

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HÌGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN TENSILE STRENGTH

Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & & LESS) A307 A325-TYPE 1 HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBISTE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT : A) ENDWALL RAFTER SPLICE $-5/8"\phi \times 0"-13/4"$ A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE - BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO DROVIDE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE
CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND
ARE NOT SUBJECT TO CLAIM, NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED
MISSITS WITHOUT THE PRIOR APPROVAL OF M.B.S. MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVIN
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
 IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STELL PLANS AND PLANS FOR
 OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
 PRACTICES, LATEST EDITION)
- PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL_ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

- THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST ENTITION)
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD
- 9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE

CONCRETE.

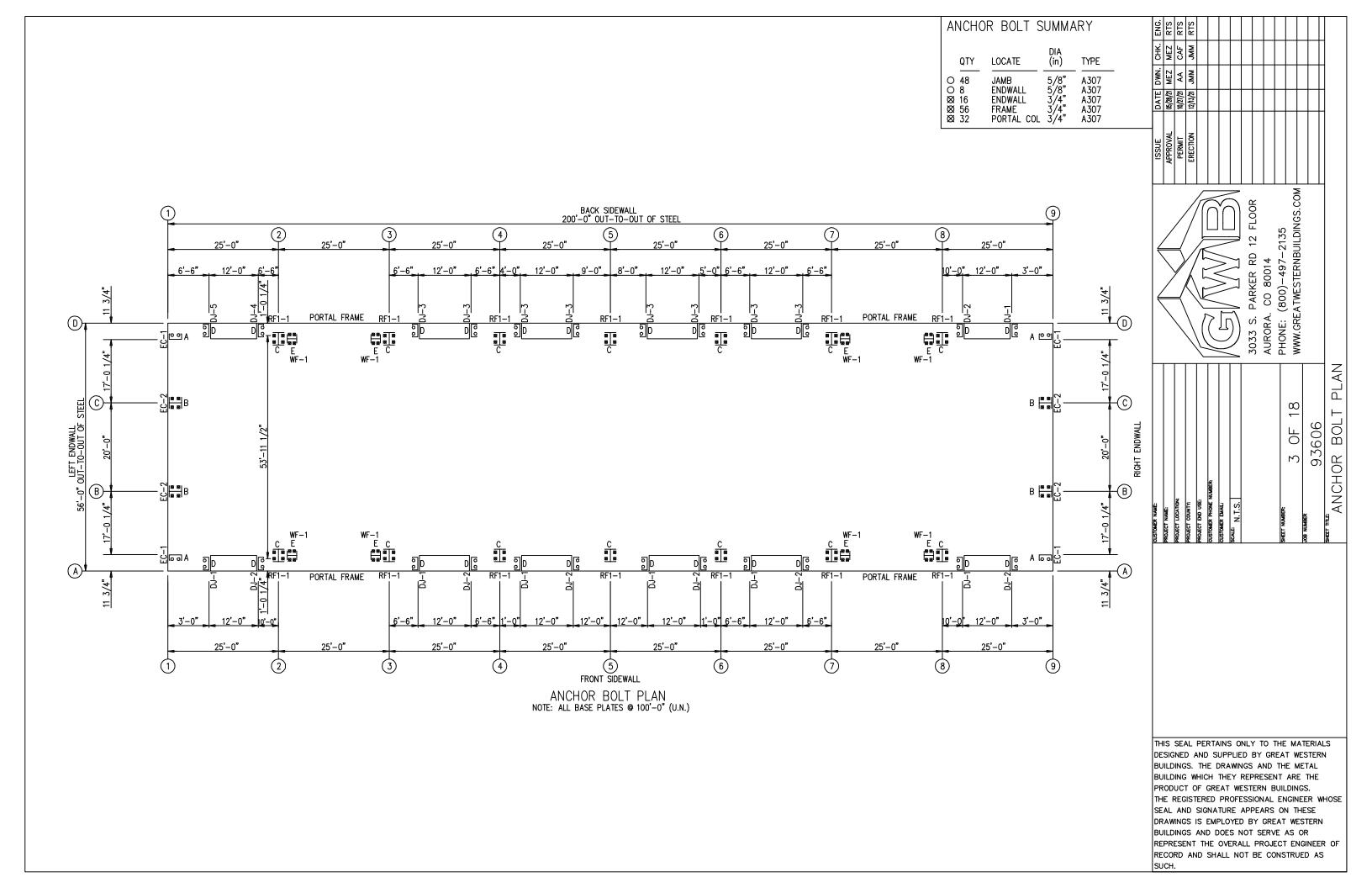
UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BY DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)

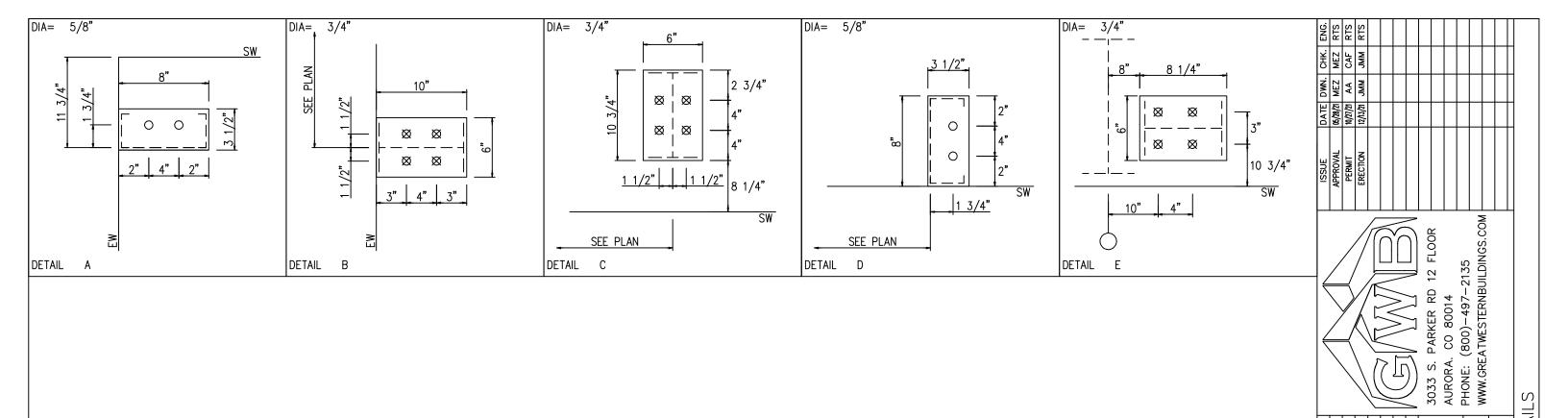
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUITING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST ENTION).
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wiring, or tubing onto galvalume should be appear.
- 2.13 SAFETY COMMITMENT. METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRIONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES, DAILY MEETINGS HICHLICHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR INC. THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: **DESIGN LOADS:** DESIGN CODE / WIND CODE : IBC-18 :II - Normal OCCUPANCY / RISK CATEGORY ENCLOSURE : Closed ROOF DEAD LOAD (D) (PSF) : 2.00 ROOF COLLATERAL LOAD (C) (PSF) : 2.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) : 115.0 WIND EXPOSURE CATEGORY : C INTERNAL PRESSURE COEFFICIENT, (GCpi) :0.18/-0.18 WALL PANEL DESIGN WIND PRESSURE (PSF) : 30.22/-32.78 WIND ENCLOSURE CLASSIFICATION : Closed FLOOR PRIMARY FRAMING (PSF) : 20.00 AURORA. CO 80014 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS. TRIB. AREA REDUCTION : No SECONDARY FRAMING (PSF) : 20.00 SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) : 15.00 ROOF SNOW LOAD, (Pf) (PSF) : 15.0 SNOW EXPOSURE FACTOR, (Ce) :1.00 ARKER SNOW IMPORTANCE FACTOR, (Is) : 1.00 THERMAL FACTOR, (Ct) : 1.00 SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) : 1.00 ι SITE CLASSIFICATION : d SHE 3033 SPECTRAL RESPONSE ACCELERATION :Ss = 0.169 :S1 = 0.084SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.180 : Sd1 = 0.134SEISMIC DESIGN CATEGORY : C BASIC SEISMIC FORCE RESISTING SYSTEM :STEEL SYSTEM NOT SPECIFICALLY OVE DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) $\tilde{\circ}$:BRACED FRAMES (OCBF/OMF) TOTAL DESIGN BASE SHEAR, (V) (KIPS) :LONGITUDINAL = 4.62 \circ :TRANSVERSE = 4.64 9 9 :RIGID FRAMES = 3.00RESPONSE MODIFICATION FACTORS, (R) $\Omega = 3.00$ 0 9 :SW WIND BENT = 3.00 $\Omega = 3.00$ NG α Ó SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.0601 :SW WIND BENT = 0.0601 $\overline{\Box}$ ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS $_{\Omega}$ **BUILDING DESCRIPTION:** WIDTH (FT) : 56.0 LENGTH (FT) : 200.0 EAVE HEIGHT AT BSW (FT):18.0 EAVE HEIGHT AT FSW (FT): 18.0 ROOF SLOPE AT BSW : 4.0:12 ROOF SLOPE AT FSW : 4.0:12 BAY SPACING (FT) :8 AT 25 **COVERING AND TRIMS:** ROOF PANELS & TRIMS :26 GA. PBR PANEL TYPE :POLAR WHITE PANEL COLOR TRIM COLORS :CHARCOAL GRAY GABLE/EAVE EAVE GUTTER :CHARCOAL GRAY WALL PANELS & TRIMS PANEL TYPE :26 GA. PBR PANEL COLOR :LIGHT STONE THIS SEAL PERTAINS ONLY TO THE MATERIALS TRIM COLORS DESIGNED AND SUPPLIED BY GREAT WESTERN :CHARCOAL GRAY CORNER BUILDINGS. THE DRAWINGS AND THE METAL :CHARCOAL GRAY FRAMED OPENING BUILDING WHICH THEY REPRESENT ARE THE DOWNSPOUTS :CHARCOAL GRAY PRODUCT OF GREAT WESTERN BUILDINGS. :LIGHT STONE BASE THE REGISTERED PROFESSIONAL ENGINEER WHOSE INSULATION SEAL AND SIGNATURE APPEARS ON THESE ROOF INSULATION : 9 1/2" (R-30) SKYLINER WALL INSULATION : 6" (R-19) WMP-VR DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS





