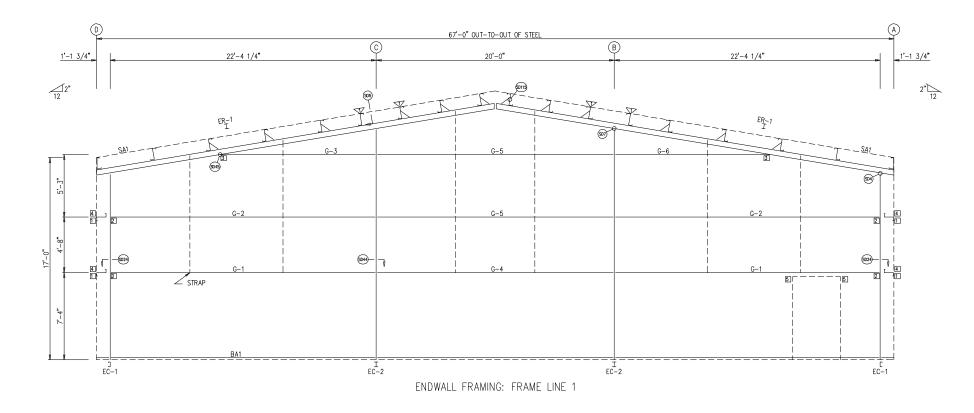
CUSTOMER EVAIL:
SCALE: N.T.S.|

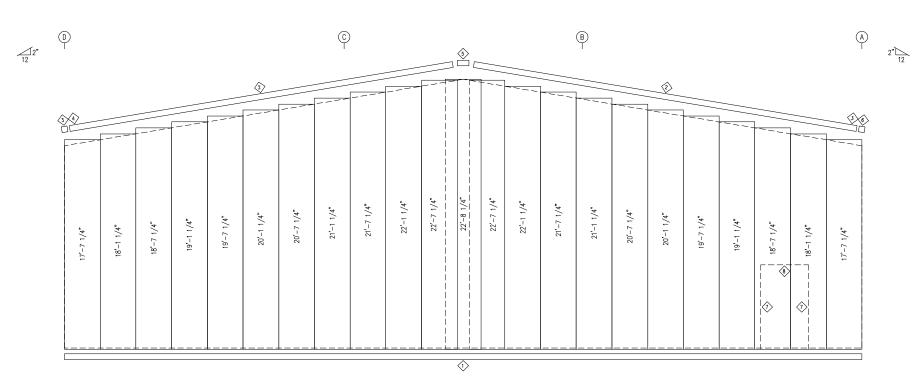
SCALE: N.T



RECORD AND SHALL NOT BE CONSTRUED AS

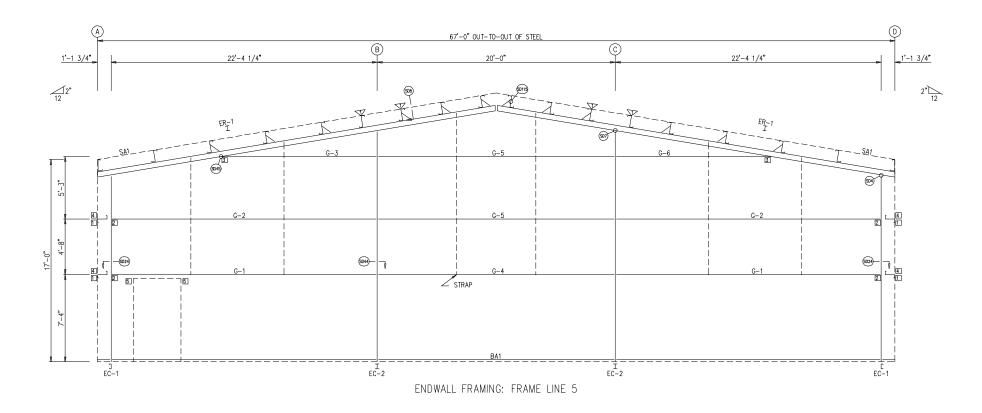


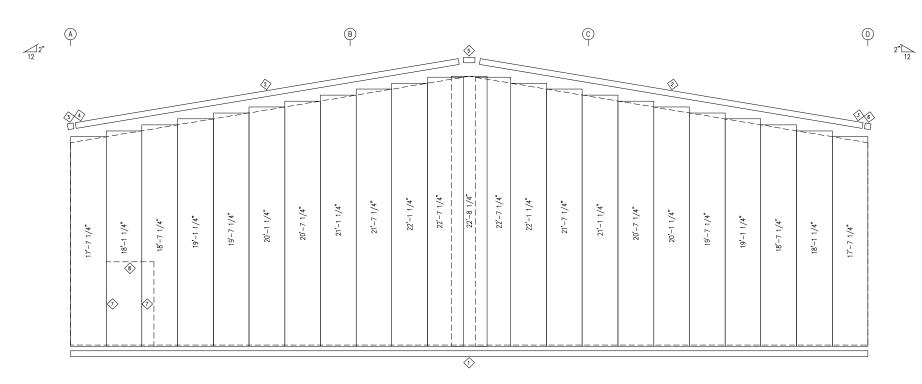




ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - ASH GRAY

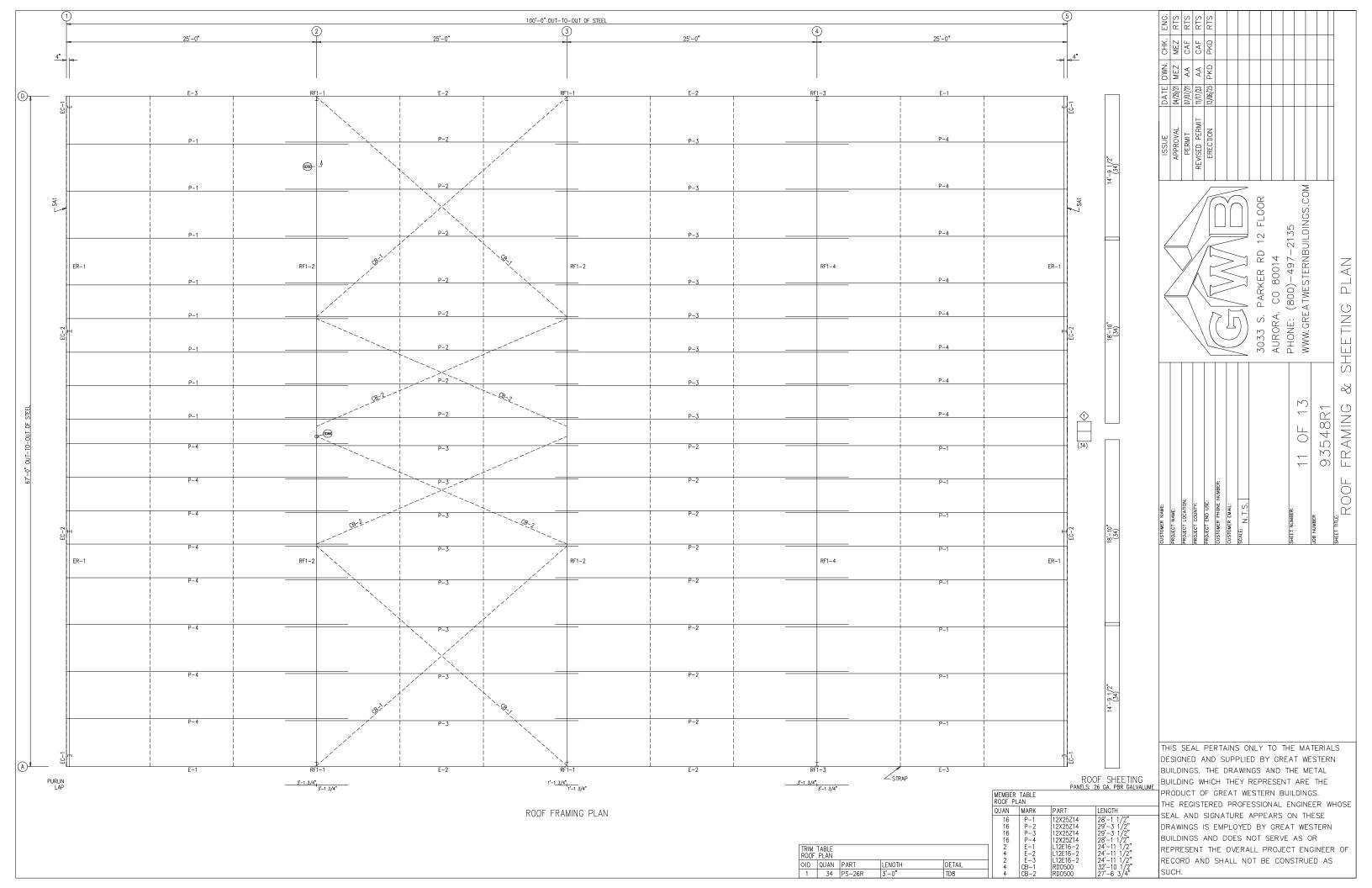
IM TABLE AME LINE 1 DETAIL DETAIL OF CALL OF C		
D QUAN PART LENGTH DETAIL ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		
