

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

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1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	$F_y = 50 \text{ KSI}$
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	$F_V = 55 \text{ KSI}$
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	$F_{V} = 50, 80 \text{ KSI}$
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			=

MIN. TENSILE STRENGTH MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & & LESS) A307 A325-TYPE 1 Fu = 120 KSI Fu = 105 KSI HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

.4 GALVANIZED OR SPECIAL COATINGS:

.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT

A) ENDWALL RAFTER SPLICE - 5/8" x 0'-1 3/4" A325-N
B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2"ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AIG. "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE - UNDER ROOF PANELS & BASE OF WALL PANELS
OUTSIDE - BETWEEN ROOF PANELS & RIDGE CAP
- BETWEEN WALL PANELS & EAVE/GABLE TRIM

3 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISRITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISRITS. THE CORRECTION OF MINOR MISRITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.) METAL BUILDING SUPPLIER (HEREATER REFERRED TO AS M.B.S.).
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/FUND USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SÉCTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/
 END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND 2.5

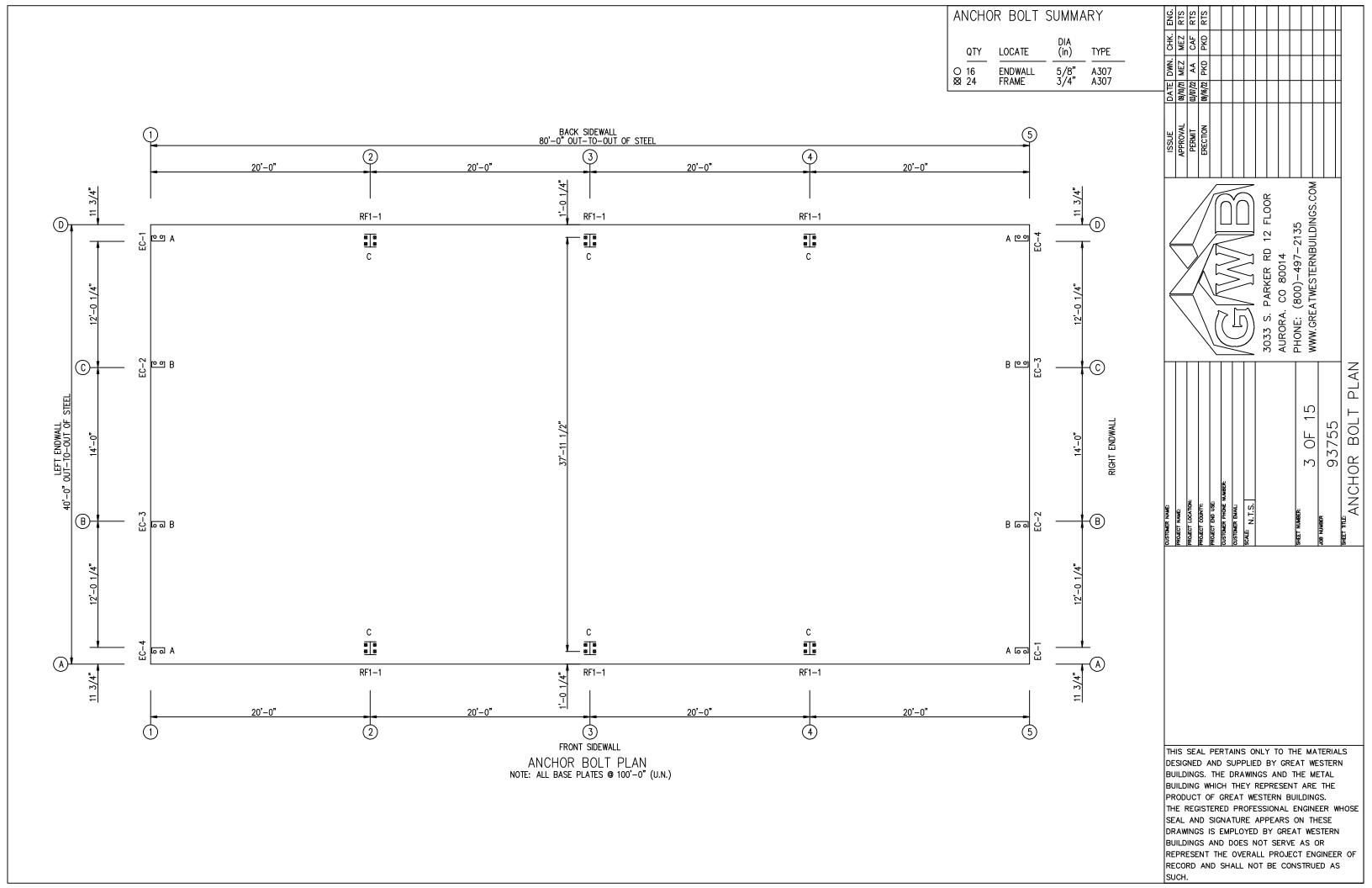
- 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST EDITION)
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DELIGING ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FIRMSHED BY M.B.S. STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)
- METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE.