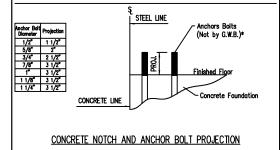
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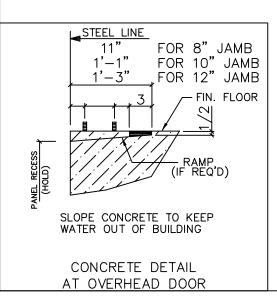
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

Anchor Bolt Diameters have been designed by the metal building engineer based on aisc method with combined shear and tension.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDINGS WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

DETAIL

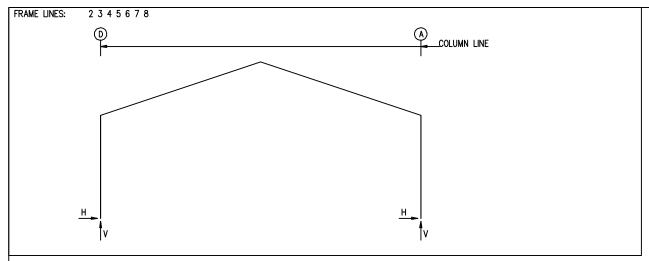
BOL

ANCHOR

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IGID I	FRAME:		MAXIMUM	REACTION	IS, ANCI	OR BOLTS	S, & BASE	PLATE	:S				
Frm Line	Col Line	Load Id	Hmax H	imn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Boli QTY	(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	1	7.9	17.7	2	-5.2	-8.2	4	0.750	6.000	10.75	0.375	0.0
2*	Α	3 1	5.2 -7.9	-8.2 17.7	1 3	-7.9 5.2	17.7 -8.2	4	0.750	6.000	10.75	0.375	0.0
2*	FRAME lir	nes:	2 3 4 5	5 6 7 8									

RIGIE	FRAN	ΛE:	BASI	C COLUM	IN REACT	10NS (k)						
FRAME Line 2* 2*	Column Line D A	 Horiz 0.9 -0.9	-Dead Vert 2.2 2.2		ateral— Vert 1.5 1.5	Horiz 6.4 -6.4	-Live Vert 14.0 14.0	Horiz 4.8 -4.8	-Snow Vert 10.5 10.5	———Wind Horiz —9.6 0.1	d_Left1- Vert -15.8 -11.2	-Wind_ Horiz -0.1 9.6	Right1- Vert -11.2 -15.8
FRAME Line 2* 2*	Column Line D A	Wind Horiz -9.3 -0.2	_Left2- Vert -9.4 -4.8	-Wind_ Horiz 0.2 9.3	Right2- Vert -4.8 -9.4	Wind Horiz -0.2 2.1	I_Long1- Vert -13.4 -12.0	Wind Horiz -2.1 0.2	d_Long2- Vert -12.0 -13.4	-Seism Horiz -0.3 -0.3	vic_Left Vert -0.2 0.2	Seismic Horiz 0.3 0.3	:_Right Vert 0.2 -0.2
FRAME Line 2* 2*	Column Line D A	F1UNB_ Horiz 3.8 -3.8	SL_L- Vert 9.8 5.9	F1UNB_ Horiz 3.8 -3.8	SL_R- Vert 5.9 9.8								
2*	FRAME lir	es:	2 3 4	5 6 7	8								

NOTES FOR REACTIONS
Building reactions are based on the following building data: Width (ft) = 56.0 Length (ft) = 200.0 Eave Height (ft) = 18.0/18.0 Roof Slope (rise/12) = 4.0:12/4.0:12 Dead Load (psf) = 2.00 Collateral Load (psf) = 2.00 Live Load (psf) = 20.00 Snow Load (psf) = 15.0 Wind Speed (mph) = 115.0 Wind Code = IBC-18 Exposure = C Closed/Open = Closed Importance Wind = 1.00 Importance Seismic = 1.00 Seismic Zone = C Seismic Coeff (Fa*Ss) = 0.27
ID Description
1 Dead+Collateral+Live 2 0.6Dead+0.6Wind_Left1 3 0.6Dead+0.6Wind_Right1 4 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L 5 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 6 0.6Dead+0.6Wind_Left1+0.6Wind_Suction 7 0.6Dead+0.6Wind_Right1+0.6Wind_Suction 8 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L 9 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

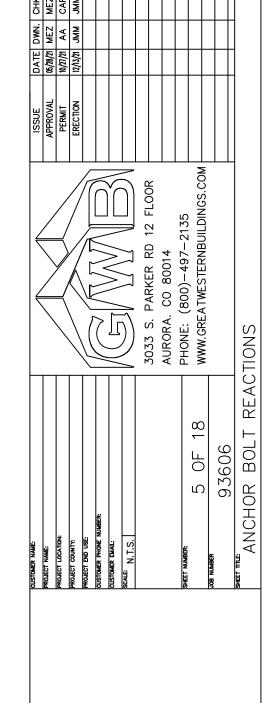
——Wa	ı —	- Col	±	Reacti	ions(k) - —Seis	mic –	Panel_ · (lb)	Shear /ft)	
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Note
L_EW	1						51	19	
F_SW	Α .	2,3 7,8					E4	40	(a) (a)
R_EW B_SW	9 D	7,8 2,3					51	19	(a) (a)

ANCHO	R BOLT S	SUMM/	ARY	
QTY 	LOCATE JAMB ENDWALL ENDWALL FRAME PORTAL COL	DIA (in) 5/8" 5/8" 3/4" 3/4"	A307 A307 A307 A307 A307 A307	

