

GENERAL NOTES FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 ASTM DESIGNATION MIN. YIELD STRENGTH 1.2 MATERIALS HOT ROLLED STEEL SHAPES (W, & C) HOT ROLLED STEEL ANGLES (L) STEEL PIPES Fy = 36 KSI Fy = 42 KSI STRUCTURAL TURING A500 Fy = 42 KSI Fy = 50 KSI STRUCTURAL TUBING STRUCTURAL STEEL WEB PLATE STRUCTURAL STEEL FLANGE PLATES/BARS A529/A572 A653/A1011 Fv = 55 KSICOLD FORMED LIGHT GAGE ROOF & WALL SHEETS CABLE BRACE ROD BRACE Ev = 55 KSI Fy = 55 KSI Fy = 50, 80 KSI EXTRA HIGH STRENGTH Fy = 36 KSI A792/A653 A475 - TYPE 1 MIN. TENSILE STRENGTH MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1"ø & LESS) HIGH STRENGTH BOLTS (51"ø TO 1 1/2"ø) ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS, METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE, METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

I.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT:

A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N

B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325 NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325—N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE — BETWEEN ROOF PANELS & RIDGE CAP — BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SÉCTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE. 2.5

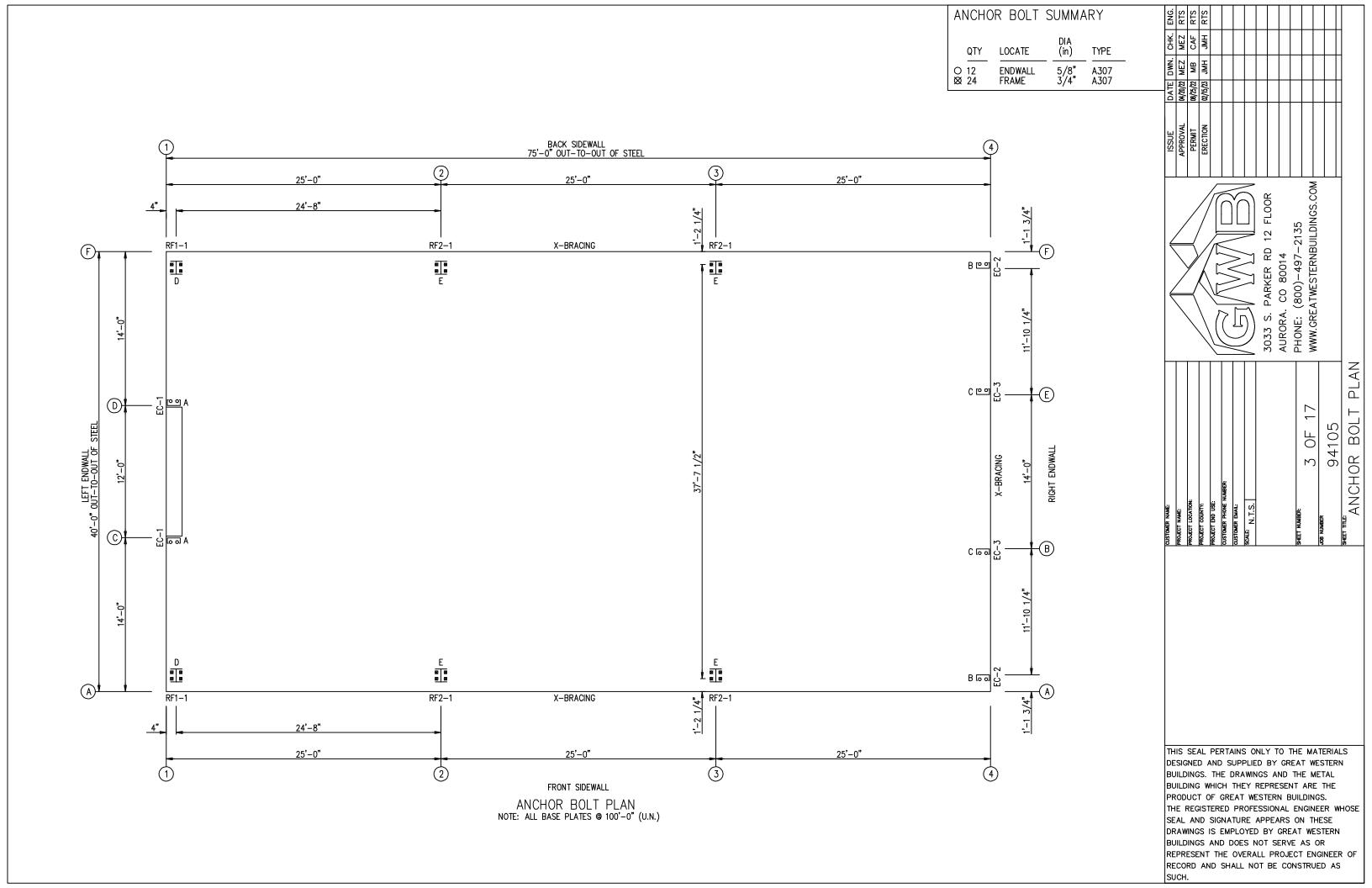
- 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR 1TS DESIGN ENGINEERS ARE ACTING AS THE REGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD
- METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE.

