

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH

Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & & LESS) MACHINE BOLTS & NUTS
HIGH STRENGTH BOLTS (1"Ø & LESS)
HIGH STRENGTH BOLTS (1"Ø TO 1 1/2"Ø)
A325-TYPE 1
ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)
A36/A307/F1554

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N

B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION)

C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION D) FLANGE BRACECONNECTIONS — 1/2" x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION. STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325—N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE - BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 <u>ERECTION NOTE:</u>
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE
CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10).
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- SUCH.

 METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)

 STANDARD SPECIFICATIONS APPLY UNILESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING. IT IS UNDESTOOD BY BOTH PARRIES THAT THE
 BUYER/FAID USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
 IN CASE OF DISCREPANCES BETWEEN M.B.S. STRUCTURAL STELE PLANS AND PLANS FOR
 OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD

 PRACTICES: LATEST EDITION)
- PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE. MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

- THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT PURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERI. CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OW M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST ENTION).
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THIS STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORA SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED F THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTION. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCH BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDAR
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERM THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN A

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AN IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AN FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)

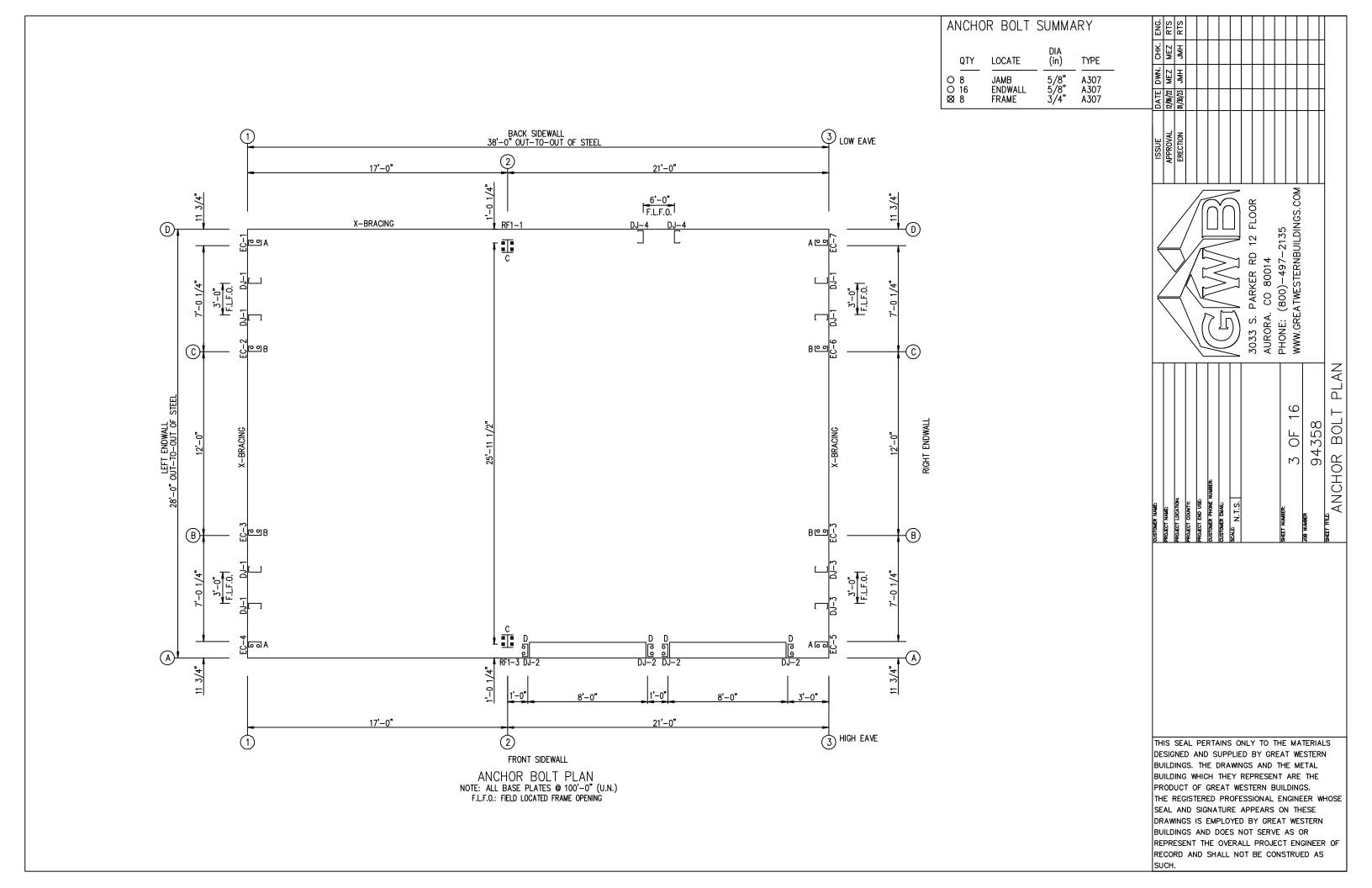
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO I REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWIS ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLES SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SUZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto galvalume should be
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SETONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS AFETY, MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBI SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NEW WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP, SSE TABLE BELOW.