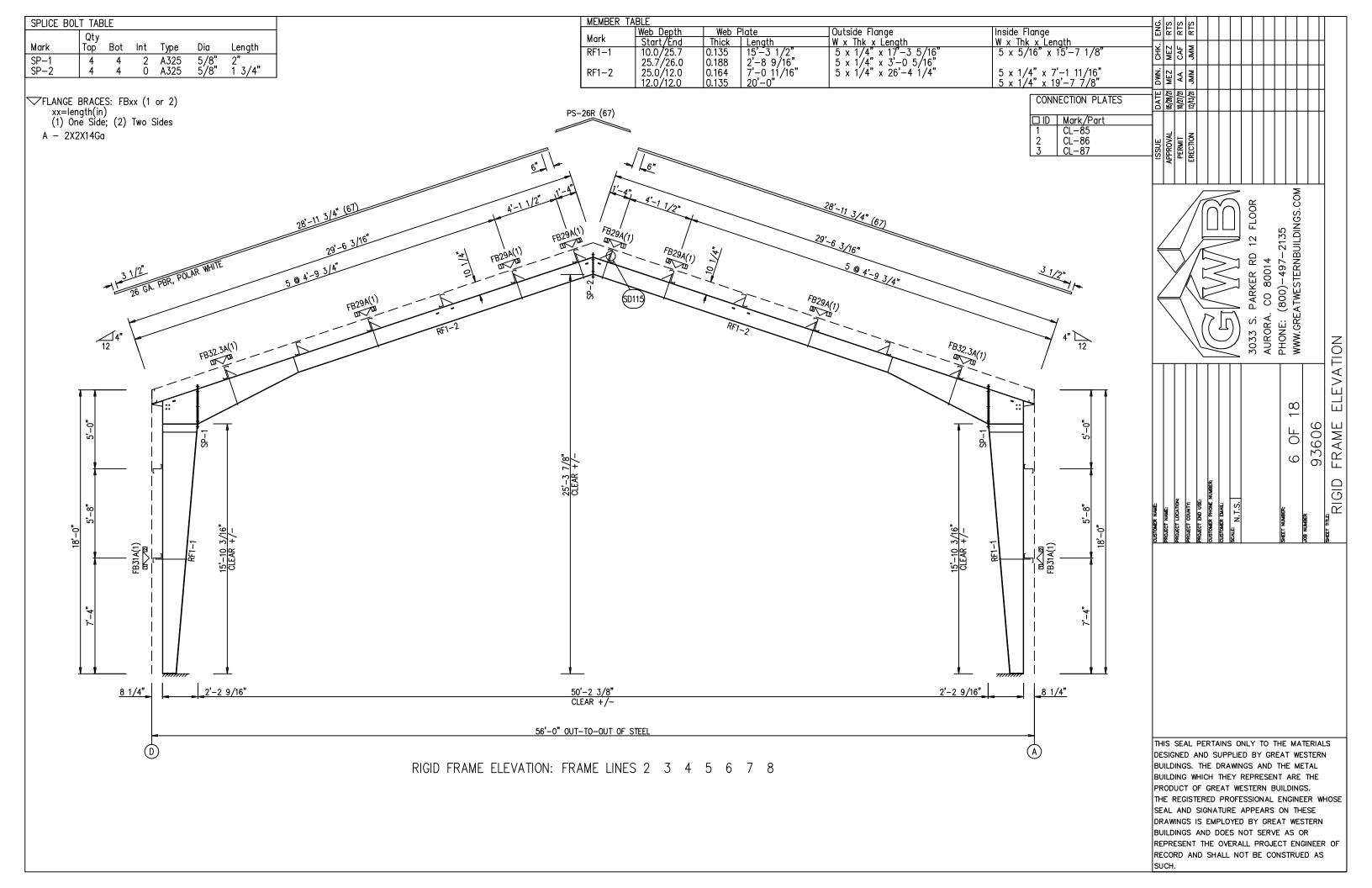
ENDW	/ALL	COLUN	1N:	BASIC CO	DLUMN RE	ACTIONS (k)								
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.3 1.0 1.0 0.3	Collat Vert 0.2 0.6 0.6 0.2	Live Vert 1.8 5.3 5.3	Snow Vert 1.3 4.0 4.0 1.3	Wind Left1 Vert -2.5 -7.6 -5.3 -1.9	Wind Right1 Vert -1.9 -5.3 -7.6 -2.5	Wind Left2 Vert -1.1 -5.7 -3.5 -0.5	Wind Right2 Vert -0.5 -3.5 -5.7 -1.1	Wind Press Horz -1.9 -5.0 -1.9	Wind Suct Horz 2.2 5.6 5.6 2.2	Wind Long1 Vert -3.3 -6.8 -3.7 -2.5	Wind Long2 Vert -2.5 -3.7 -6.8 -3.3	
Frm Line 1 1 1	Col Line D C B A	Seis Left Vert 0.0 0.0 -0.1	Seis Right Vert 0.1 -0.1 0.0	E1UNB_5 Horz 0.0 0.0 0.0	SL_L- Vert 1.3 4.6 1.8 0.3	E1UNB_SL_R- Horz Vert 0.0 0.3 0.0 1.8 0.0 4.6 0.0 1.3								
Frm Line 9 9 9	Col Line A B C D	Dead Vert 0.3 1.0 1.0 0.3	Collat Vert 0.2 0.6 0.6 0.2	Live Vert 1.8 5.3 5.3	Snow Vert 1.3 4.0 4.0 1.3	Wind Left1 Vert -2.5 -7.6 -5.3 -1.9	Wind Right1 Vert -1.9 -5.3 -7.6 -2.5	Wind Left2 Vert -1.1 -5.7 -3.5 -0.5	Wind Right2 Vert -0.5 -3.5 -5.7 -1.1	Wind Press Horz -1.9 -5.0 -5.0	Wind Suct Horz 2.2 5.6 5.6 2.2	Wind Long1 Vert -3.3 -6.8 -3.7 -2.5	Wind Long2 Vert -2.5 -3.7 -6.8 -3.3	
Frm Line 9 9 9	Col Line A B C D	Seis Left Vert 0.0 0.0 -0.1	Seis Right Vert 0.1 -0.1 0.0	E2UNB_ Horz 0.0 0.0 0.0 0.0	SL_L- Vert 1.3 4.6 1.8 0.3	E2UNB_SL_R- Horz Vert 0.0 0.3 0.0 1.8 0.0 4.6 0.0 1.3								
ENDW	/ALL	COLUM	IN:	MAXIMUM	REACTION	S, ANCHOR BOL	TS, & BAS	E PLATES						
Frm Line			oad Hmax Id H	lumn_Reac V Vmax		Hmin V H Vmin	— Bolt QTY	(in) DIA	Base_Plate		Grout (in)	_		
1	D)	4 1.3	-1.8	5	-1.1 -1.8	2	0.625	3.500 8.00	0 0.250	0.0			