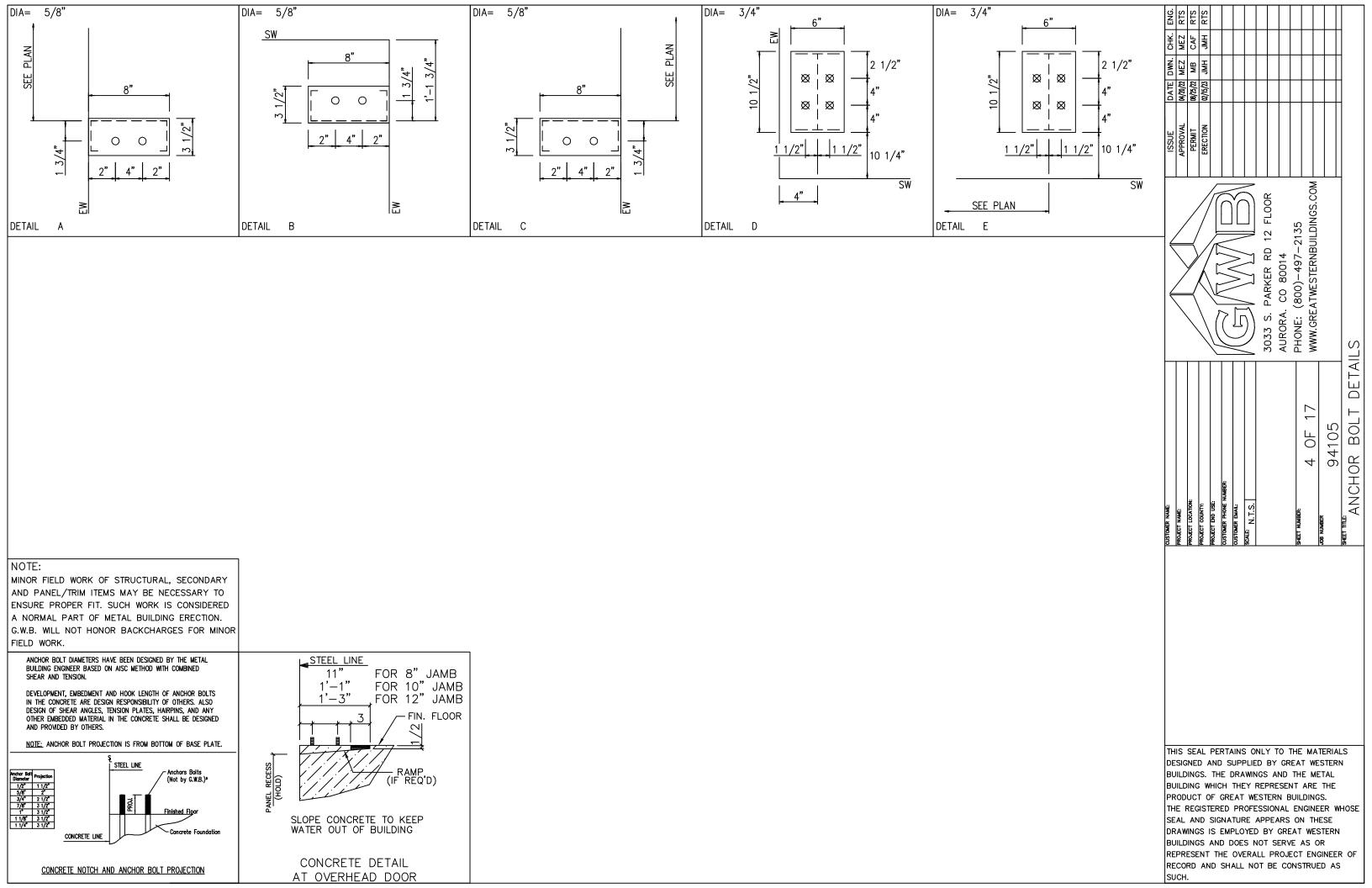
 UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with LEAD or copper. Both LEAD and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wiring, or tubing onto galvalume should be
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP, SSE TABLE BELOW.

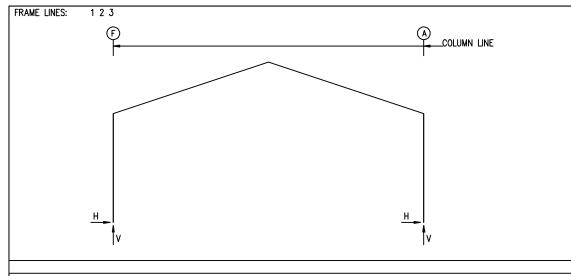
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

	BUILDING LOADS	RTS
THIS STRUCTURE HAS BEEN DESIGNE	D IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:	OHK. OMA OMA
DESIGN LOADS: DESIGN CODE / WIND CODE OCCUPANCY / RISK CATEGORY ENCLOSURE ROOF DEAD LOAD (D) (PSF) ROOF COLLATERAL LOAD (C) (PSF) WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF) WIND ENCLOSURE CLASSIFICATION LIVE LOAD PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF) SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct) SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie)	: CBC-19/IBC-18 : II-Normal : Closed : 2.00 : 1.00 : 115.00 : C : 0.18/-0.18	ISSUE DATE DWN.
SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM TOTAL DESIGN BASE SHEAR, (V) (KIPS)	: D : Ss = 0.581 :S1 = 0.259 : Sds = 0.517 :Sd1 = 0.359 : D : STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE : RIGID FRAMES (OMF) : BRACED FRAMES (OCBF/OMF) : LONGITUDINAL = 3.21 : TRANSVERSE = 3.24	7 0 COVER
RESPONSE MODIFICATION FACTORS, (R) SEISMIC RESPONSE COEFFICIENTS, (Cs) ANALYSIS PROCEDURE USED	:RIGID FRAMES = 3.25 Ω = 3.00 :SW X-BRACING = 3.25 Ω = 2.00 :RIGID FRAMES = 0.1592 :SW X-BRACING = 0.1592 :EQUIVALENT LATERAL FORCE PROCEDURE	2 OF 1 94105 DING INF
OTHER LOADS/REQUIREMENTS		CUSTOMER NAME: PROJECT NAME: PROJECT LOCATION: PROJECT COUNTY: PROJECT COUNTY: CUSTOMER PHONE NUMBER CUSTOMER PLANL: SCALE: N.T.S. SPRET INLE: SHEET INLE: SHEET INLE:
BUILDING DESCRIPTION: WIDTH (FT) : 40.00 LENGTH (FT) : 75.00 EAVE HEIGHT AT BSW (FT): 14.00 ROOF SLOPE AT BSW : 4.0:12 ROOF SLOPE AT FSW : 4.0:12 BAY SPACING (FT) : 3 AT 25.00 COVERING AND TRIMS: ROOF PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : DESERT SAND TRIM COLORS GABLE/EAVE : BURNISHED SLATE EAVE GUTTER : N/A WALL PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : RUSTIC RED TRIM COLORS CORNER : BURNISHED SLATE FRAMED OPENING : RUSTIC RED SOFFIT PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : RUSTIC RED TRIM COLORS : RUSTIC RED TRIM COLORS : RUSTIC RED TRIM COLORS : RUSTIC RED INSULATION ROOF INSULATION : N/A		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.