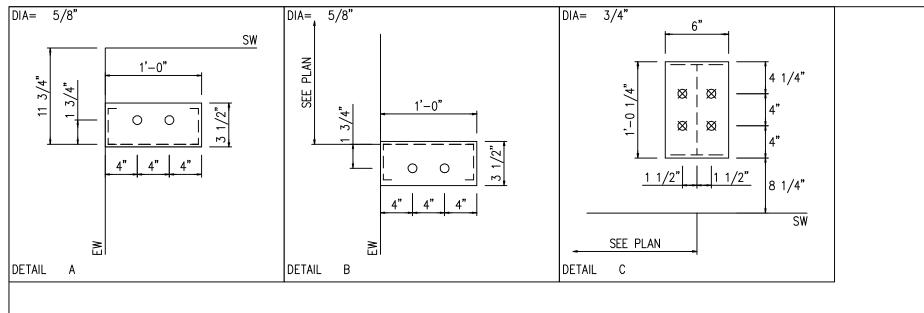
 UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELT THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MEMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWNIGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with LEAD or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wiring, or tubing onto galvalume should be
- 2.13 <u>SAFETY COMMITMENT</u> METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY, MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: DESIGN LOADS: DESIGN CODE / WIND CODE : IBC-18 OCCUPANCY / RISK CATEGORY : II-Normal ENCLOSURE : Closed ROOF DEAD LOAD (D) (PSF) : 2.00 ROOF COLLATERAL LOAD (C) (PSF) :1.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) : 115.00 WIND EXPOSURE CATEGORY : C INTERNAL PRESSURE COEFFICIENT, (GCpi) :0.18/-0.18 WALL PANEL DESIGN WIND PRESSURE (PSF) : 31.47/-34.13 WIND ENCLOSURE CLASSIFICATION : Closed PRIMARY FRAMING (PSF) : 20.00 ATWESTERNBUILDINGS. TRIB. AREA REDUCTION : No SECONDARY FRAMING (PSF) : 20.00 7 SNOW LOAD 2 GROUND SNOW LOAD, (Pg) (PSF) : 5.00 80014 ROOF SNOW LOAD, (Pf) (PSF) : 5.00 SNOW EXPOSURE FACTOR, (Ce) :1.00 PARKER SNOW IMPORTANCE FACTOR, (Is) :1.00 CO 8 (800)-THERMAL FACTOR, (Ct) : 1.00 SEISMIC LOAD Ī SEISMIC IMPORTANCE FACTOR, (le) :1.00 AURORA. PHONE: (WWW.GREA ι SITE CLASSIFICATION : d RSHE 3033 SPECTRAL RESPONSE ACCELERATION :Ss = 0.076 :S1 = 0.048SPECTRAL RESPONSE COEFFICIENTS :Sd1 = 0.075: Sds = 0.081SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM : STEEL SYSTEM NOT SPECIFICALLY COVE DETAILED FOR RESISTANCE : RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) TOTAL DESIGN BASE SHEAR, (V) (KIPS) :LONGITUDINAL = 0.57 Ω 0 : TRANSVERSE = 0.54K Ω RESPONSE MODIFICATION FACTORS, (R) :RIGID FRAMES = 3.00 $\Omega = 3.00$ 2 0 37 SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.0270BUILDING α Ó ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS N.T.S. THIS BUILDING IS DESIGNED USING PANEL SHEAR BRACING. NO ADDITIONAL OPENINGS ARE TO BE ADDED WITHOUT CONSULTING THE ORIGINAL DESIGN ENGINEER OR A LOCAL ENGINEERING PROFESSIONAL **BUILDING DESCRIPTION:** WIDTH (FT) : 40.00 LENGTH (FT) : 80.00 EAVE HEIGHT AT BSW (FT): 20.00 EAVE HEIGHT AT FSW (FT): 20.00 ROOF SLOPE AT BSW : 4.0:12 ROOF SLOPE AT FSW : 4.0:12 BAY SPACING (FT) :4 AT 20 COVERING AND TRIMS: **ROOF PANELS & TRIMS** PANEL TYPE :26 GA. PBR PANEL COLOR : GALVALUME TRIM COLORS : COAL BLACK GABLE/EAVE EAVE GUTTER :N/A WALL PANELS & TRIMS PANEL TYPE :26 GA. PBR PANEL COLOR : KOKO BROWN THIS SEAL PERTAINS ONLY TO THE MATERIALS TRIM COLORS DESIGNED AND SUPPLIED BY GREAT WESTERN CORNER : COAL BLACK BUILDINGS. THE DRAWINGS AND THE METAL FRAMED OPENING : COAL BLACK BUILDING WHICH THEY REPRESENT ARE THE DOWNSPOUTS :N/A PRODUCT OF GREAT WESTERN BUILDINGS. : KOKO BROWN BASE THE REGISTERED PROFESSIONAL ENGINEER WHOSE INSULATION SEAL AND SIGNATURE APPEARS ON THESE ROOF INSULATION : N / A DRAWINGS IS EMPLOYED BY GREAT WESTERN WALL INSULATION :N/A BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH



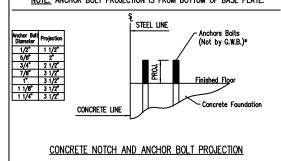


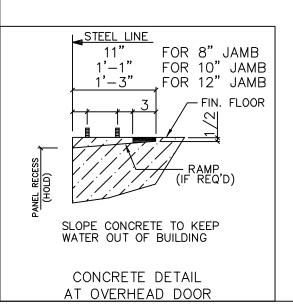
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR

FIELD WORK. ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED

SHEAR AND TENSION. DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

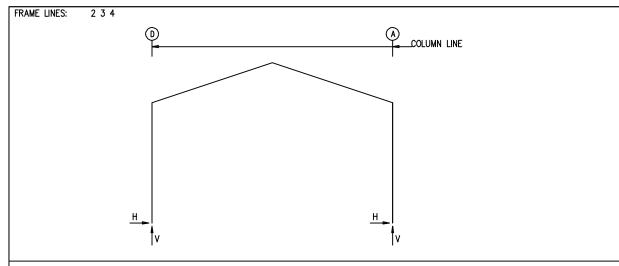
NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





3033 S. PARKER RD 12 FLOOR
AURORA. CO 80014
PHONE: (800)-497-2135
WWW.GREATWESTERNBUILDINGS.COM DETAILS BOLT 93755 OF ANCHOR 4 THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF

RECORD AND SHALL NOT BE CONSTRUED AS



RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BAS	E PLATE	S				
Frm Line		Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolt QTY	(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	3 1	3.4 2.3	5.6 9.9	6 4	-3.9 -3.2	-3.3 -5.6	4	0.750	6.000	12.25	0.375	0.0
2*	Α	7 1	3.9 -2.3	-3.3 9.9	2 5	-3.4 3.2	5.6 -5.6	4	0.750	6.000	12.25	0.375	0.0
2*	FRAME li	nes:	2 3 4										