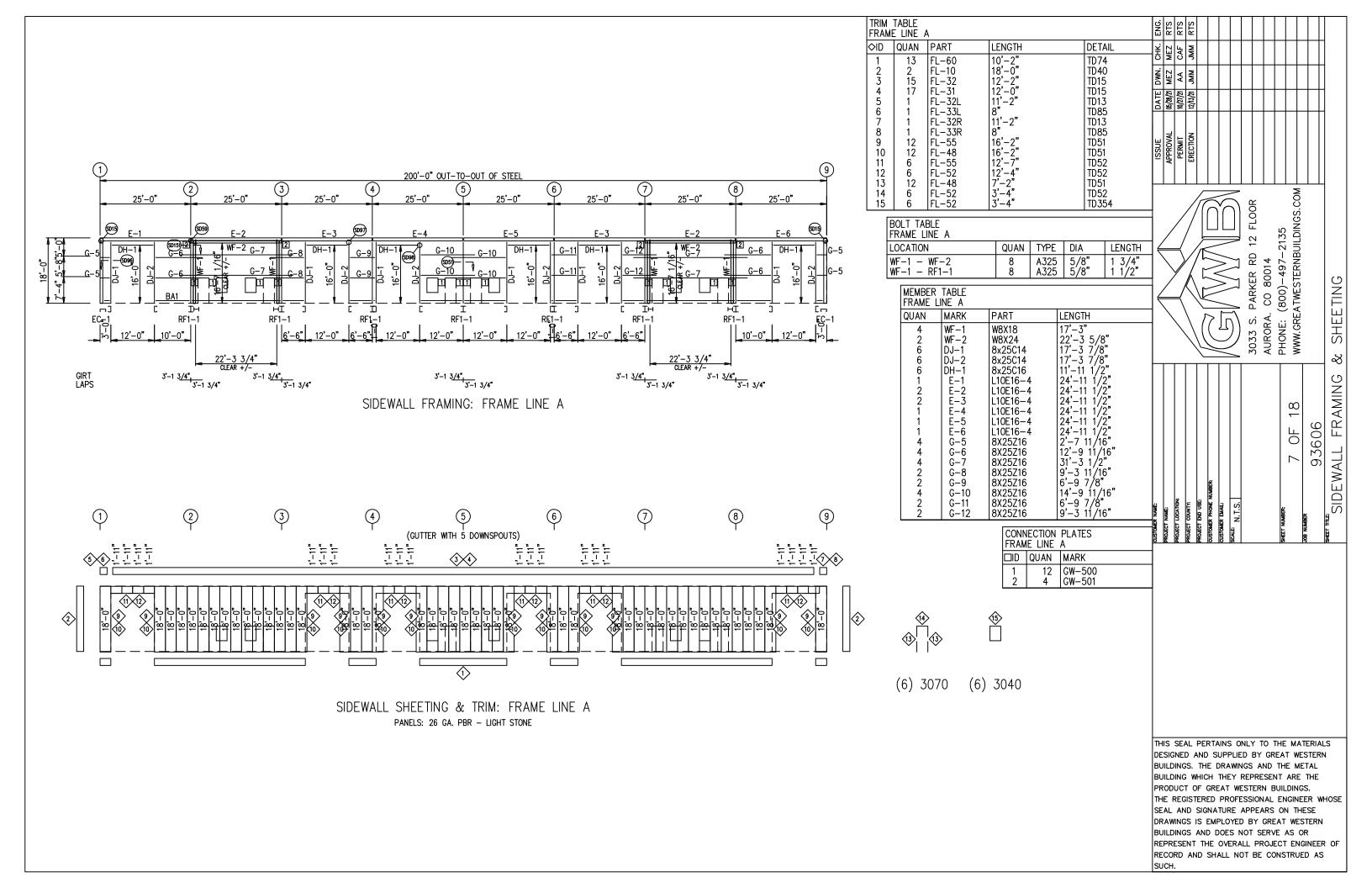
Frm	Col	Load	Coli	umn_Reac	tions(k Load) Hmin		Bol [.]	t(in)	Base	e_Plate(in)		Grout
Line	Line	ld I	H	Vmax	ld	H	Vmin	QTY	DIA	Width	Length	Thick	(in)
1	D	4 1	1.3 0.0	-1.8 2.3	5 4	-1.1 1.3	-1.8 -1.8	2	0.625	3.500	8.000	0.250	0.0
1	С	6 1	3.3 0.0	-4.0 6.8	5 6	-3.0 3.3	-3.5 -4.0	4	0.750	6.000	10.00	0.375	0.0
1	В	7 1	3.3 0.0	-4.0 6.8	8 7	-3.0 3.3	-3.5 -4.0	4	0.750	6.000	10.00	0.375	0.0
1	Α	9 1	1.3 0.0	-1.8 2.3	8 9	-1.1 1.3	−1.8 −1.8	2	0.625	3.500	8.000	0.250	0.0
9	Α	4 1	1.3 0.0	-1.8 2.3	5 4	-1.1 1.3	−1.8 −1.8	2	0.625	3.500	8.000	0.250	0.0
9	В	6 1	3.3 0.0	-4.0 6.8	5 6	-3.0 3.3	-3.5 -4.0	4	0.750	6.000	10.00	0.375	0.0
9	С	7 1	3.3 0.0	-4.0 6.8	8 7	-3.0 3.3	-3.5 -4.0	4	0.750	6.000	10.00	0.375	0.0
9	D	9 1	1.3 0.0	-1.8 2.3	8 9	-1.1 1.3	−1.8 −1.8	2	0.625	3.500	8.000	0.250	0.0

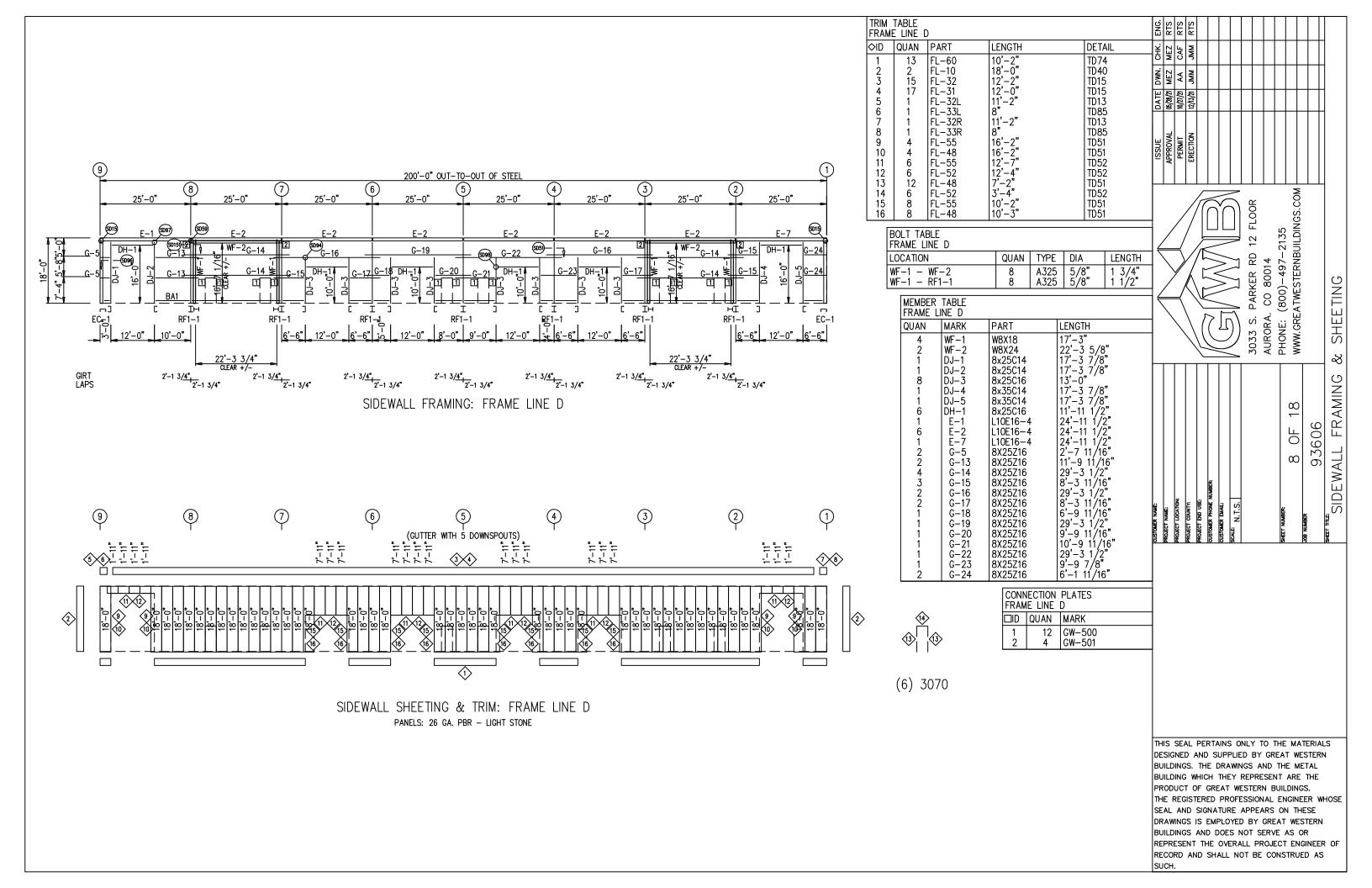
WIND BENT REA	CTIONS										
		— Col Line	Wind Horz	± Reac d(k) Vert	tions Seismic Horz	(k) Vert	Bolt QTY	(in) DIA	Base_l Width	Plate(in) Length	Thick
H	F_SW A F_SW A F_SW A F_SW A B_SW D B_SW D B_SW D B_SW D	2 3 7 8 8 7 3 2	1.5 1.5 1.5 1.5 1.5 1.5 1.5	2.2 2.2 2.2 2.2 2.2 2.2 2.2 2.2 2.2	0.6 0.6 0.6 0.6 0.6 0.6 0.6	0.9 0.9 0.9 0.9 0.9 0.9 0.9	4 4 4 4 4 4 4	0.750 0.750 0.750 0.750 0.750 0.750 0.750 0.750	6.000 6.000 6.000 6.000 6.000 6.000 6.000	8.25 8.25 8.25 8.25 8.25 8.25 8.25 8.25	0.375 0.375 0.375 0.375 0.375 0.375 0.375 0.375