The content of the		
LOCATION OLIAN TYPE DIA LENGTH		
RR-1/ER-1		
COLUMNS/RAFTER   2   A325   5/8"   1 1/2"		
2 EC-2 W8X10 19'-1 5/8" 2 ER-1 W8X15 33'-11 5/16"	$\coprod$	
2   ER-1   W6X15   33-11 5/16"   2   G-1   8X25X12   21'-11 15/16"   2   G-2   8X35X14   21'-11 15/16"   1   G-3   8X25X16   11'-0 13/16"   1   G-4   8X35X16   19'-3 13/16"   2   G-5   8X25X16   11'-0 13/16"   1   G-6   8X25X16   11'-0 13/16"   1   G-6   8X25X16   11'-0 13/16"   G-6   G-7   G-	C.C.C	
2   G=2   8X35214   21 - 11   15/16   1   G=3   8X25216   11'-0   13/16'   1   G=4   8X35216   19'-3   13/16'   2   G=5   8X25216   19'-3   13/16'   1   G=6   8X25216   11'-0   13/16'   1   G=6   11'-0   13/16'	25   15   15	
CONNECTION PLATES   FRAME LINE 1	WWW.GREAIWESIERNBUILDINGS.COM	
DID   QUAN   MARK	T Z	
1	MES.	S N
FLANGE BRACE TABLE FRAME LINE 1  FINE COUNTY DATE:	(F.A.)	