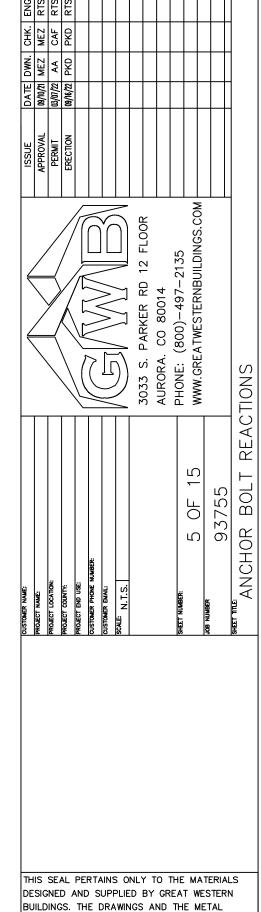
2*	FRAME lines: 2 3 4		
OTES	FOR REACTIONS		
	ding reactions are based on		
the	following building data:	40.00	
	Width (ft)	= 40.00	
	Length (ft)	= 80.00	
	Roof Slope (rise/12)	= 20.00/20.00 = 4.0:12/4.0:12	
	Eave Height (ft) Roof Slope (rise/12) Dead Load (psf)	= 2.00	
	Collateral Load (pst)	= 1.00	
	Live Load (psf)	= 20.00	
	Snow Load (psf)	= 5.00	
	Ultimate Wind Speed (mph) Wind Code	= 115.00 = IBC-18	
	Exposure	= C	
	Closed/Open	= Closed	
	Importance Wind	= 1.00	
	Importance Seismic	= 1.00	
	Seismic Zone Seismic Coeff (Fa*Ss)	= B = 0.12	
	Seismic Coen (ru-Ss)	- 0.12	
ID	Description		
1	Dead+Collateral+Live		
2	Dead+Collateral+0.75Live+0.45Wind_l		
1 2 3 4 5 6 7	Dead+Collateral+0.75Live+0.45Wind_f 0.6Dead+0.6Wind_Left1	Righti	
5	0.6Dead+0.6Wind_Right1		
6	0.6Dead+0.6Wind_Left2		
7	0.6Dead+0.6Wind_Right2		
8	0.6Dead+0.6Wind_Suction+0.6Wind_L	.ong1L	
9 10	U.bDedd+U.bWind_Pressure+U.bWind_	LongiL	
11		ection	
12	0.6Dead+0.6Wind_Pressure+0.6Wind_	Long2L	
13	0.6Dead+0.6Wind_Suction+0.6Wind_L	ongŽL	

ANCHO	OR BOLT	SUMMA	4RY
QTY	LOCATE	DIA (in)	TYPE
O 16 Ø 24	ENDWALL FRAME	5/8" 3/4"	A307 A307

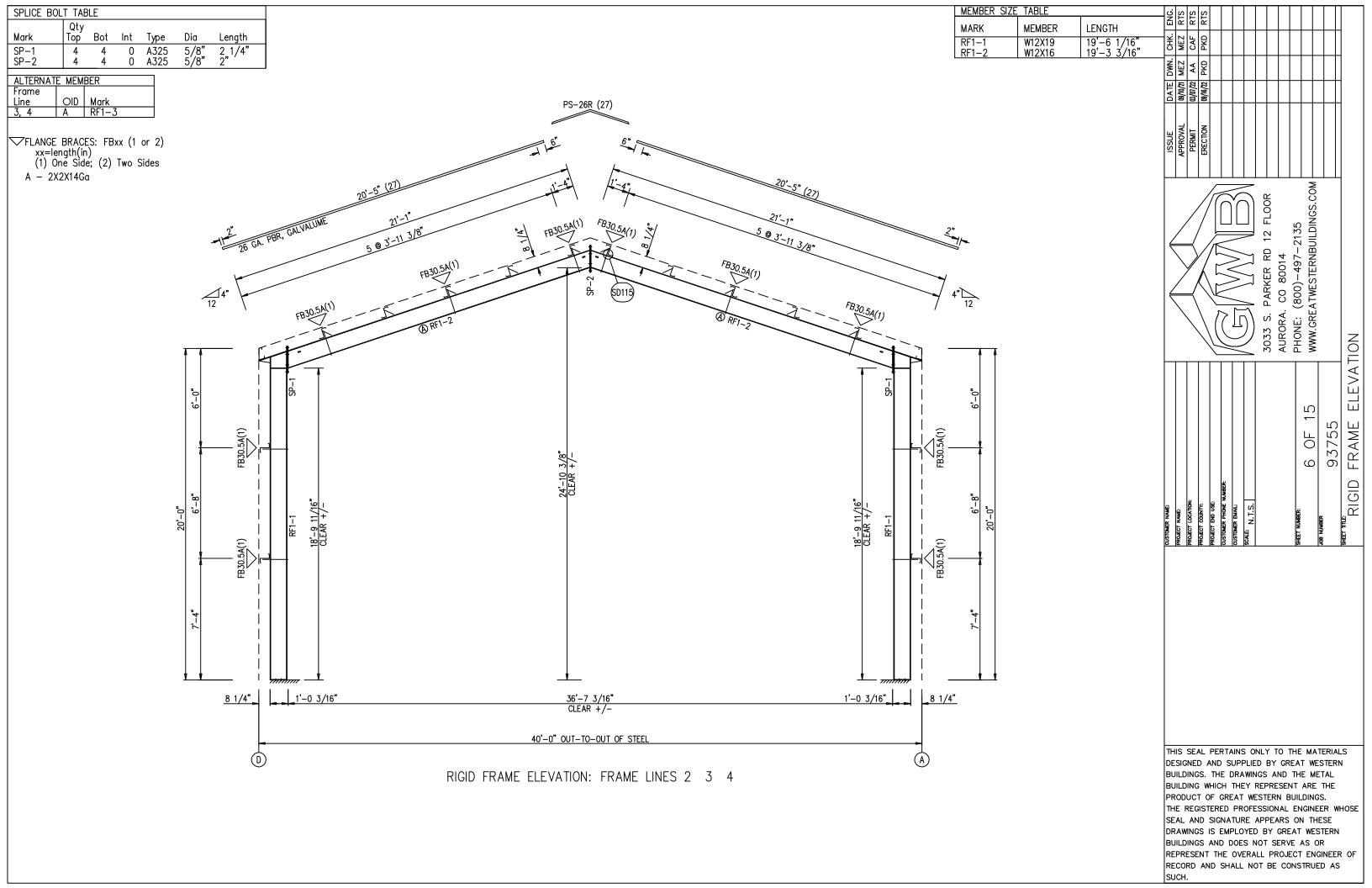
BUILDING BRACING I	REACTIONS		
Wall Col Hor: L_EW	± Reactions(k) —Wind — Seismic — z Vert Horz Vert	Panel_Shear (lb/ft) Wind Seis 74 7 58 4 69 7 58 4	_