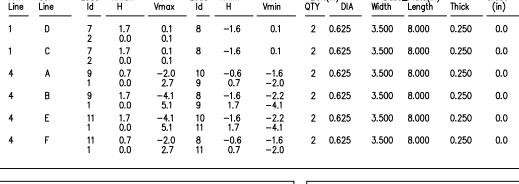


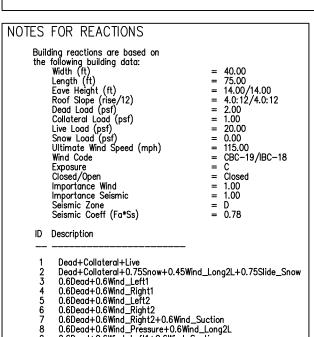
RIGID I	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOL	rs, & Base	PLATE	S				
Frm Line	Col Line	Load Id	Hmax H	imn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol ¹ QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
1	F	1	2.3	7.9	5 3	-1.9 -1.8	-2.3 -3.8	4	0.750	6.000	10.50	0.375	0.0
1	Α	6 1	1.9 -2.3	-2.3 7.9	1 4	-2.3 1.8	7.9 -3.8	4	0.750	6.000	10.50	0.375	0.0

RIGID F	RAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BAS	E PLATI	ES				
Frm Line	Col Line	Load Id	Hmax H	ımn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	F	1	4.2	13.2	5 3	-3.3 -3.2	-4.0 -6.7	4	0.750	6.000	10.50	0.375	0.0
2*	A	6 1	3.3 -4.2	-4.0 13.2	1 4	-4.2 3.2	13.2 -6.7	4	0.750	6.000	10.50	0.375	0.0
2*	FRAME II	ines:	2 3										

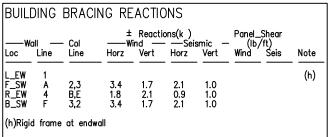
RIG	ID FRAI	νE:	BAS	IC COLUM	N REACTI	ONS (k))						
FRAM Line 1 1 2* 2*	IE Column Line F A F A	Horiz 0.3 -0.3 0.5 -0.5	-Dead Vert 1.1 1.1 1.6 1.6	Collo Horiz 0.1 -0.1 0.2 -0.2	oteral— Vert 0.3 0.3 0.6 0.6	Horiz 1.9 -1.9 3.6 -3.6	-Live Vert 6.5 6.5 11.0 11.0	Wind Horiz -3.2 -0.8 -5.8 -1.0	I_Left1- Vert -7.5 -4.8 -12.8 -8.1	-Wind_ Horiz 0.8 3.2 1.0 5.8	Right1- Vert -4.8 -7.5 -8.1 -12.8	Wind Horiz -3.4 -0.6 -6.0 -0.8	Left2- Vert -4.9 -2.2 -8.4 -3.7
FRAM Line 1 1 2* 2*	ME Column Line F A F A	-Wind_ Horiz 0.6 3.4 0.8 6.0	Right2- Vert -2.2 -4.9 -3.7 -8.4	Wind Horiz 0.5 0.2 0.7 0.6	I_Long1- Vert -5.4 -4.9 -10.9 -10.0	Wind Horiz -0.2 -0.5 -0.6 -0.7	I_Long2- Vert -4.9 -5.4 -10.0 -10.9	-Seism Horiz -0.4 -0.4 -0.6 -0.6	ic_Left Vert -0.3 0.3 -0.4 0.4	Seismic Horiz 0.4 0.4 0.6 0.6	_Right Vert 0.3 -0.3 0.4 -0.4	-Seism Horiz 0.0 0.0 0.0 0.0	ic_Long Vert 0.0 0.0 -1.0 -1.0
2*	FRAME li	nes:	2 3										