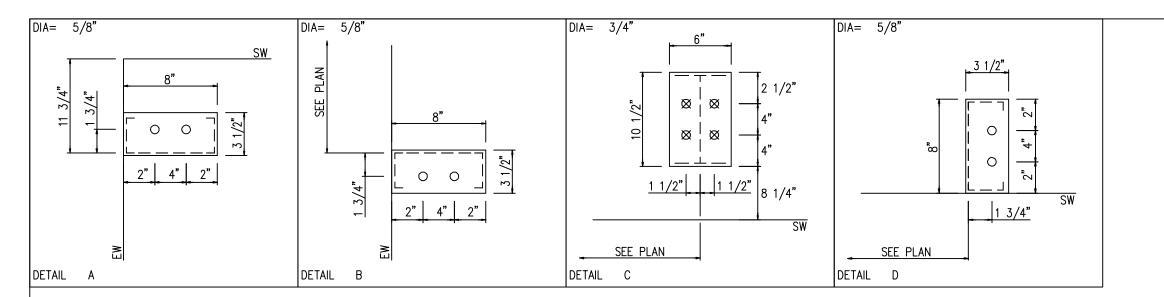
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

		BUILDING LOADS	ENG.	RTS							
.	THIS STRUCTURE HAS BEEN DESIGNED	O IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:		MEZ JMH							
O BE TERIA FOR	DESIGN LOADS: DESIGN CODE / WIND CODE	: IBC-21	DWN.	MEZ JMH			 				
ANS	OCCUPANCY / RISK CATEGORY ENCLOSURE	: II—Normal : Closed	DAT	2/06/22	Ц	$\perp \downarrow$	\perp				
	ROOF DEAD LOAD (D) (PSF) ROOF COLLATERAL LOAD (C) (PSF)	: 2.00 : 1.00									
THE	WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH)	:115.00	ISSUE	APPROVAL ERECTION							
	WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi)	: C : 0.18/-0.18		ERE APP							
RARY FOR CTOR.	WALL PANEL DESIGN WIND PRESSURE (PSF)	: 25.62/-27.76	Ш								
CHOR ARD	WIND ENCLOSURE CLASSIFICATION LIVE LOAD	: Closed			/,	$\overline{}$	<	~	3	<u>∑</u>	
ERMIT	PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION	: 20.00 : No		/	//	\bigcap	\bigcap	FLOOR	{	ر د	
r	SECONDARY FRAMING (PSF)	: 20.00		/		빌			2135	WWW.GREAIWESIERNBUILDINGS.COM	
I AND OR	SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF)	: 5.00	{	\dashv	//	eq	$\overline{}$	12	-21	ZCIL ZCIL	
DLUMN	ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce)	: 5.00 : 1.00	\	$\checkmark angle$	//		>	RD .	80014)-497-	<u>Z</u> K	
E AND	SNOW IMPORTANCE FACTOR, (Is)	:1.00		/ ,	\bigwedge	\geq	≥ 1	Ä Š	CO 80014 800)-497 TWE OTERS	2	
TE	THERMAL FACTOR, (Ct) SEISMIC LOAD	:1.00	(\dashv	//	\leq	_	PARKER	000	<u> </u>	
THE	SEISMIC IMPORTANCE FACTOR, (Ie) SITE CLASSIFICATION	: 1.00 : D		\	\	17			} ::: ⊱	Ř J	HH.
robe Ver ND	SPECTRAL RESPONSE ACCELERATION	: Ss = 0.055 : S1 = 0.024		\	//		((ك	3033	AURORA. PHONE:	ۆ	SHE
	SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY	: Sds = 0.059 : Sd1 = 0.038 : A			/		\leq	30	A T :	<u>*</u>	L SZ
RWISE NLESS IS	BASIC SEISMIC FORCE RESISTING SYSTEM	: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE			П						OVER
		:RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF)									C0
тн	TOTAL DESIGN BASE SHEAR, (V) (KIPS)	: LONGITUDINAL = 0.41 : TRANSVERSE = 0.46							ی ا	اد	0
6. E									-	1 00	ᆡᄔ
B.S.	RESPONSE MODIFICATION FACTORS, (R)	: RIGID FRAMES = 3.00 Ω = 3.00 : SW X-BRACING = 3.00 Ω = 3.00								435	-