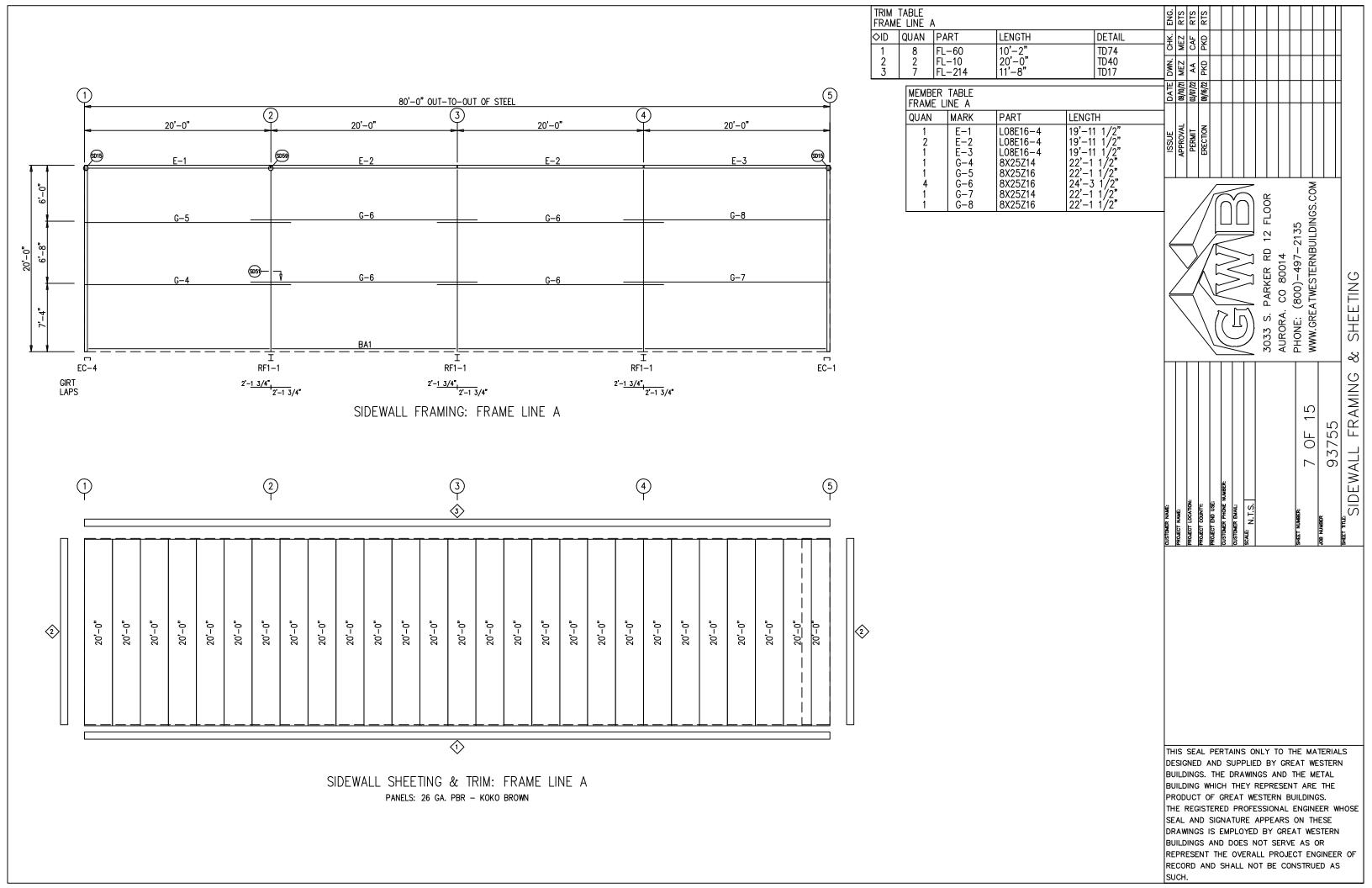
RIGI	FRAN	νE:	BAS	IC COLUM	IN REACT	IONS (k)							
FRAME Line 2* 2*	Column Line D A	Horiz 0.3 -0.3	Dead—— Vert 1.5 1.5	Collo Horiz 0.1 -0.1	oteral— Vert 0.4 0.4	 Horiz 1.9 -1.9	Live Vert 8.0 8.0	Horiz 0.5 -0.5	-Snow Vert 2.0 2.0	Wind Horiz -5.7 -3.5	I_Left1- Vert -10.9 -5.2	-Wind_ Horiz 3.5 5.7	Right1- Vert -5.2 -10.9
FRAME Line 2* 2*	Column Line D A	Wind Horiz -6.8 -2.3	_Left2- Vert -7.0 -1.4	-Wind_ Horiz 2.3 6.8	Right2- Vert -1.4 -7.0	Wind Horiz 2.4 -1.3	_Long1- Vert -7.8 -7.4	Wind Horiz 1.3 -2.4	l_Long2- Vert -7.4 -7.8	-Seism Horiz -0.1 -0.1	ic_Left Vert -0.1 0.1	Seismic Horiz 0.1 0.1	:_Right Vert 0.1 -0.1
FRAME Line 2* 2*	Column Line D A	F1UNB_ Horiz 0.4 -0.4	SL_L- Vert 1.9 1.2	F1UNB_ Horiz 0.4 -0.4	SL_R- Vert 1.2 1.9								
2*	FRAME lin	nes:	2 3 4	1									