TINTWILL LINE   TINTWILL LIN	W. Gr	SHE
M A W W A H	<b>S</b>	8
		ENDWALL FRAMING
	548R1	ZAN
	2 2	
	9 63	
HER I		<u>/</u>
NAME: WE: CATION: CATION: DUNTY: T. S. T. S.		
CUSTOMER NAME: PROJECT LOCATION PROJECT COUNTY: PROJECT END USE: CUSTOMER PHONE CUSTOMER ENAIL: SCALE: N. T. S.	3 NUMBER	SHEET IILE
10 184 184 184 110 100 100 11 11 11 11 11 11 11 11 11	900	35
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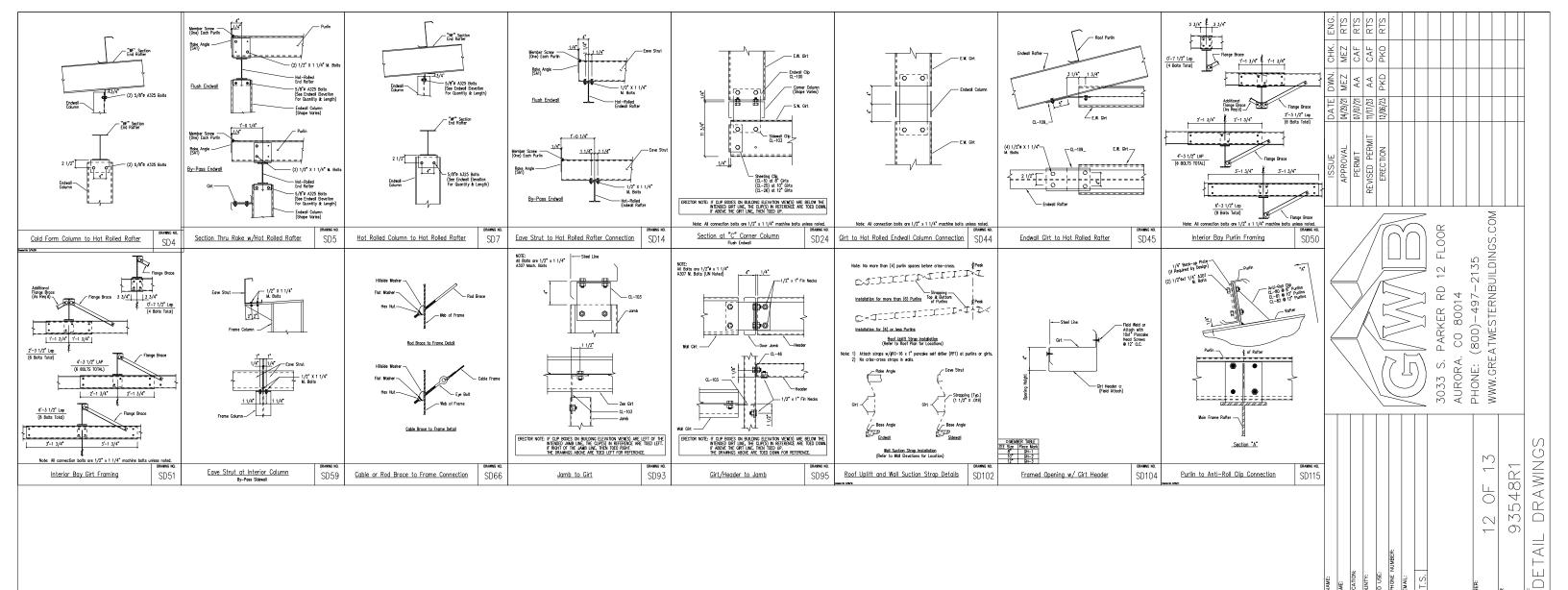