ON AND IAKE	SEISMIC RESPONSE COEFFICIENTS, (Cs)	:RIGID FRAMES = 0.0196 :SW X-BRACING = 0.0196				Ë			(9 0	BUILDIN
GS UBBER NETS	ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	: EQUIVALENT LATERAL FORCE PROCEDURE	ا نو	NON:	رين مهند	CUSTOMER PHONE NUMBER:	T.S.		ين		3011
TION			CUSTOMER NAME:	ECT LOC	PROJECT COUNTY: PROJECT END USE:	OMER PH	CUSTOMER EM SCALE: N.T		HEET NUMBER:	JOB NUMBER	
F H 'OR			CUST	PRO	7. O. P.	usno Esta	SCAL		<u> </u>	80	뿛
~ ¬	BUILDING DESCRIPTION:										
RT	WIDTH (FT) : 28.00 LENGTH (FT) : 38.00										
	EAVE HEIGHT AT BSW (FT):14.00 EAVE HEIGHT AT FSW (FT):16.50										
	ROOF SLOPE AT BSW :1.0714:12 ROOF SLOPE AT FSW :N/A										
7	BAY SPACING (FT) :1 AT 17.00, 1 AT 21.0	00									
]	COVERING AND TRIMS:										
	ROOF PANELS & TRIMS PANEL TYPE : 26 GA. PBR										
	PANEL COLOR : GALVALUME TRIM COLORS										
_	GABLE/EAVE : COAL BLACK										
	EAVE GUTTER : N/A WALL PANELS & TRIMS										
	PANEL TYPE : 26 GA. PBR PANEL COLOR : CHARCOAL GRAY		TUI	S SE	A1 E	DEDTA	UNS () V T	O THE N	AATEDIA	AI S
	TRIM COLORS								GREAT		
	CORNER : COAL BLACK FRAMED OPENING : COAL BLACK								ND THE SENT AF		
	DOWNSPOUTS : N/A BASE : CHARCOAL GRAY		PR	ODUC	T OF	GRE	EAT W	ESTERN	N BUILDI	NGS.	
	SOFFIT PANELS								IAL ENGI ARS ON		WHOSE
	PANEL TYPE : 26 GA. PBR PANEL COLOR : COAL BLACK		DR	AWING	GS IS	S EMP	PLOYE	D BY	GREAT W	/ESTERN	N
	INSULATION ROOF INSULATION : N/A								ERVE AS ROJECT E		ER OF
	WALL INSULATION : N/A			CORD					CONSTR		
			1511	-							

SUCH.





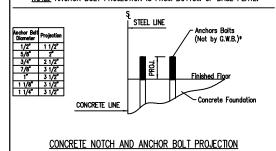
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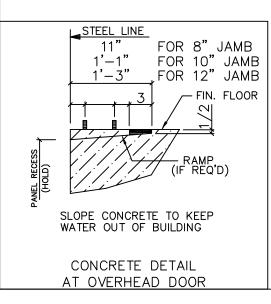
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM

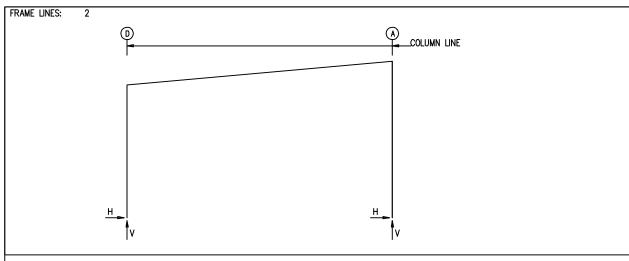
DETAIL

BOL

ANCHOR

4 OF 3

3033 S. F AURORA.



RIGID F	RAME:		MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES													
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolf QTY	t(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)			
2	D	3 1	2.7 1.6	6.8 8.4	5 7	-2.8 -0.4	-3.0 -6.3	4	0.750	6.000	10.50	0.375	0.0			
2	Α	6 1	2.5 -1.6	-2.4 9.1	2 4	-2.2 2.0	4.8 -4.1	4	0.750	6.000	10.50	0.375	0.0			