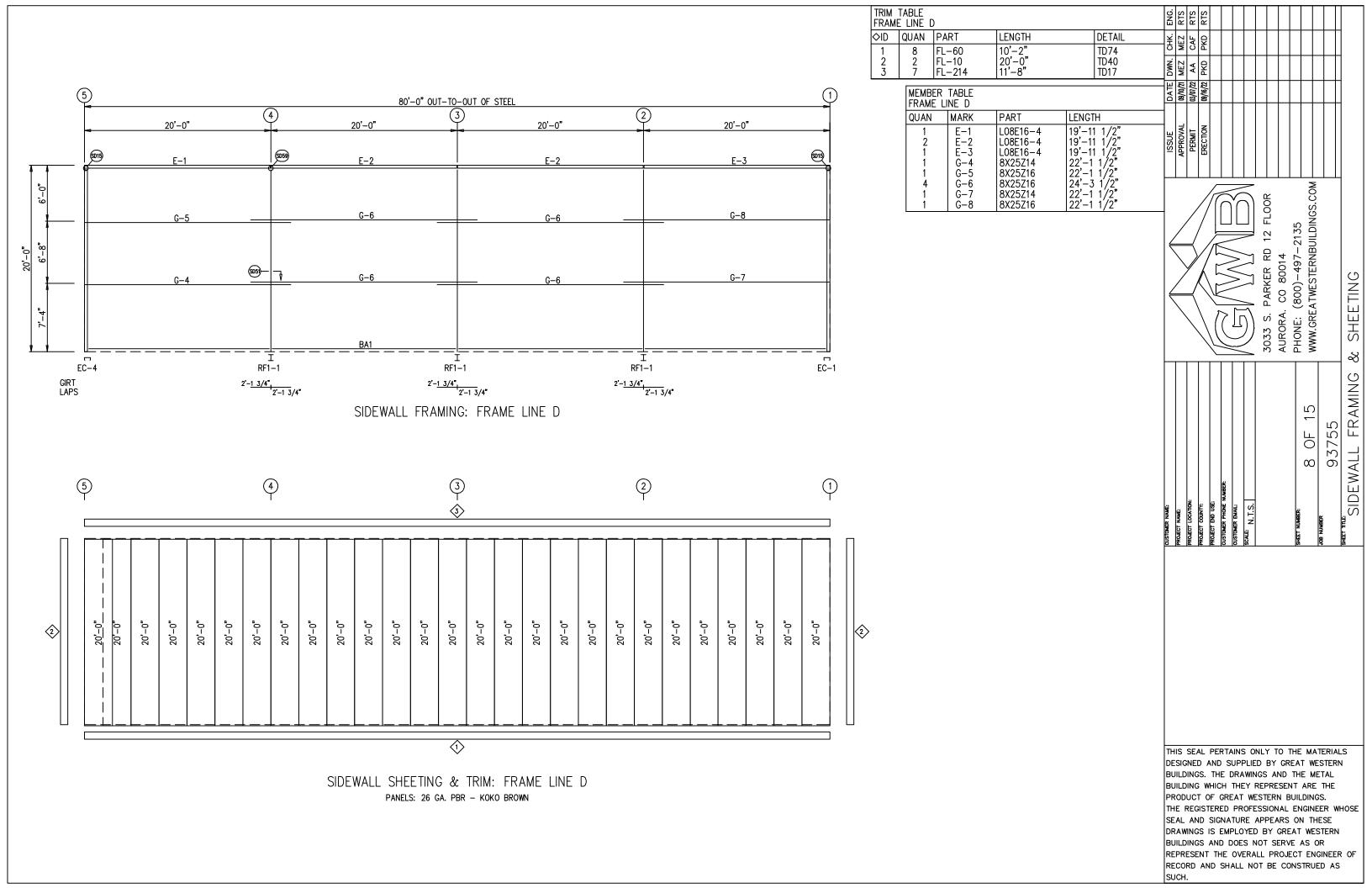
∠+ 2*	A	-0.4 -0.4	. 1			.2 .9										
2*	FRAME	lines:	2	3 4												
END)WALL	COLU	MN:		BASIC	COLUMN	REACTIONS	S (k)								
Frm Line 1 1 1	Col	Dead Vert 0.2 0.5 0.5	!	Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.1 3.0 3.0 1.1	Sno Ver 0.3 0.8 0.8 0.3	ow L t \ 	Vind eft1 /ert 1.3 4.5 3.3	Wind Right1 Vert -1.0 -3.3 -4.5 -1.3	Wind Left2 Vert -0.4 -3.6 -2.3 0.0	Wii Rig Ve 0.0 -2. -3. -0.	ght2 ert 0 - 3 - 6 -	Wind Press Horz -1.6 -3.8 -3.8	Wind Suct Horz 1.9 4.2 4.2 1.9	Wind Long1 Vert -2.2 -3.7 -1.9 -1.8	Wind Long2 Vert -1.8 -1.9 -3.7 -2.2
Frm Line 1 1 1		Seis Left Vert 0.0 0.0 0.0	 	Seis Right Vert 0.0 0.0 0.0 0.0	E1UNB Horz 0.0 0.0 0.0 0.0	_SL_L- Vert 0.2 0.9 0.4 0.1	E1UNE Horz 0.0 0.0 0.0 0.0	3_SL_R- Vert 0.1 0.4 0.9 0.2								
Frm Line 5 5 5 5		Dead Vert 0.2 0.5 0.5	, (Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.1 3.0 3.0 1.1	Snc Ver 0.3 0.8 0.8	ow L t \ 	Vind eft1 /ert 1.3 4.5 3.3	Wind Right1 Vert -1.0 -3.3 -4.5 -1.3	Wind Left2 Vert -0.4 -3.6 -2.3 0.0	Wii Riq Ve 0.0 -2. -3. -0.	ght2 rt 0 - 3 - 6 -	Wind Press Horz -1.6 -3.8 -3.8	Wind Suct Horz 1.9 4.2 4.2 1.9	Wind Long1 Vert -2.2 -3.7 -1.9 -1.8	Wind Long2 Vert -1.8 -1.9 -3.7 -2.2
Frm Line 5 5 5 5	Col Line A B C D	Seis Left Vert 0.0 0.0 0.0	 	Seis Right Vert 0.0 0.0 0.0 0.0	E2UNB Horz 0.0 0.0 0.0 0.0	SL_L- Vert 0.2 0.9 0.4 0.1	E2UNE Horz 0.0 0.0 0.0 0.0	3_SL_R- Vert 0.1 0.4 0.9 0.2								
END)WALL	COLU	MN:		MAXIMU	M REACTI	ONS, ANC	HOR BOL	TS, & BA	ISE PLATES						
	rm Co ine Lir		Load Id	Hmax H	lumn_Red V Vmax	ctions(k Load Id) Hmin H	V Vmin	— Bo QTY	It(in) DIA	Base Width	e_Plate(in Length	r) Thick	Grout (in)		
1		D	8	1.1	-1.2	— — 9 8	-1.0	-1.2 -1.2		0.625	3.500	12.00	0.250	0.0	•	
1	(С	1 10 1	0.0 2.5 0.0	1.3 -2.4 3.7		1.1 -2.3 2.5	-1.2 -1.9 -2.4	2	0.625	3.500	12.00	0.250	0.0		
1	ŧ	В	11 1	2.5 0.0	-2.4 3.7	12 11	-2.3 2.5	-2.4 -1.9 -2.4	2	0.625	3.500	12.00	0.250	0.0		
1	,	A	13 1	1.1 0.0	-1.2 1.3	12 13	-1.0 1.1	-1.2 -1.2	2	0.625	3.500	12.00	0.250	0.0		
5	5	A	8 1	1.1 0.0	-1.2 1.3	9	-1.0 1.1	-1.2 -1.2	2	0.625	3.500	12.00	0.250	0.0		
5	5 1	В	10 1	2.5 0.0	-2.4 3.7	9 10	-2.3 2.5	-1.9 -2.4	2	0.625	3.500	12.00	0.250	0.0		
5	5 (С	11 1	2.5 0.0	-2.4 3.7		-2.3 2.5	-1.9 -2.4	2	0.625	3.500	12.00	0.250	0.0		
5	5 (D	13 1	1.1 0.0	-1.2 1.3	12 13	-1.0 1.1	−1.2 −1.2	2	0.625	3.500	12.00	0.250	0.0		