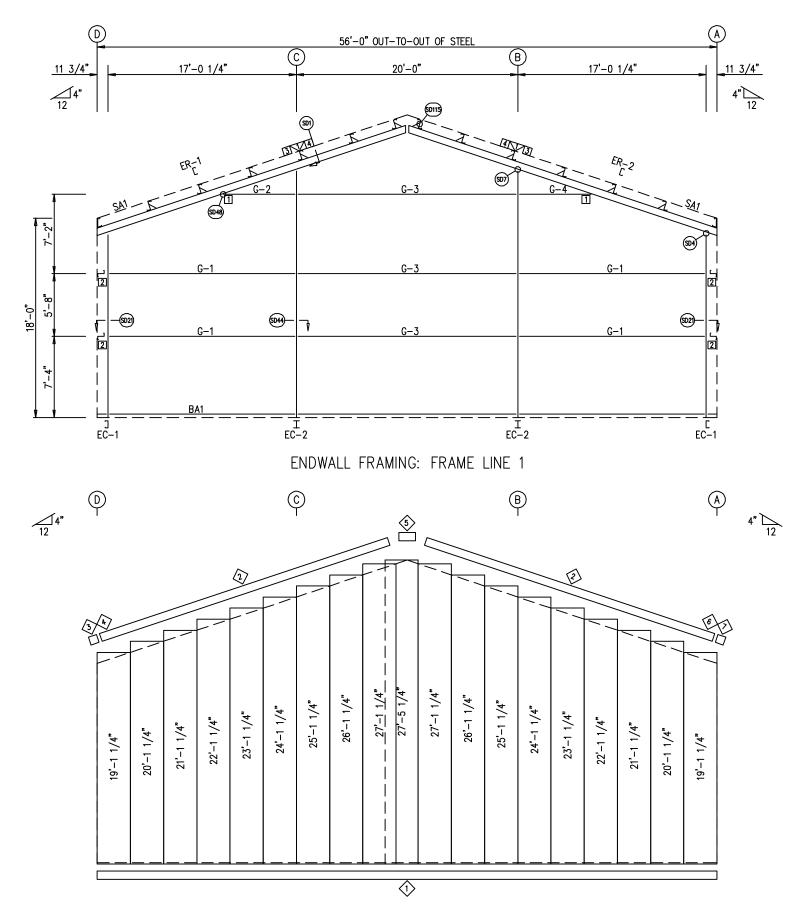


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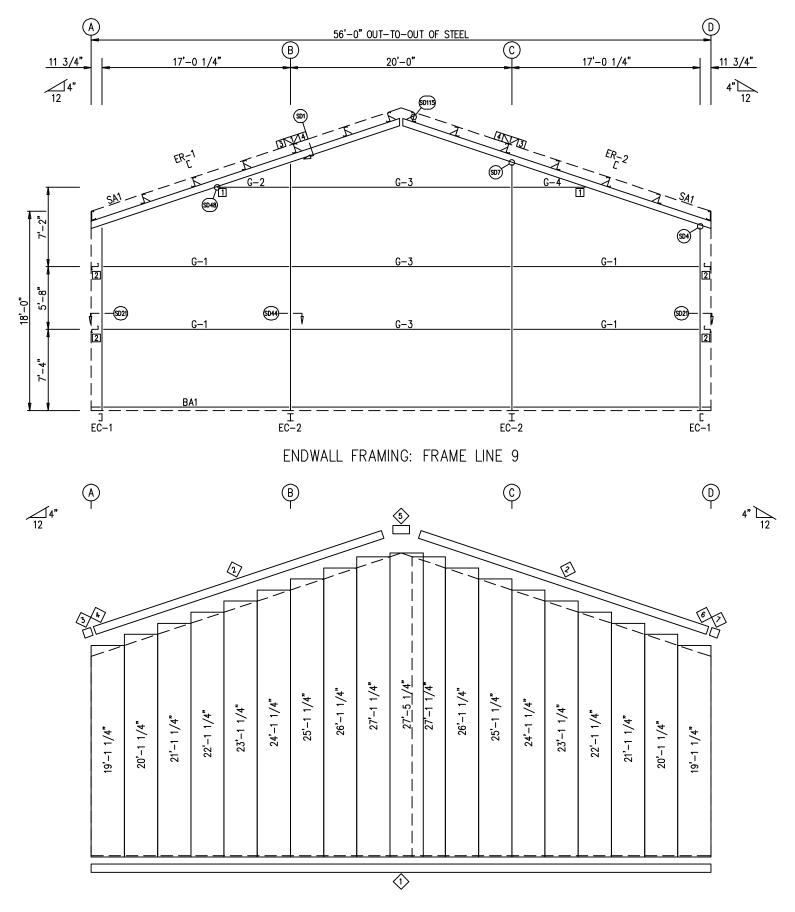






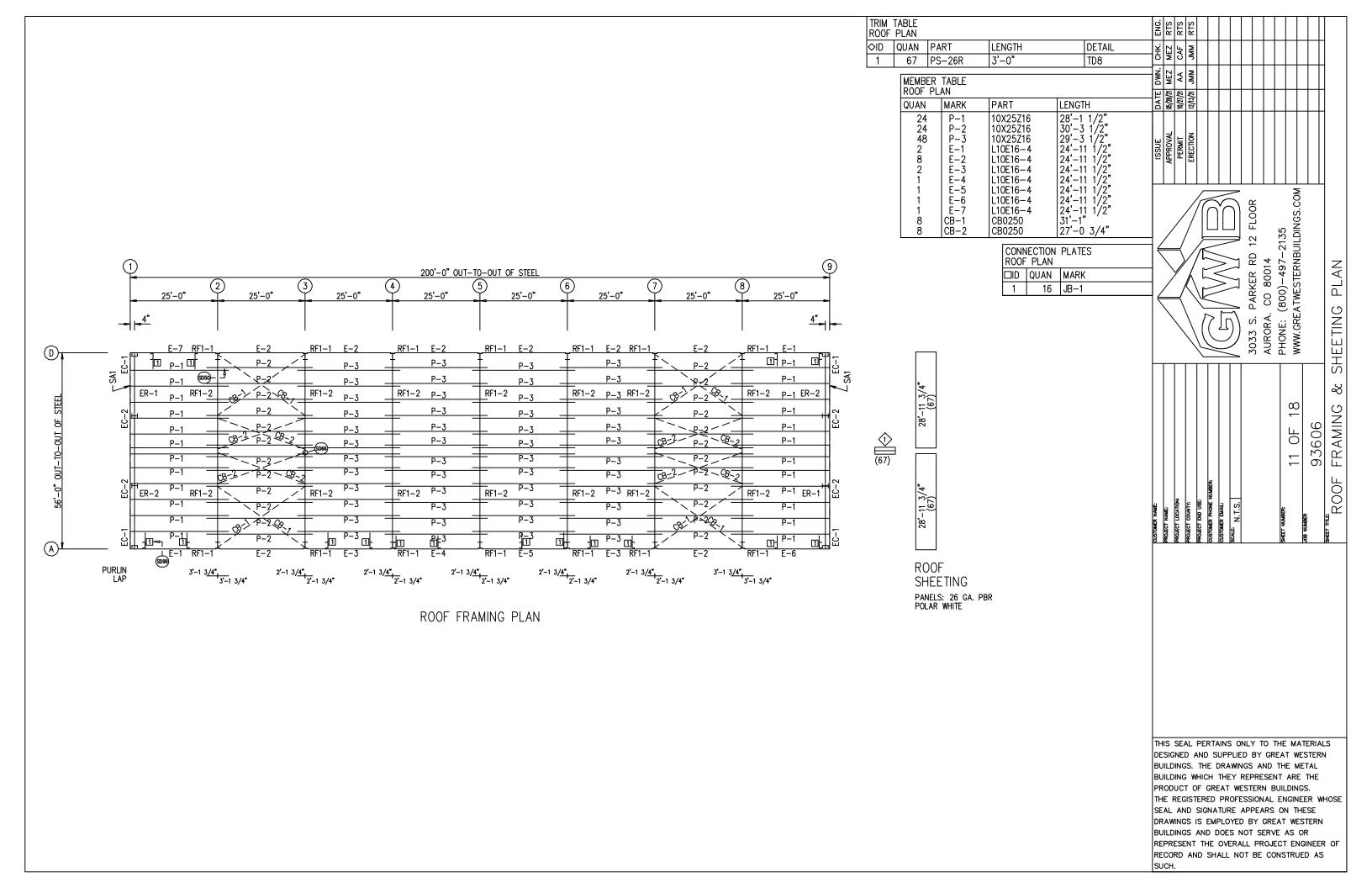
ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - LIGHT STONE