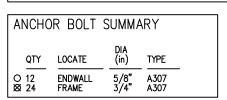
END۱	WALL	COLL	JMN:	E	BASIC CO	LUMN RI	EACTIONS	(k)							
Frm Line 1 1 4 4 4	Col Line D C A B E F	Dead Vert 0.1 0.3 0.6 0.6 0.3	Colle Vert 0.0 0.0 0.1 0.2 0.2		Live Vert 0.0 0.0 2.2 4.2 4.2 2.2	Wind Horz 0.0 0.0 0.0 -1.8 0.0 0.0	_Left1 Vert 0.0 0.0 -3.6 -7.5 -1.1 -2.9	Wind_I Horz 0.0 0.0 0.0 0.0 1.8 0.0	Right1 Ver 0.0 0.0 -2.9 -1.1 -7.5 -3.6	t Ho 0.0 0.0 0.0 -1.8 0.0	0.0 0.0 0.0 0.2 0.3 0.3	rt Horz 0 0.0 0 0.0 5 0.0 0 0.0 3 1.8	d_Right2 z Ver 0.0 0.0 -1.7 0.3 -6.0 -2.5	-2.6 -2.6 -1.0 -2.6 -2.6	ss Suct z Horz 5 2.9 5 2.9 1.2 5 2.9 2.9
Frm Line 1 1 4 4 4	Col Line D C A B E F	Wind_ Horz 0.0 0.0 0.0 0.0 1.1	Long1 Vert 0.0 0.0 -3.0 -3.5 -4.2 -2.0	Wind, Horz 0.0 0.0 0.0 -1.1 0.0 0.0	_Long2 Vert 0.0 0.0 -2.0 -4.2 -3.5 -3.0	Ho 0. 0. 0. -0. 0.	0 0.0 0 0.1 .9 -1.1 0 1.1	rt Hor: 0 0.0 0 0.0 1 0.0 1 0.0 0.9	0 0 1 -1	/ert).0).0).0).0 .1					
END۱	WALL	COLL	JMN:	М	AXIMUM I	REACTIO	NS, ANCHO	OR BOLTS,	& BAS	E PLATES					
Fri Lin				— Colum nax H	nn_React V Vmax	ions(k) Load Id	Hmin H	V Vmin	Bolt(QTY	(in) DIA	Base Width	=_Plate(in) Length	Thick	Grout (in)	
1	[) r	2	1.7 0.0	0.1 0.1	8	-1.6	0.1		0.625	3.500	8.000 8.000	0.250	0.0	

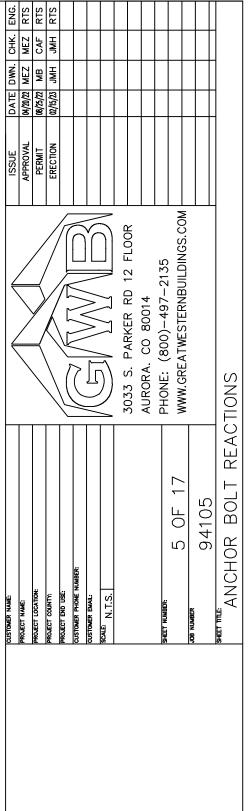




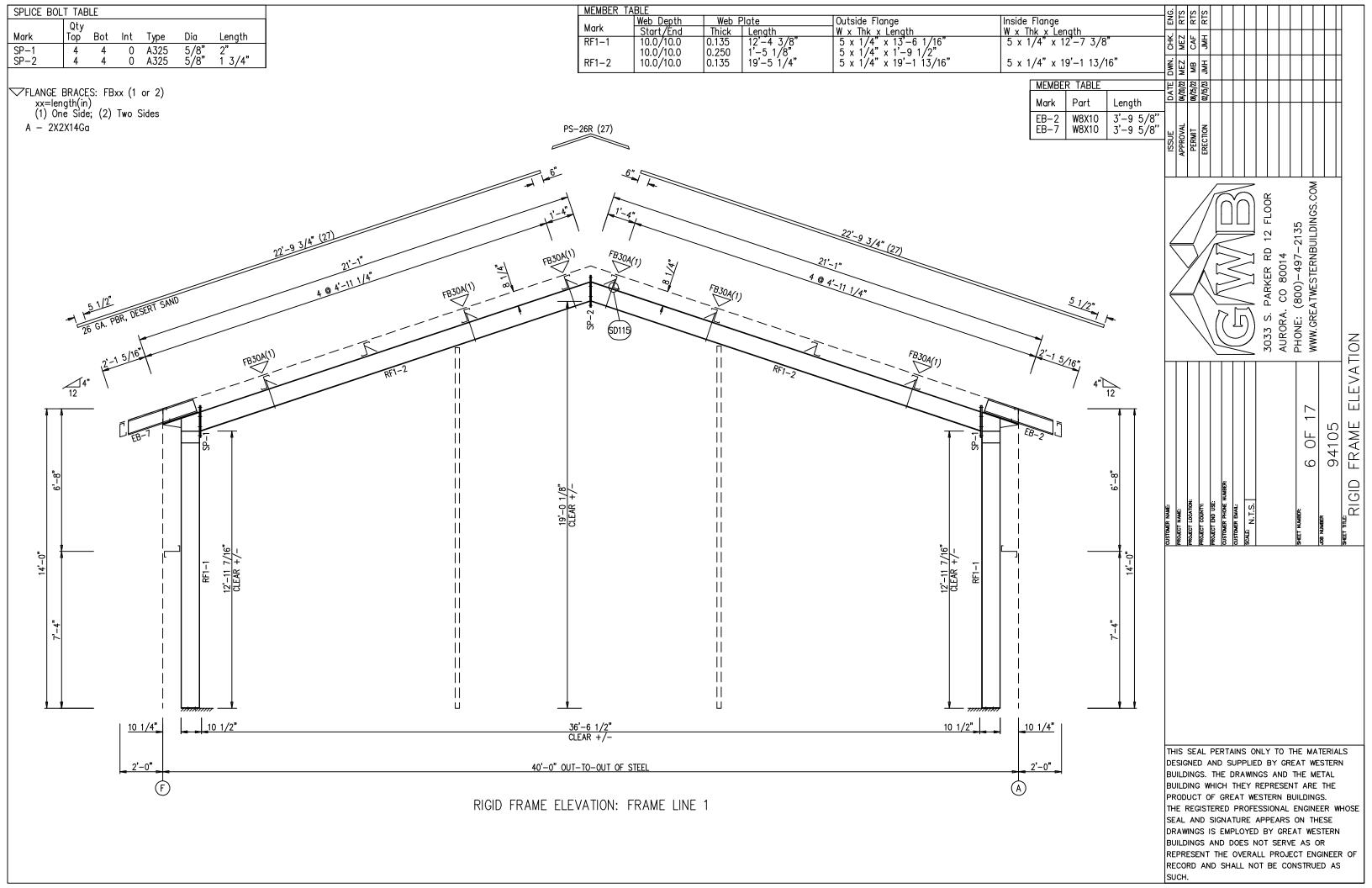
0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L 0.6Dead+0.6Wind_Right1+0.6Wind_Suction