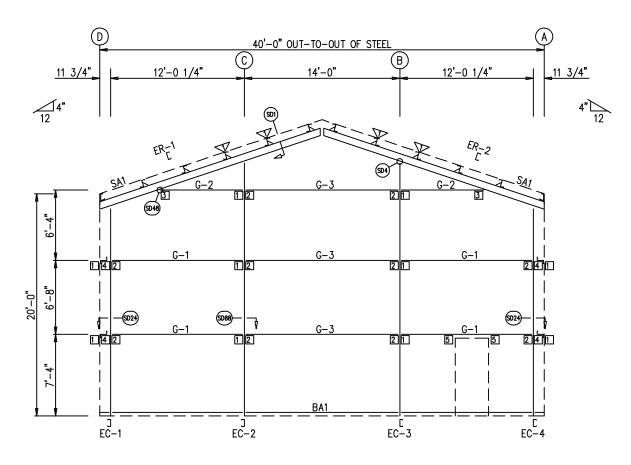


BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS

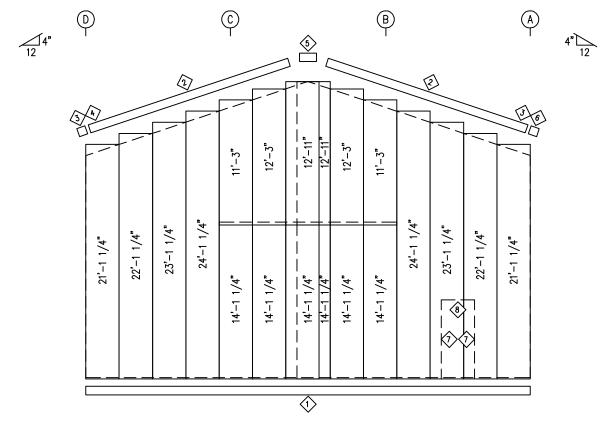








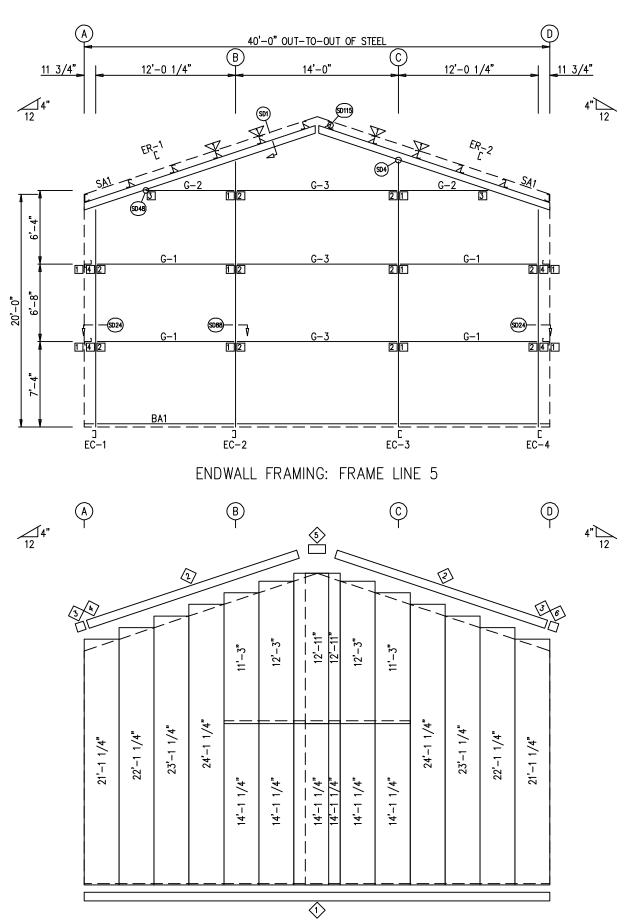
ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - KOKO BROWN