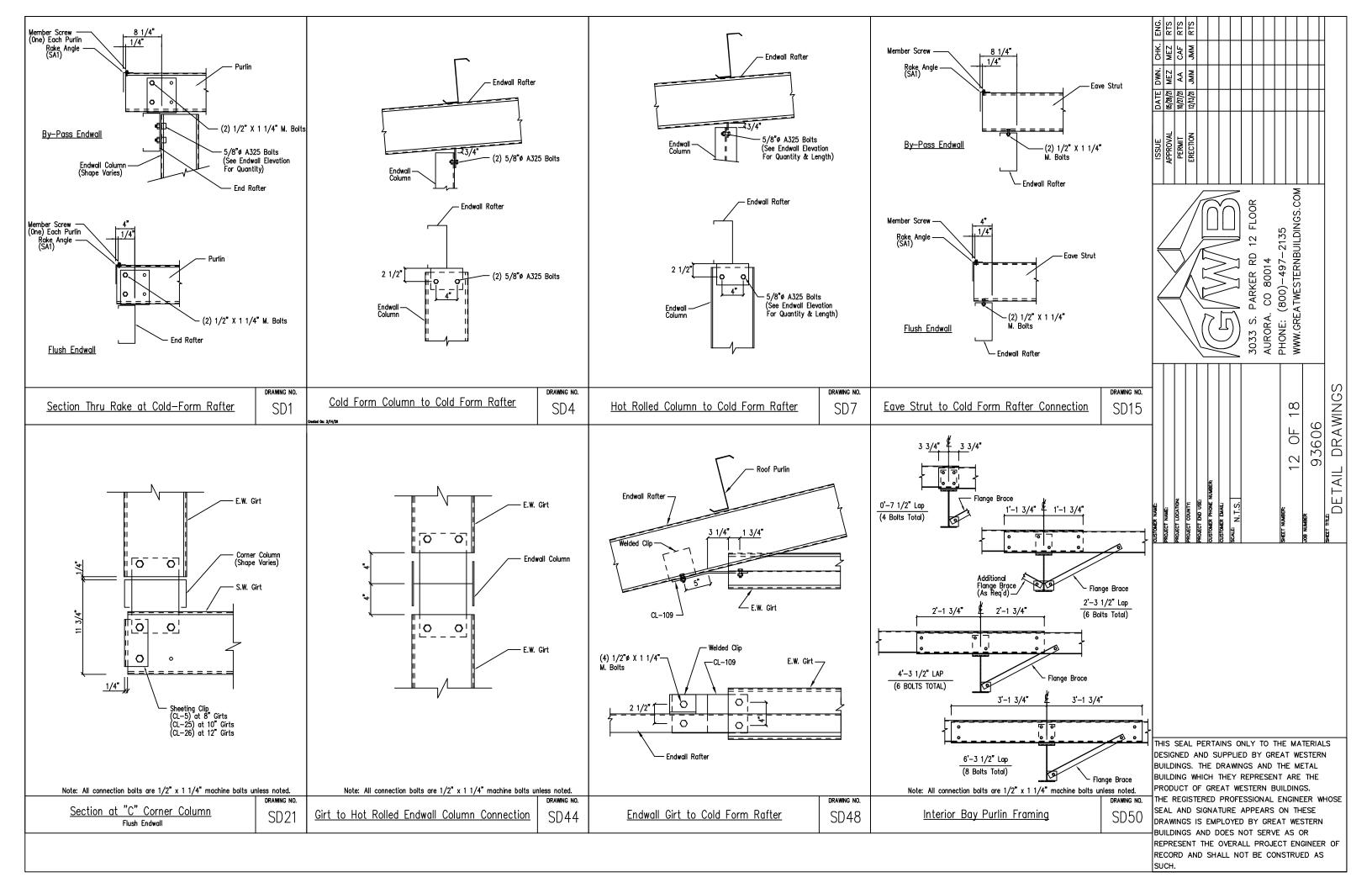
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1 2 3 4 5 6	4	FL-21 FL-21L	11'-2"		TD3	5 5	DWN.	MEZ										
5	1 1	FL-21L FL-328L FL-23	9, 1/2" 1'-4"		TD1		DATE	12/82/50	10/27/21	12/13/21								
6 7	1 1	FL-21R FL-328R	11'-2" 9 1/2"		TD8		ľ		Ī	Ī					Ħ	$\dagger \dagger$		
E	BOLT TAE	BLE					۱	JVAL	ļ	NOL								
F	RAME LI	NE 1	LOUANI	TVDE D	I A	LENGTH	ISSNE	APPROVAL	PER	ERECTION								
E	OCATION R-1/ER	-2	QUAN 4	TYPE D A325 5	/8" /8"	LENGTH 1 3/4" 1 1/2"												
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	1 1	ER-1 ER-2	10X35C12 10X35C12	29'-	-5 15/ -5 15/	/16" /16"			$\sqrt{}$		Ľ	\geq		ίκ ικ	2017	-497 TERN		
	4	G-1 G-2	8X25Z14 8X25Z16 8X25Z12	16'-	-5 13/ -1 7/8 -5 15/ -5 15/ -7 7/8 -3 5/8' -3 13/8	, "	∠	_	_/	$\langle \rangle$	\[\geq		PARKER RD	CO 80014	(800)-497-2135 ATWESTERNBUILDIN		N N
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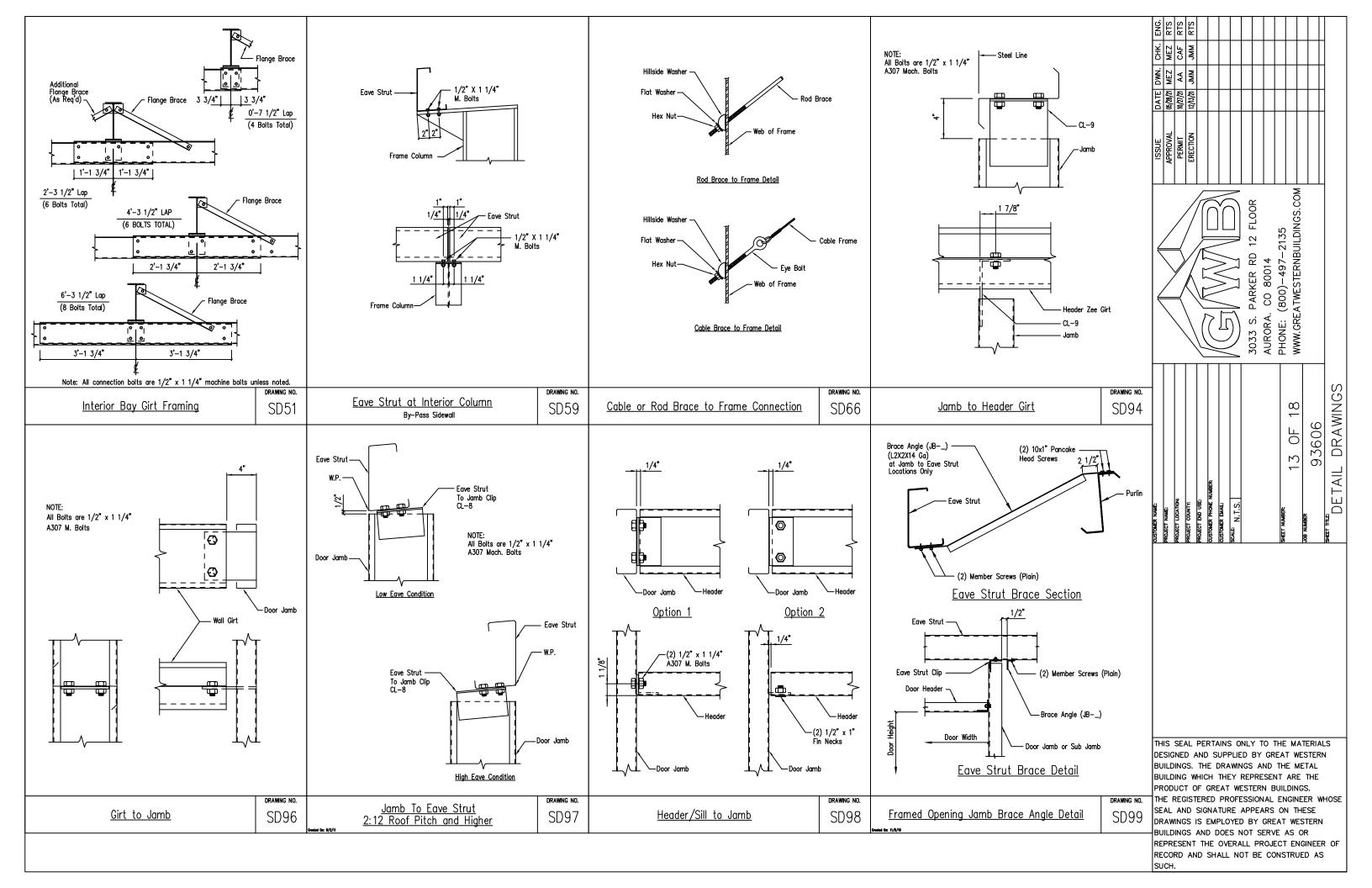