RIGID	FRAN	ΛE:	BASI	IC COLUN	IN REACT	IONS (k))						
FRAME Line 2 2	Column Line D A	Horiz 0.2 -0.2	-Dead Vert 1.1 1.2	Coll Horiz 0.1 -0.1	ateral- Vert 0.4 0.4	Horiz 1.4 –1.4	Live Vert 7.0 7.6	Horiz 0.3 -0.3	Snow Vert 1.8 1.9		d_Left1- Vert -9.0 -5.9	-Wind_ Horiz 3.2 3.5	Right1— Vert -2.2 -8.0
FRAME Line 2 2	Column Line D A	Wind Horiz -4.8 -1.0	d_Left2- Vert -6.1 -3.0	-Wind_ Horiz 2.5 4.3	Right2- Vert 0.6 -5.1	Wind Horiz -0.8 -2.1	_Long1- Vert -11.5 -5.3	Wind Horiz 3.2 0.0	_Long2- Vert -5.6 -6.1	-Seism Horiz -0.1 -0.1	ic_Left Vert -0.1 0.1	Seismic Horiz 0.1 0.1	:_Right Vert 0.1 -0.1
FRAME Line 2 2	Column Line D A	-Seism Horiz -0.3 -0.2	nic_Long Vert -0.6 0.3	-MIN_9 Horiz 0.3 -0.3	SNOW Vert 1.7 1.9								

NOTES FOR REACTIONS	
Building reactions are based on the following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= 28.00 = 38.00 = 14.00/16.50 = 1.0714.12/ = 2.00 = 1.00 = 5.00 = 115.00 = 18C-21 = C = Closed = 1.00 = A = 0.09
ID Description	_
1 Dead+Collateral+Live 2 Dead+Collateral+0.75Live+0.45V 3 Dead+Collateral+0.75Live+0.45V 4 0.6Dead+0.6Wind_Right1 5 0.6Dead+0.6Wind_Right2 6 0.6Dead+0.6Wind_Right2 7 0.6Dead+0.6Wind_Lorg1L 8 0.6Dead+0.6Wind_Lorg1L 9 0.6Dead+0.6Wind_Pressure+0.61 10 Dead+Collateral+0.75Live+0.45V 11 Dead+Collateral+0.75Live+0.45V 12 0.6Dead+0.6Wind_Suction+0.6Wind_Suction+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.6Wind_Right1+0.45V	Vind_Long2R _Suction Wind_Long1L Wind_Right2+0.45Wind_Suction Wind_Long1L d_Long1L d_Suction+0.45Wind_Long2L ind_Long1L d_Suction

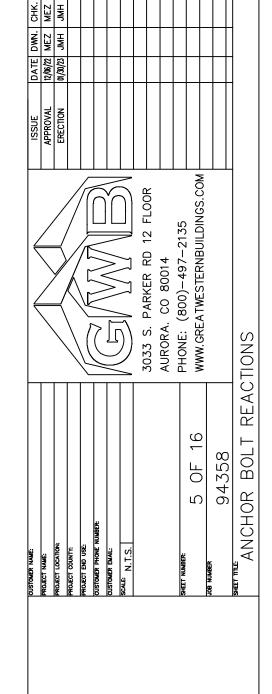
BUIL	DING	BRACIN	IG RE	ACTIO)NS			
——Wo	ıll — Line	- Col Line	——W Horz	Reaction — Vert	ons(k) - —Sei Horz	smic – Vert	Panel_ - (lb, Wind	Shear ft) Seis
L_EW F_SW	1 A	C,B Torsional	4.2 Bracina	5.1 Used	0.3	0.4		
R_EW B_SW	A 3 D	B,C 2,1	2.2 4.1	2.6 3.0	0.1 0.4	0.2 0.3		