TRIM	TABLE ME LINE	1			ENG.	RTS	RTS	2							
◇ID	QUAN 4	PART FL-60	LENGTH	DETAIL TD74	_¥	-	CAF	+					$\dagger \dagger$		
2 3	4 2	FL-00 FL-15 FL-601E	10'-2" 11'-2" 8 1/8" 5 5/8" 1'-4" 5 5/8" 7'-2" 3'-4"	TD36 TD85			¥ 2								
4	1 1	FL-600L FL-17	5 5/8" 1'-4"	TD13	DATE DWN.	12/01/60	03/07/22		$^{+}$		+	\Box		\parallel	
6 7	1 2	FL-600R FL-48	5, 5/8" 7,-2"	TD13 TD51		8	2 2	3	\dagger		\dagger	\Box		\parallel	
8	1 BOLT TA	FL=52	3'-4"	TD52	ISSUE	APPROVAL	PERMIT								
	FRAME L	JNE 1	OHAN TVD	E DIA LIENOTII	<u>S</u>	APP									
	LOCATION ER-1/EF	N R-2 S/RAFTER	QUAN TYP 4 A32 2 A32			<u> </u>		<u> </u>		\vdash			⊥⊥ ∑		
		SER TABLE	2 A32	25 5/8 1 1/2				///	γ		FLOOR		PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM		
	FRAM	IE LINE 1	PART	LENGTH							12 FL	!	135 ILDIN		
	1 1	EC-1 EC-2	12x25C16	18'-10 1/16"	\dashv			/⊏	\leq	<u></u>	В. У	4 1	(800)—497—2135 ATWESTERNBUILDIN		
		EC-2 EC-3 EC-4	12X35C12 12X35C12 12x25C16 8x25C14 8x25C14 8X25Z16 8X25Z16 8X25Z16	18'-10 1/16" 22'-10 1/8" 22'-10 1/8" 18'-10 1/16" 21'-0 3/4" 21'-0 3/4" 11'-7 7/8" 7'-1 1/4" 13'-11 1/2"			¶ [\	\leq		PARKER	CO 80014)-49 :STEF		9
		ER-1 ER-2	8x25C14 8x25C14	21'-0 3/4" 21'-0 3/4"	┥	<u></u>	\prec $$		\leq	<u>></u>	PAR	8	(800 ATWE		SHEETING
	4 2 3	G-1 G-2 G-3	8X25Z16 8X25Z16	11'-7 7/8" 7'-1 1/4"		\	//	((][`)	3 S.	AURORA.	PHONE: WWW.GRE		
	3	G-3	[8X25Z16 CONNECTIO				\	17	\leq	/ _	3033	AUR	M W		
			FRAME LINE	<u>E 1</u>											3 &
			1 10	CL-103											FRAMING
			2 10 3 2 4 4 5 2	CL-100 CL-109E CL-5									15	5	RAN
				CL-200									OF	375	
			FRAN	IGE BRACE TABLE ME LINE 1									တ	9	DWALL
				QUAN MARK 4 FB29.3				- GEORGE							M QI
					R NAME:	NAME:	PROJECT LOCATION	PROJECT END USE	CUSTOMER EMAIL:	N.T.S.			UMBER:	8	
					CUSTOMER NAME	PROJECT NAME	PROJECT PROJECT	PROJECT	CUSTOME	SCALE			sheet number	JOB NUMBER	SHEET 1
													HE MA		
					Вι	JILD	INGS	. THE	DRA	WIN	GS A	ND 1	AT WE	TAL	1
					PF	ROD	UCT	OF G	REAT	WE	STER	N BL	T ARE	S.	
					SE	EAL	AND	SIGN	ATUF	RE A	APPE/	ARS	ENGINE ON THI	ESE	HOSE
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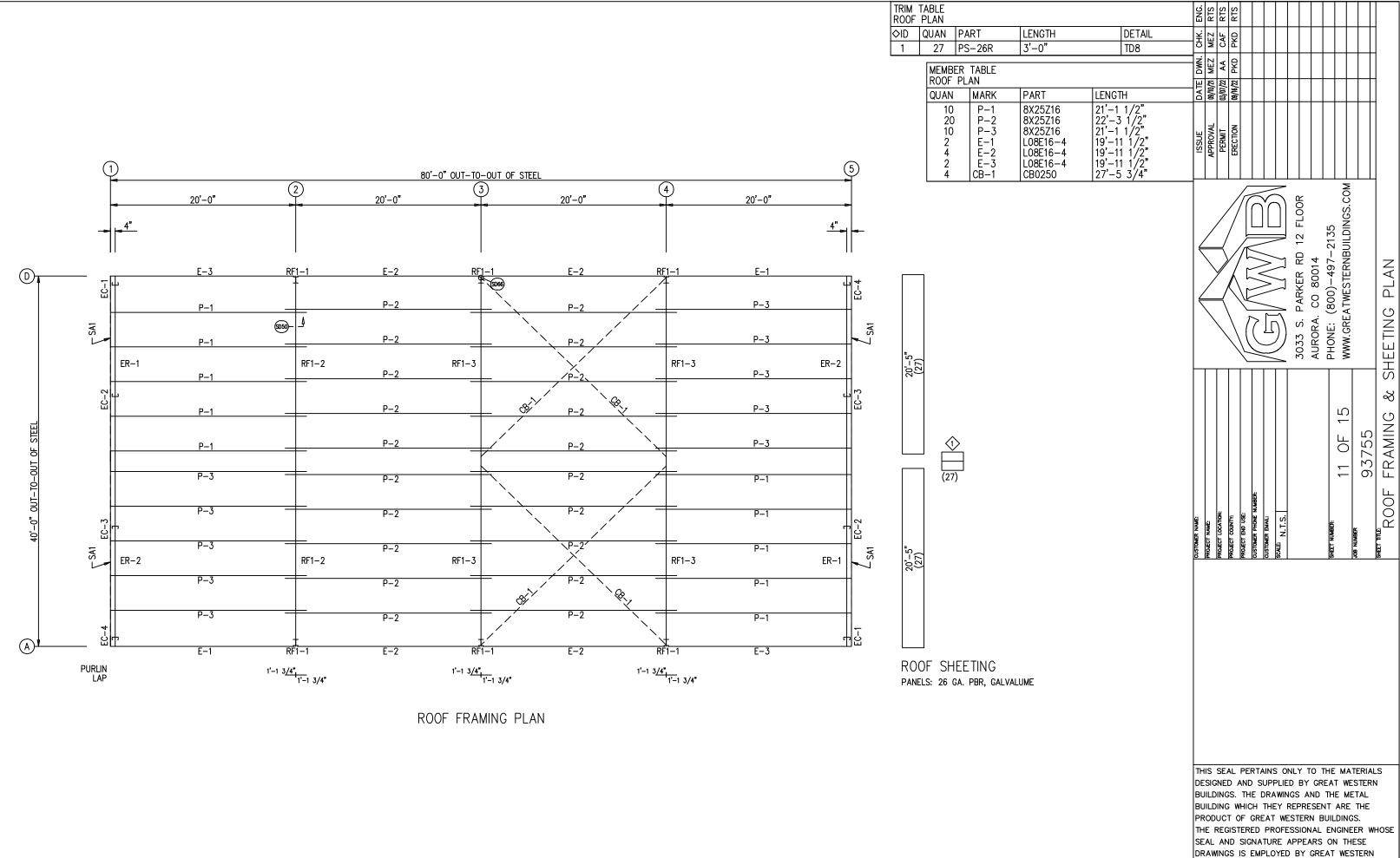
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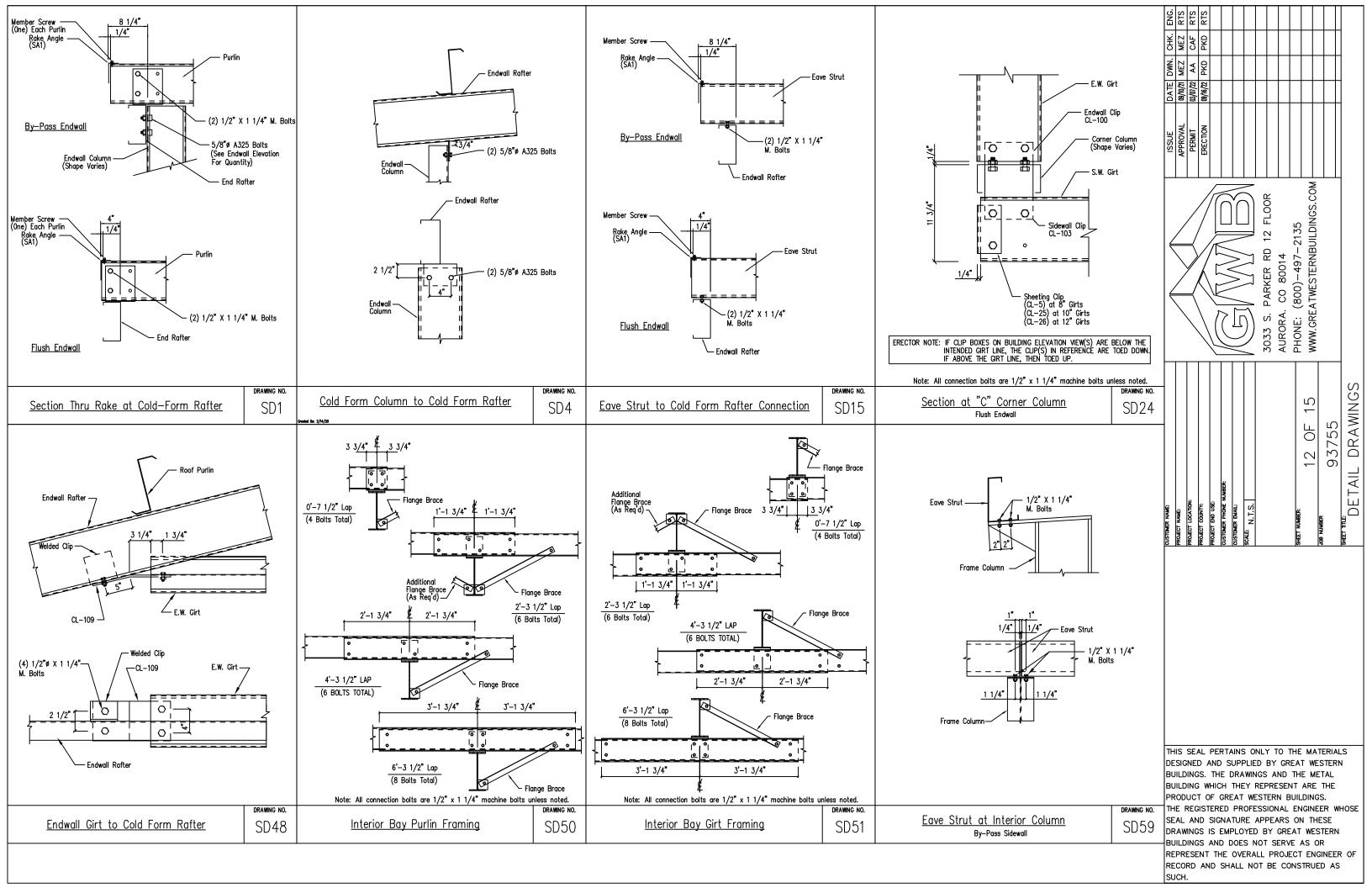
ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 GA. PBR - KOKO BROWN