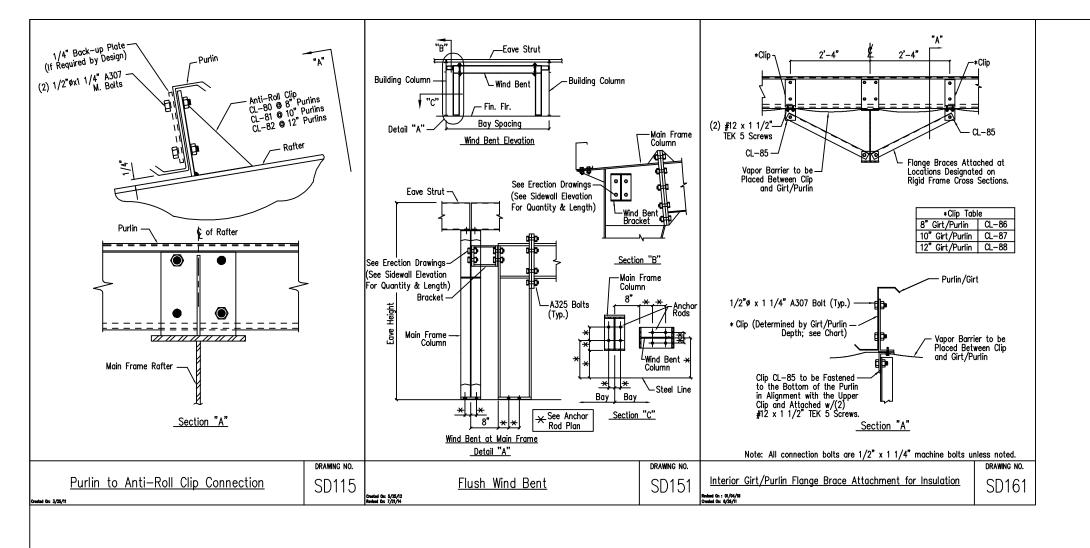
ENDWALL SHEETING & TRIM: FRAME LINE 9
PANELS: 26 GA. PBR - LIGHT STONE

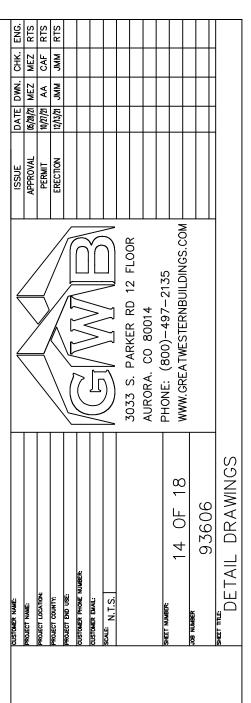
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BOLT TABLE FRAME LINE 9 LOCATION QUAN TYPE DIA LENGTH ER-1/ER-2	3 4	1 1	FL-21L FL-328L	11'-2" 9_1/2"		TD85 TD13					Н	+	+		H	\perp		
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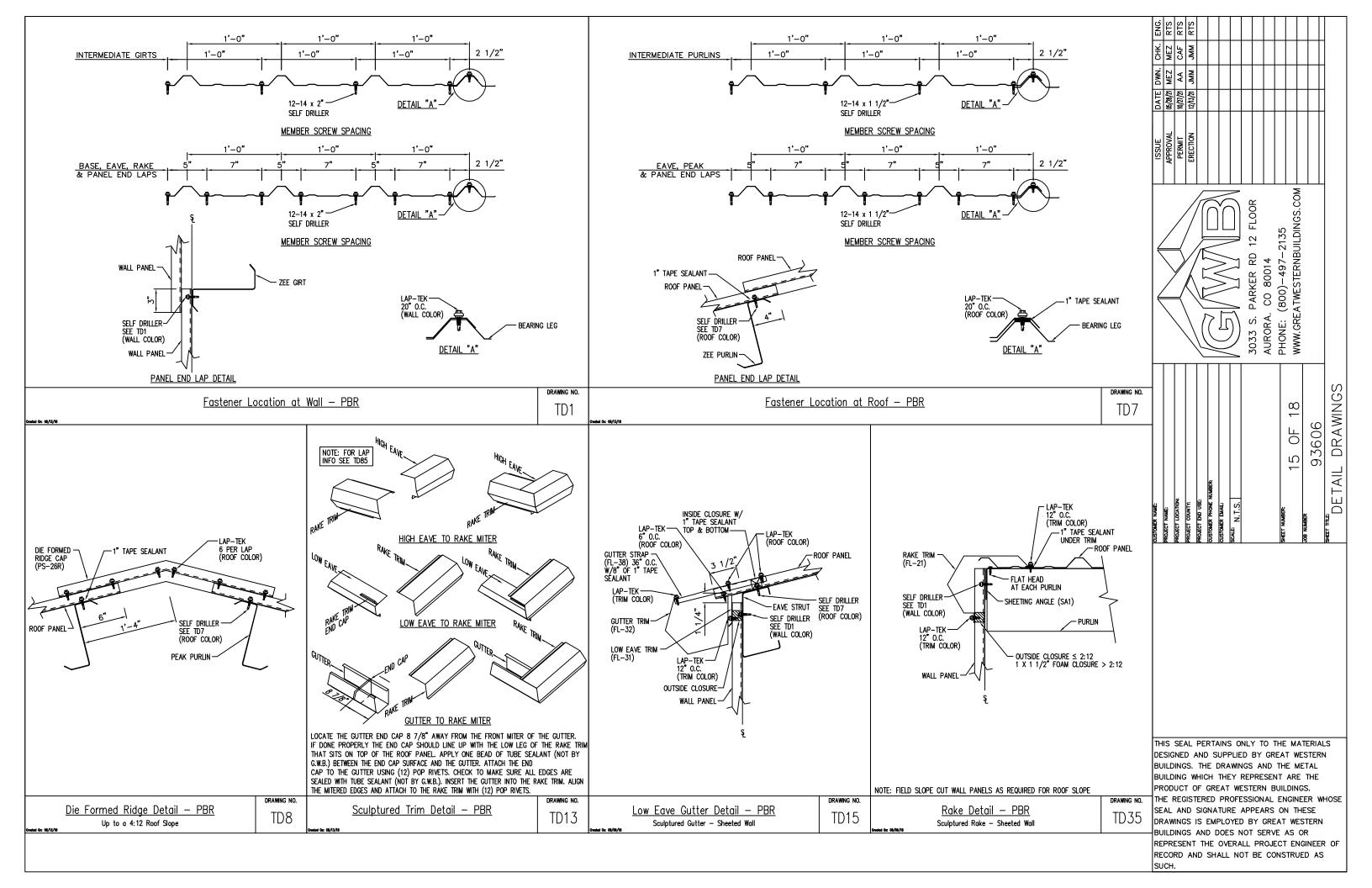