ANCHO	R BOLT	SUMMA	\RY	
QTY O 8 O 16 Ø 8	LOCATE JAMB ENDWALL FRAME	DIA (in) 5/8" 5/8" 3/4"	A307 A307 A307 A307	-

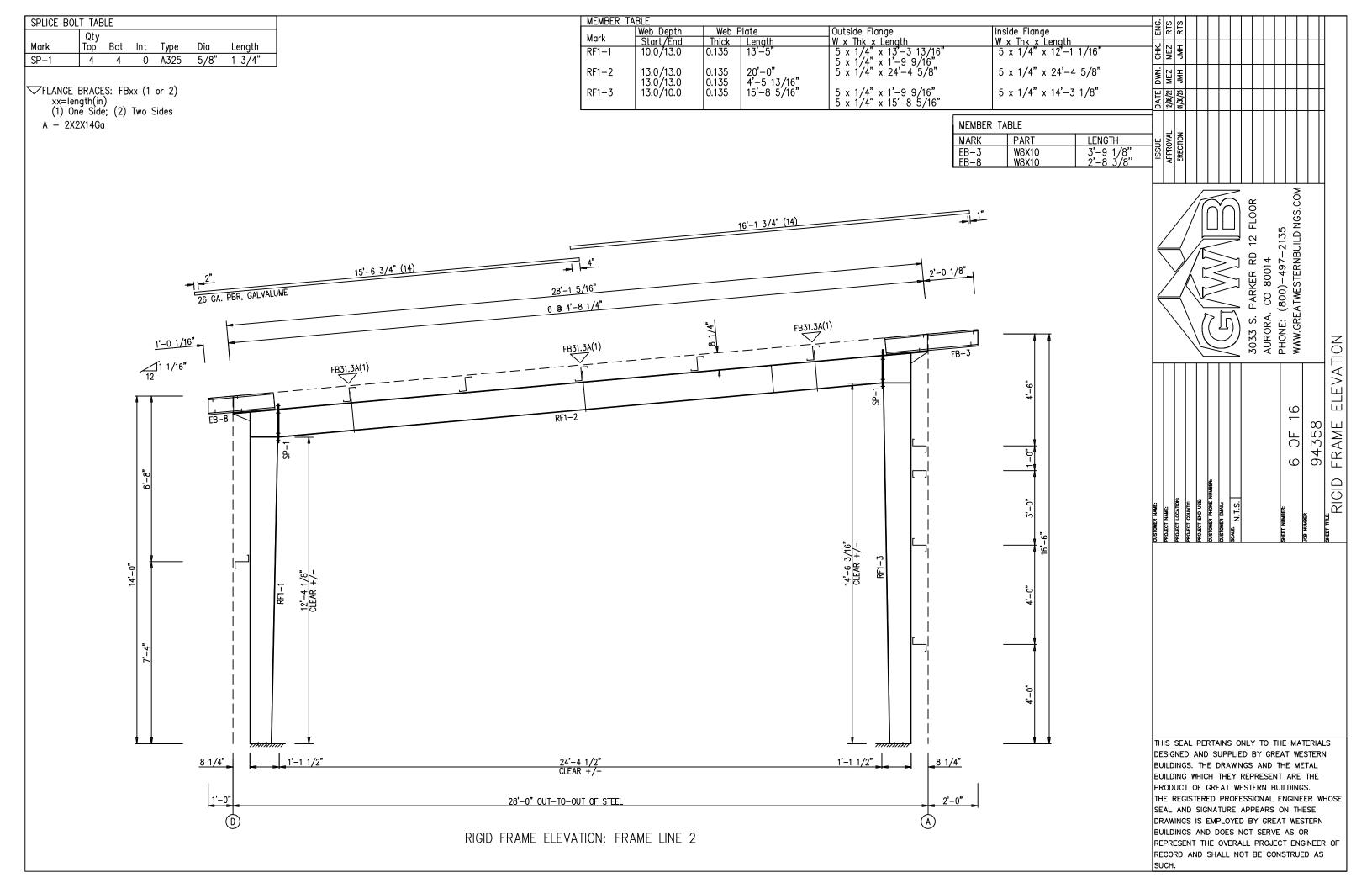
END'	WALL	COLU	JMN:	В	ASIC COLU	JMN REAC	TIONS (k)								
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.2 0.3 0.3 0.2	Collo Vert 0.0 0.1 0.1	(ive /ert).8 2.1 2.0 .1	Snow Vert 0.2 0.5 0.5	Wind_ Horz 0.0 -1.0 0.0 0.0	Left1 Vert -1.1 -4.0 -1.7 -1.0	Win Hor 0.0 0.0 1.8 0.0	- (Vert 0.6	Wind_Le Horz 0.0 -1.0 0.0 0.0	ft2 Vert -0.8 -3.1 -0.7 -0.8	0.0 1.7	ht2 Vert -0.3 1.5 -2.5 -1.3	
Frm Line 1 1 1	Col Line D C B A	Wind_ Horz -4.7 -1.3 -1.4 -0.7	Press Vert -3.0 0.0 0.0 0.0	Wind Horz 0.7 1.5 1.6 0.8	_Suct Vert 3.0 0.0 0.0 0.0	Wind_ Horz 0.0 0.0 0.5 0.0	Long1 Vert -0.8 -2.2 -3.4 -0.7	Wind Horz 0.0 0.0 0.2 0.0	_Long2 Ver -0.5 -1.4 -1.9 -0.4	t 1 5 (. –	Seis_Le Horz 0.0 0.1 0.0 0.0	ft Vert 0.0 -0.1 0.1 0.0	Seis_Rigi Horz 0.0 0.0 0.1 0.0	Vert 0.0	Seis_L Horz -0.4 -0.3 0.0 0.0	ong Vert -0.3 -0.4 0.4 0.0
Frm Line 1 1 1	Col Line D C B A	-MIN Horz 0.0 0.0 0.0 0.0	_SNOW Vert 0.2 0.5 0.5 0.3	E1PA1 Horz 0.0 0.0 0.0 0.0	C_SL_1- Vert 0.1 0.1 0.0 0.0	E1PAT Horz 0.0 0.0 0.0 0.0	-SL_2- Vert 0.0 0.0 0.1 0.1	E1PA Horz 0.0 0.0 0.0 0.0	T_SL_3- Ver 0.1 0.3 0.2 0.0	t	E1PAT_SI Horz 0.0 0.0 0.0 0.0	Vert 0.0 0.2 0.3 0.1				