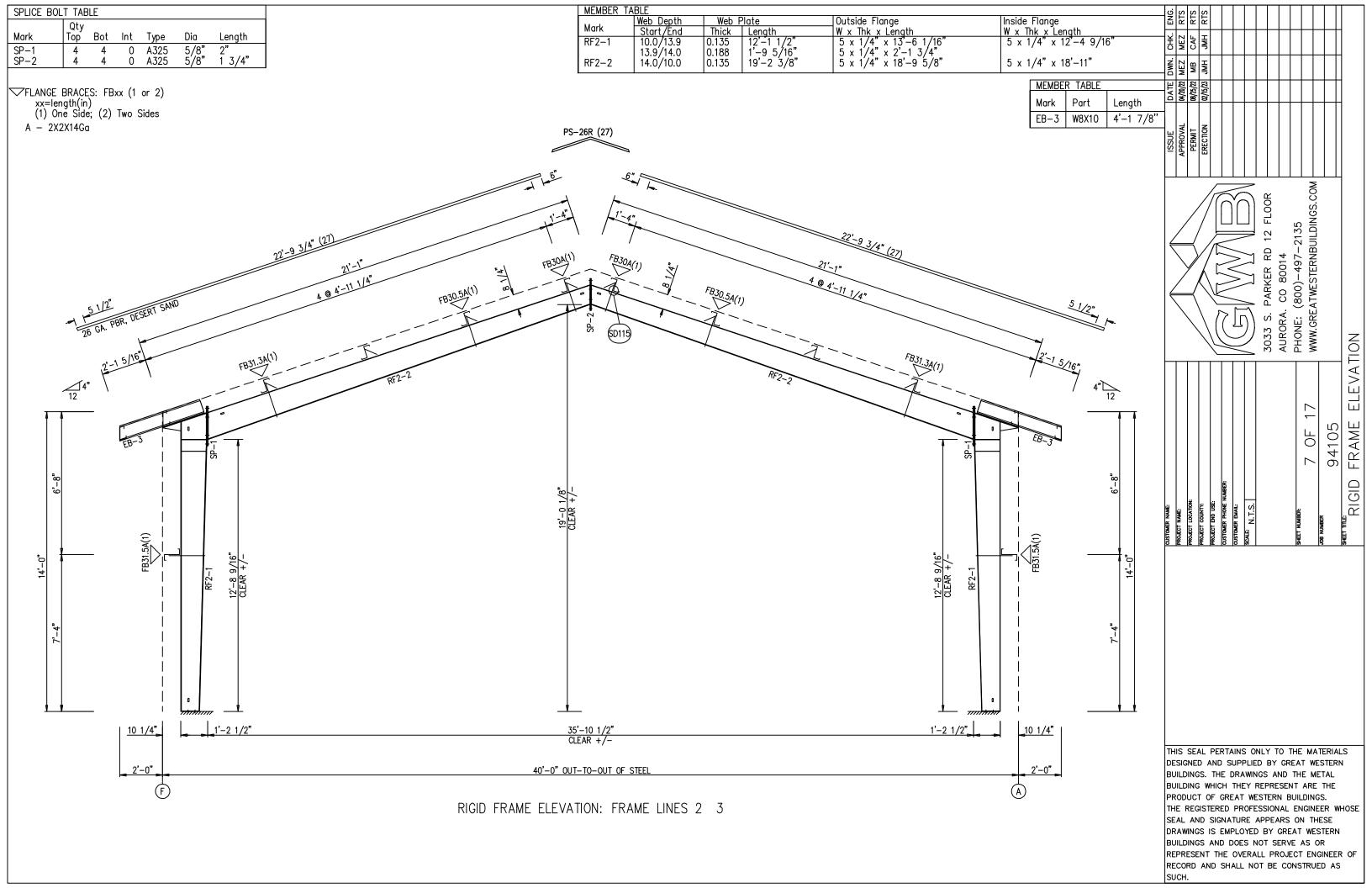


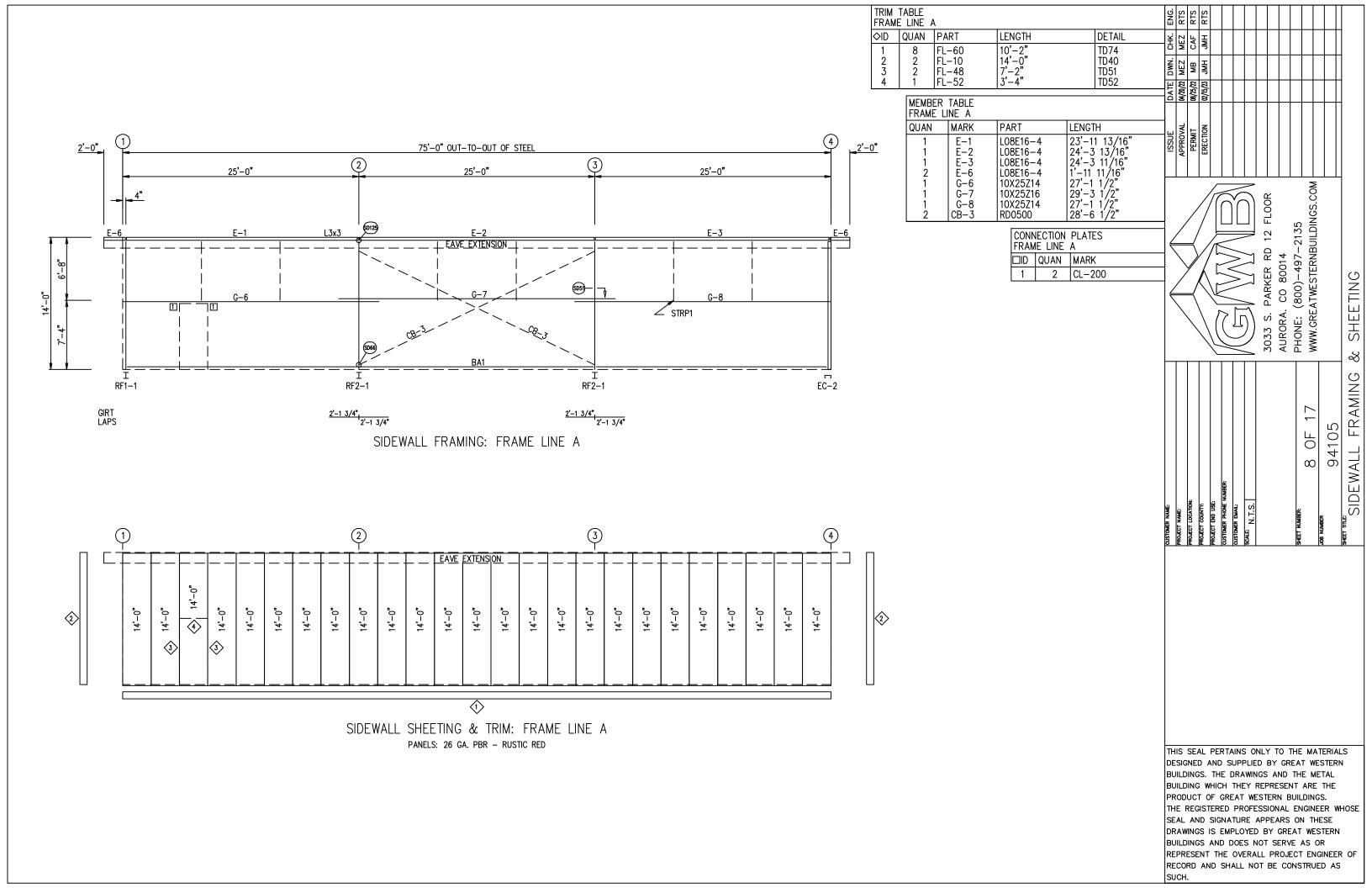


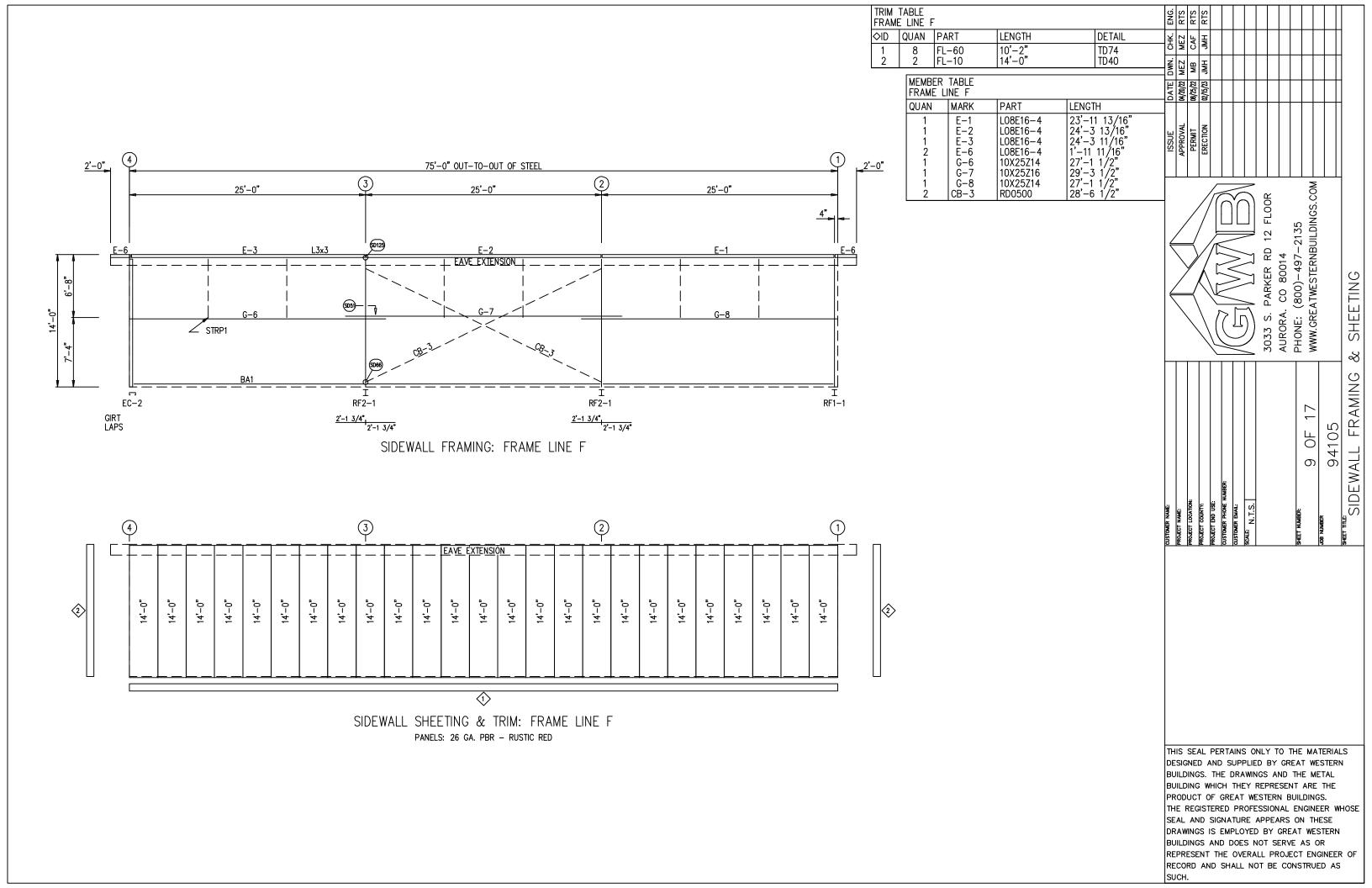


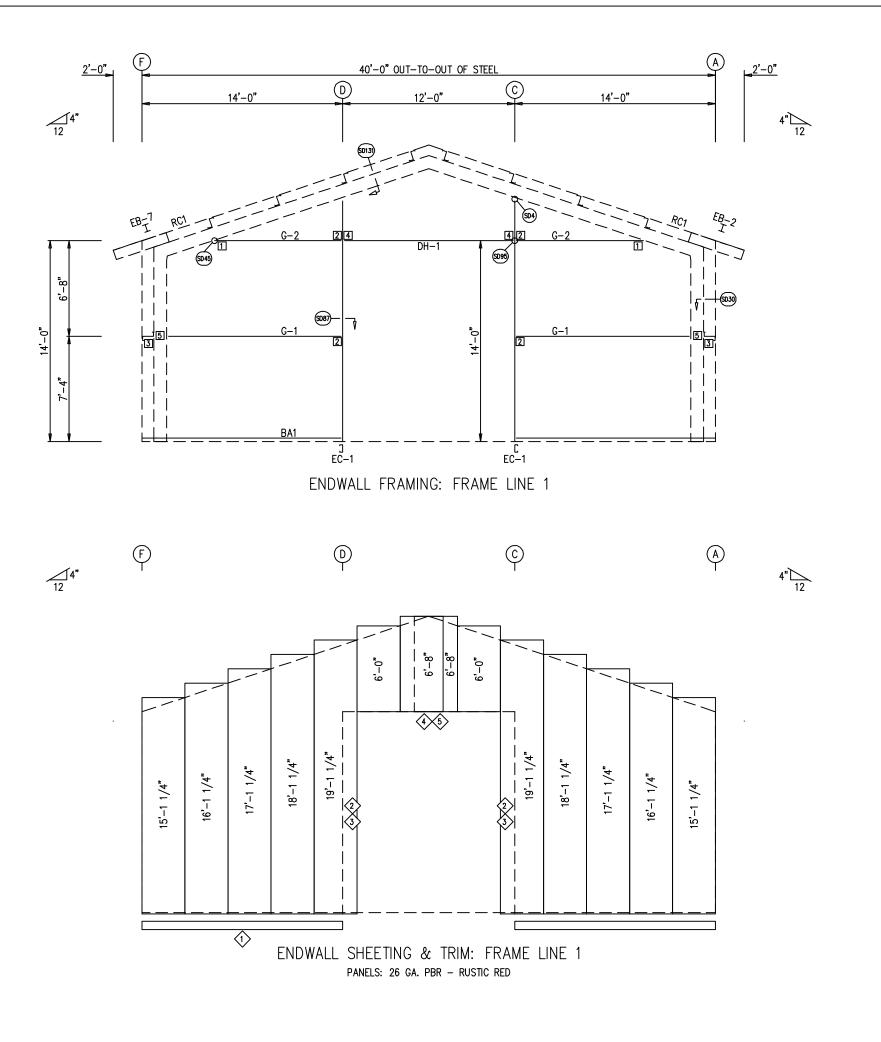
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BUILDING WHICH THEY REPRESENT ARE THE
PRODUCT OF GREAT WESTERN BUILDINGS.
THE REGISTERED PROFESSIONAL ENGINEER WHOSE
SEAL AND SIGNATURE APPEARS ON THESE
DRAWINGS IS EMPLOYED BY GREAT WESTERN
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REPRESENT THE OVERALL PROJECT ENGINEER OF
RECORD AND SHALL NOT BE CONSTRUED AS
SUCH.