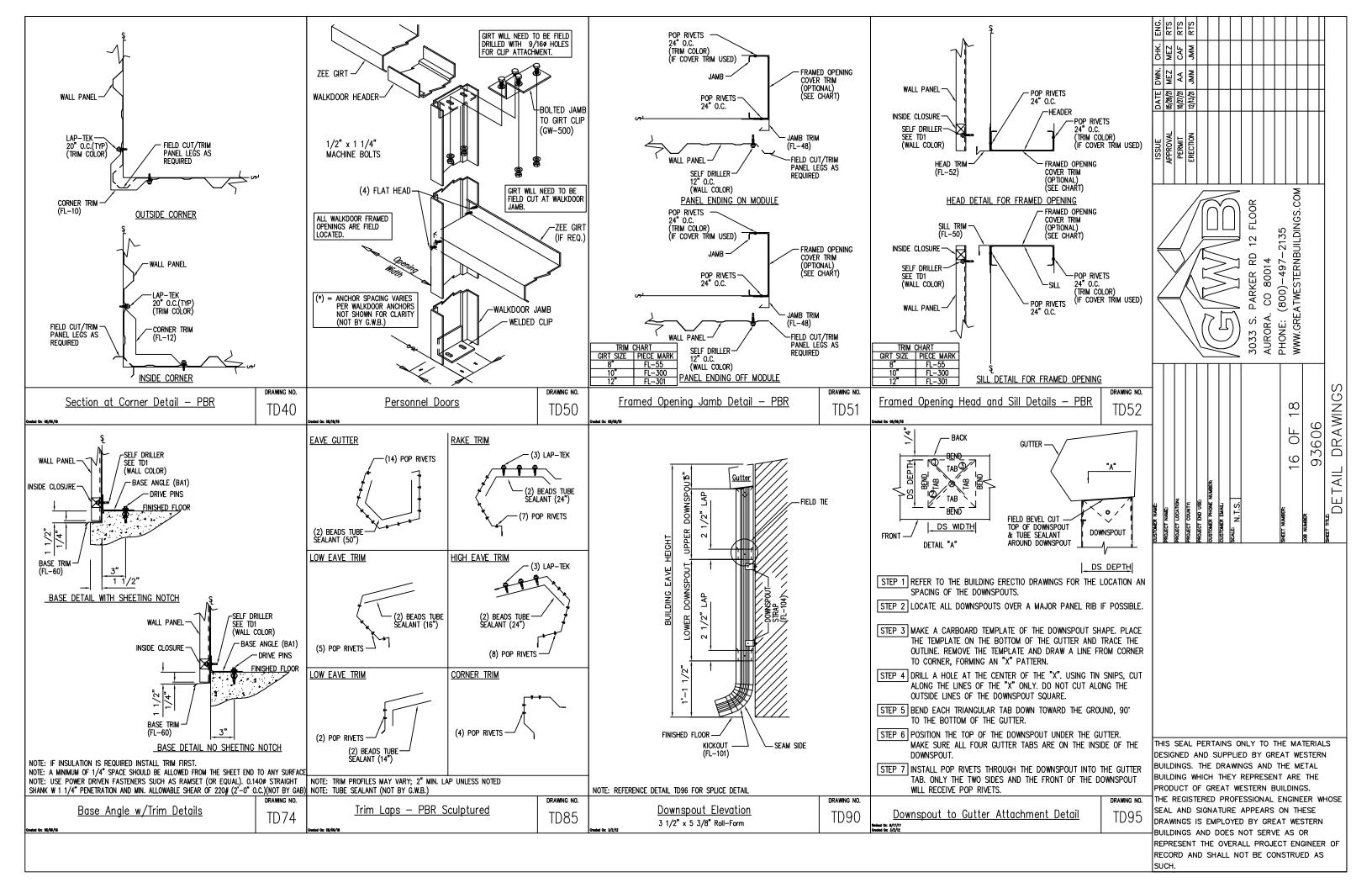


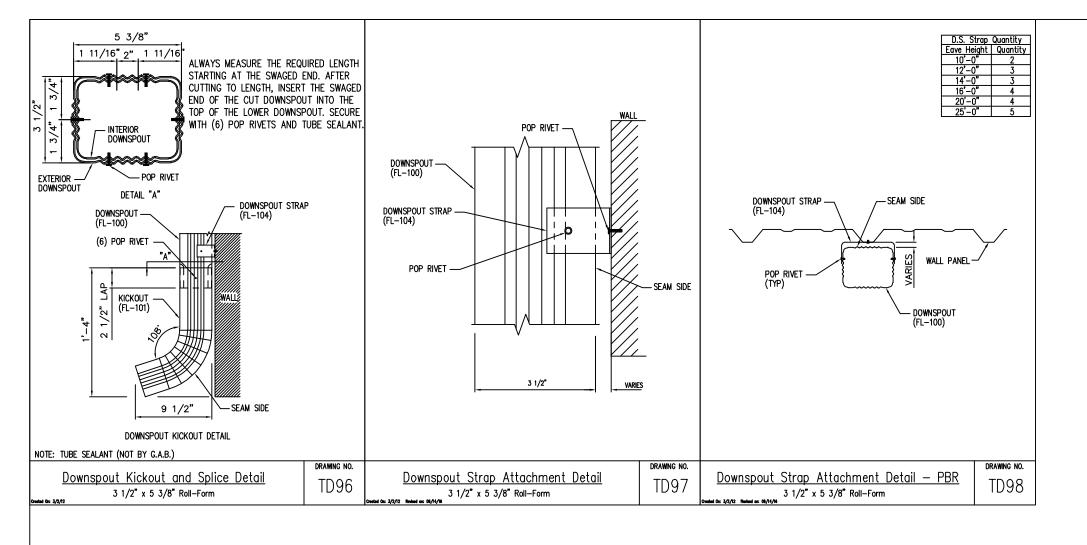




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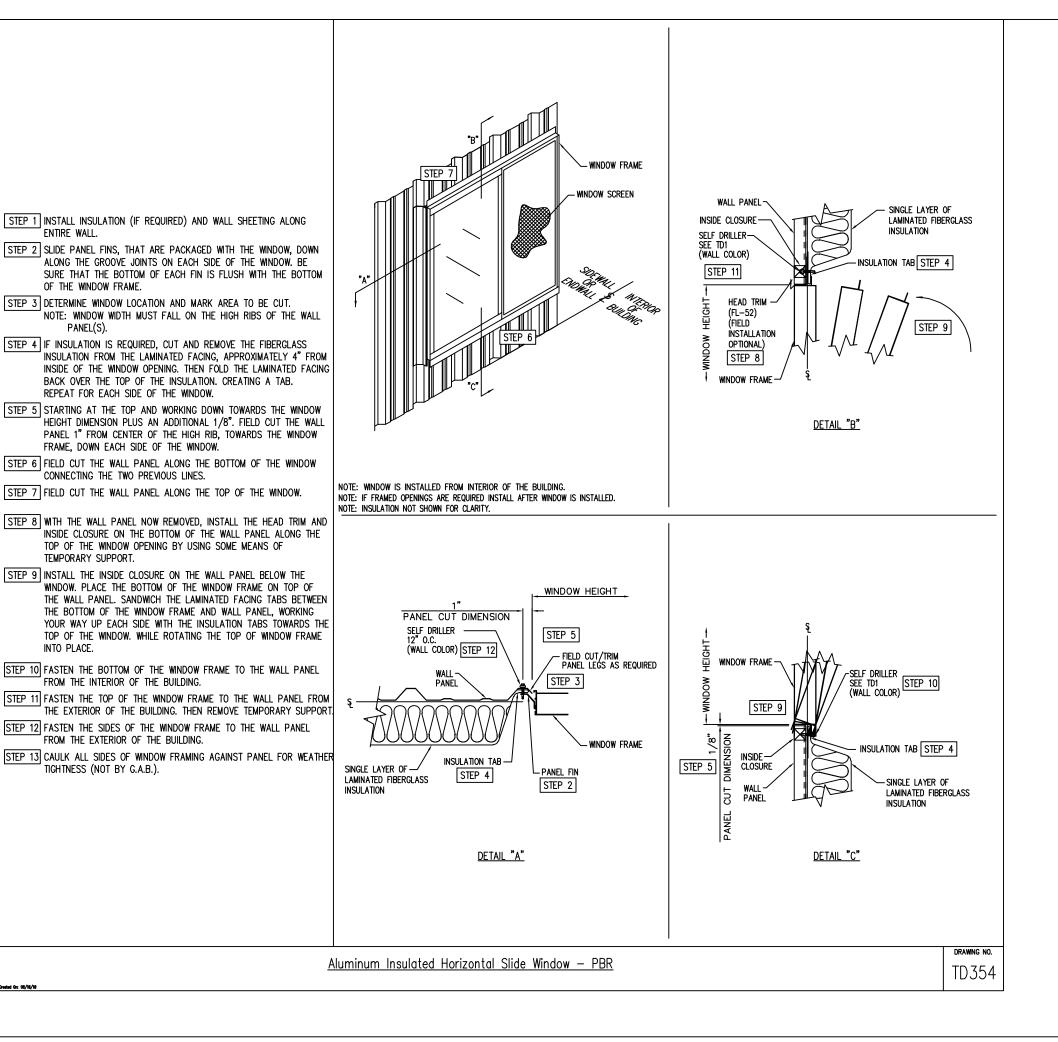






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CUSTOMER PHONE NUMBER:						
CUSTOMER EMAIL:						
SCALE: N.T.S.						
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	JUST S. PARNER NO 12 FLOOR					
	AURORA. CO 80014					
SHEET NUMBER:	HONE: (800)-497-2135					
17 OF 18	WWW CREATWESTERNIH DINGS COM					
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DETAIL DRAWINGS						

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ENTIRE WALL.

OF THE WINDOW FRAME.

TEMPORARY SUPPORT.

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WWW.GREATWE AURORA. DRAWING 3606 OF. $\frac{1}{\infty}$ တ်

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