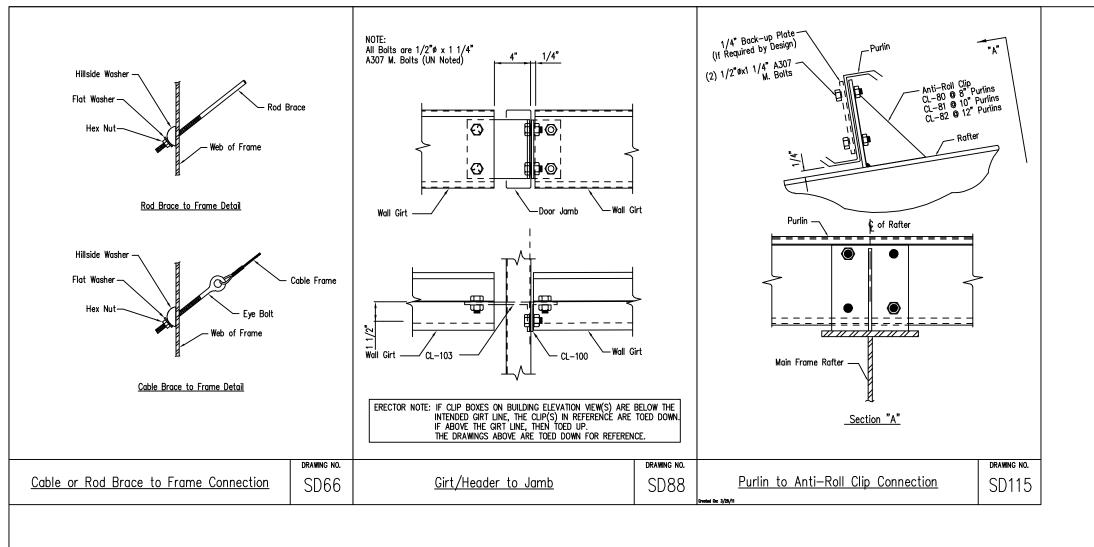
TRIM TABLE FRAME LINE 5 ◇ID QUAN PART 1	LENGTH	OUSTOWER NAME: PROCEST NAME: PROCEST BANK: PROCE
		THIS SEAL PERTAINS ONLY TO THE MATERIA DESIGNED AND SUPPLIED BY GREAT WESTER BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER OF SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERI BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER RECORD AND SHALL NOT BE CONSTRUED A

RECORD AND SHALL NOT BE CONSTRUED AS



BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS





CUSTOMER NAME:	←	ISSUE	DATE DWN. CHK.	· Ę	ENG.
PROJECT NAME:		APPROVAL	09/10/21 MEZ	MEZ	RTS
PROJECT LOCATION:		PERMIT	03/07/22 AA	CAF	RTS
PROJECT COUNTY:		ERECTION	09/16/22 PKD	PKD	RTS
Project end use:					
CUSTOMER PHONE NUMBER:					
CUSTOMER EMAIL:					
SCALE N.T.S.					
	000 ii 01 da ai/ava 0 2 2202				
	JUSS S. PARNER ND 12 FLUUR				
	AURORA. CO 80014				
SHEET NUMBER:	PHONE: (800)-497-2135				
13 OF 15	MOO SOMIC III I I I I I I I I I I I I I I I I				
	WWW.GREALWESTERINDOILDINGS.COM				
93755					
DETAIL DRAWINGS					

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