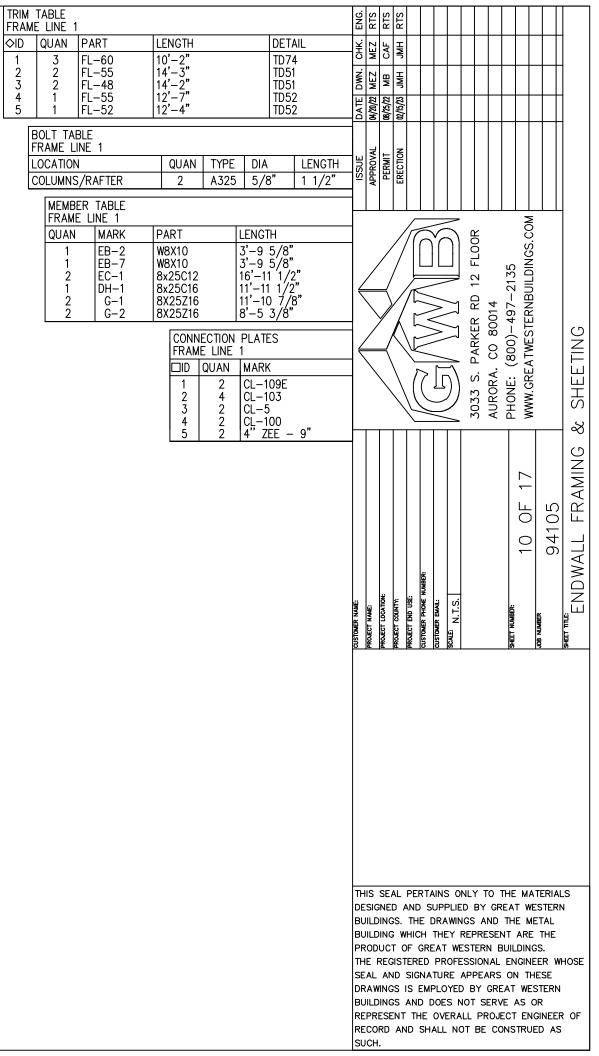


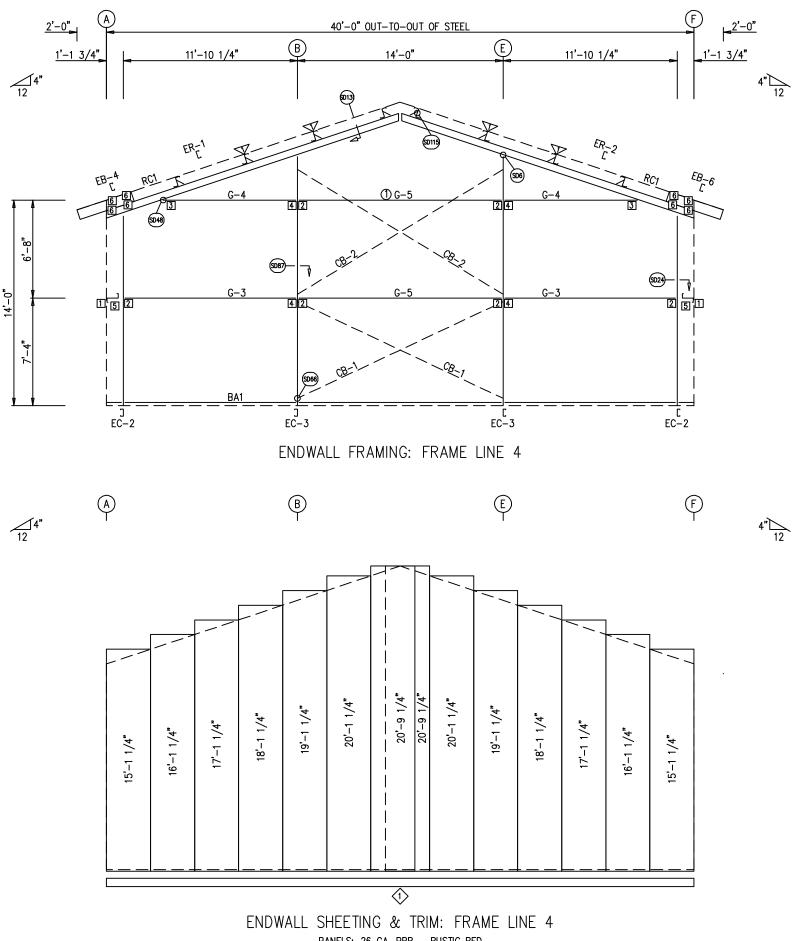












PANELS: 26 GA. PBR - RUSTIC RED

				FR LO ER	1 BC	
		2 2 1 1 2 2 2 2 2 2 2	MEMB FRAM QUAN 1	CATION CATION	4)LT TA	ABLE LINE 4 QUAN
		EC- EC- ER- ER- G- G-	ER TAB E LINE MA EB- EB-	INE 4	FL-60	4 PART
		-2 -3 -1 -2 -3 -4 -5 -1 -2 FIELD FRAM	4 RK -4	ER		
	FF	8x25; 8x25; 8x25; 8x25; 8X25; 8X25; RD05; RD05 WORK E LINE DETAIL	PART 8x25 8x25	QU 4 2	10'-2	LENG
	RAME ID Q 1 2 3 4 5 6	C16 C12 C12 C12 Z16 Z16 Z16 O00 TABL 4	C16	IAN L	2"	
	2 6 2 4 2	ENSION 9 7/16	4	TYPE A325 A325		
	MARK CL-2 CL-1 CL-1 CL-5 CL-2	12,-10 16,-10 21,-0 21,-0 11,-5 8,-1 13,-11 15,-10 16,-10	_ENGT	DIA 5/8 5/8		
	(11 00 09E 03 04 .CE TA	0 11/0 3/4' 3/4' 7/8" 1/4" 1 1/2 0 1/4 DIME	H 1/16"		TD74	DETA
		16°	•	LENGTH 1 1/2" 1 1/2"		IL
DE BU BU PF TH SE DF BU	CUSTOMER NAME:	€	ISSUE		DWN.	CHK. ENG.
SIGI JILDI RODI IE F AL RAWI JILDI	PROJECT NAME:	<u>/</u>	APPROVAL	04/20/22	-	-
NED ING: ING UCT REGI ANI ING:	PROJECT LOCATION:		PERMIT	08/25/22	MB CAF	\F RTS
O AN WHI OF STEF D SI S IS	PROJECT COUNTY: PROJECT END USE:		ERECTION	02/15/23	+	JMH RTS
D SI HE D CH GRE RED GNA EMF	CUSTOMER PHONE NUMBER:					
JPPL RAW THEY AT ' PRO TURE PLOY OES	CUSTOMER ENAIL:					
IED I	<i>i</i> i: : : :	ZOZZ S DABKEP BD 12 ELOOP				
AND RESE ERN I ONAL PEARS Y GR		3. 1 ANYEN NO 12 RA. CO 80014				
REAT THE ONT A BUILL ONE ON EAT VE A		800				
F WEST	11 OF 17	WWW.GREATWESTERNBUILDINGS.COM	MO			
THE S. ER WH ESE TERN	94105					
HOSE	SHET THE ENDWALL FRAMING &	& SHEETING				

RECORD AND SHALL NOT BE CONSTRUED AS