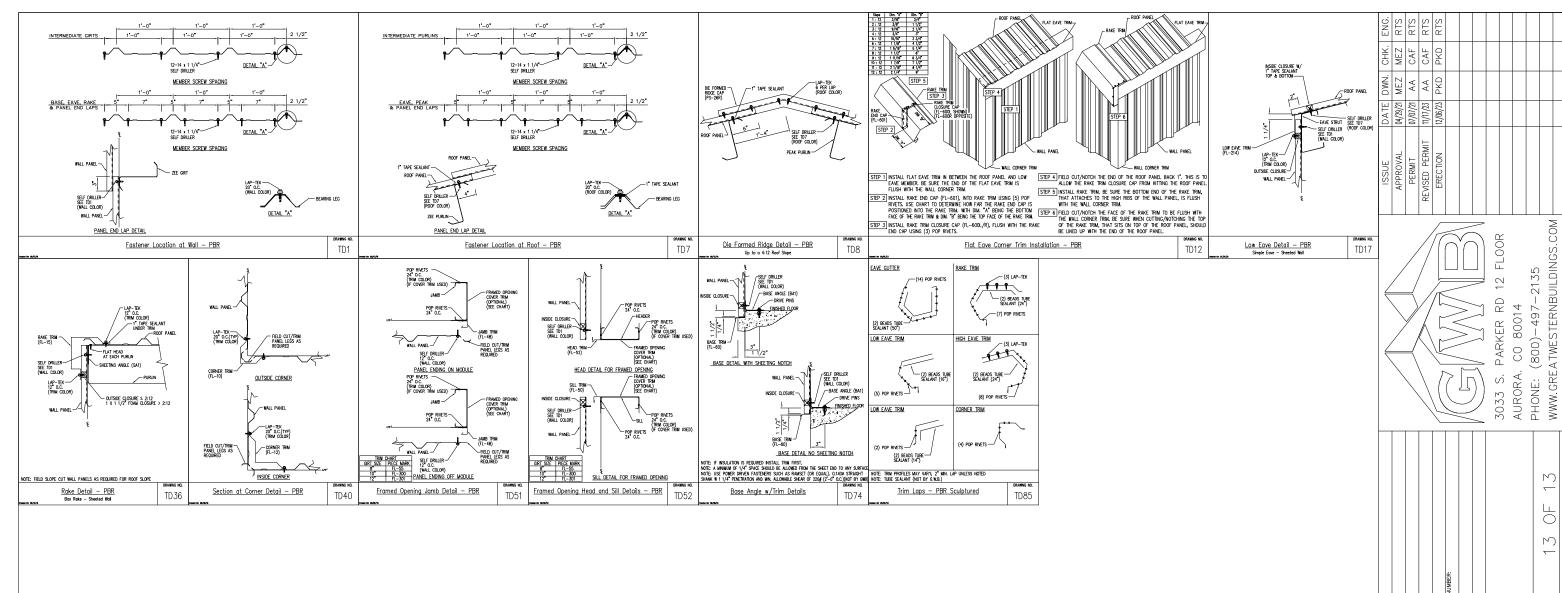


ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 GA. PBR - ASH GRAY

AME LINE 5 D QUAN PART	LENGTH	DETAIL	ENG	RTS	RTS	RTS	RTS									
7 FL-60 6 FL-15 3 2 FL-601C	10'-2" 11'-10" 7 13/16" 5 5/8" 1'-4" 5 5/8" 7'-2" 4'-4"	TD74 TD36 TD85	CHK.	MEZ	CAF	CAF	PKD									
1   FL-600L 5   1   FL-17 6   1   FL-600R 7   2   FL-48 8   1   FL-52	5 5/8 1'-4" 5 5/8" 7'-2"	TD12 TD12 TD51	DWN.	MEZ	ΑA	ΑA	PKD									
BOLT TABLE	4'-4"	TD52	DATE	04/29/21	12/20/20	11/17/23	12/06/23									
FRAME LINE 5 LOCATION ER-1/ER-1 COLUMNS/RAFTER  MEMBER TABLE FRAME LINE 5	QUAN TYPE DIA 8 A325 5/8 2 A325 5/8		ISSUE	APPROVAL	PERMIT	REVISED PERMIT	ERECTION									
OUAN         MARK           2         EC-1           2         EC-2           2         ER-1           2         G-1           2         G-2           1         G-3           1         G-5           1         G-6	PART	7/8" 5/8" 1 5/16" 1 15/16" 1 15/16" 1 15/16" 1 13/16" 13/16" 13/16" 13/16"  ES	←			REWI					3033 S. PARKER RD 12 FLOOR	AURORA. CO 80014	PHONE: (800)-497-2135	WWW.GREATWESTERNBUILDINGS.COM		& SHEETING
			lœ:						CUSTOMER EMAIL:	SCALE: N.T.S.			SHEET NUMBER:	5	93548R1	SHEET THE ENDWALL FRAMING &
			DE BU BU PR TH SE DR BU RE	SIG ILD OD E F AL AW ILD PRI	ING ING UC AN ING ING	D A SS. WIT C SIST SS I	TH THIC ERE SIG S E AND	SI E C CH GRE D NA NA NA D C	JPF )RA THE AT PRO TUR PLO )OE:	PLIED WIN Y R WE OFE RE A YED S N	D BY  STER  SSIOI  PPE  BY  CT S  CT	GRI ND ESEN NAL ARS GRE ERVI	EAT THE UILDI ENGI ON AT W E AS	WES MET NGS NEE THE VEST	THE S. IR WH SE TERN	HOSE







THIS SEAL PERTAINS ONLY TO THE MATERIALS
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BUILDINGS. THE DRAWINGS AND THE METAL
BUILDING WHICH THEY REPRESENT ARE THE
PRODUCT OF GREAT WESTERN BUILDINGS.
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SEAL AND SIGNATURE APPEARS ON THESE
DRAWINGS IS EMPLOYED BY GREAT WESTERN
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RECORD AND SHALL NOT BE CONSTRUED AS

DRAWINGS

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