Frm Line 3 3 3	Col Line A B C D	Dead Vert 0.2 0.3 0.3	Collo Vert 0.1 0.1 0.1 0.0	1	ive /ert .3 2.4 2.6 0.9	Snow Vert 0.3 0.6 0.6 0.2	Wind_ Horz 0.0 -2.2 0.0 0.0	Left1 Vert -1.7 -4.3 0.8 -0.7	Win Hor 0.0 0.0 1.3 0.0	_ _ _	Vert 1.1	Wind_Le Horz 0.0 -2.0 0.0 0.0	ft2 Vert -1.5 -3.0 1.7 -0.4	0.0 1.3	ht2 Vert -0.9 -0.7 -3.7 -1.0	Wind Press Horz -0.7 -1.4 -1.3
Frm Line 3 3 3	Col Line A B C D	Wind Suct Horz 0.8 1.6 1.5	Wind Horz 0.0 -0.5 0.0 0.0	L_Long1 Ver -0.8 -4.0 -2.6 -1.0	t Hor 3 0.0 3 –0.2 5 0.0) -0.4 2 -2.2) -1.6	t Hoo 2 -0.0 0.0) 0.0 l –0.) 0.3	rt 1 0 (2 (2 (Seis_Rig Iorz).0).0).1).1	ght Vert 0.0 0.1 -0.2 0.0	-MIN. Horz 0.0 0.0 0.0 0.0	_SNOW Vert 0.3 0.6 0.6 0.2	E2PAT Horz 0.0 0.0 0.0 0.0	SL_1- Vert 0.1 0.1 0.0 0.0	
Frm Line 3 3 3	Col Line A B C D	E2PA Horz 0.0 0.0 0.0 0.0	T_SL_2- Vert 0.0 0.0 0.1 0.1	E2PA Horz 0.0 0.0 0.0 0.0	T_SL_3- Vert 0.1 0.3 0.2 0.0	E2PAT Horz 0.0 0.0 0.0 0.0	T_SL_4- Vert 0.0 0.2 0.3 0.1									
END'	WALL	COLU	JMN:	MA	XIMUM RE	EACTIONS,	ANCHOR	BOLTS, &	k BASE	PLATES	;					
Fr Lir			Load Hn	nax	n_Reactio V l Vmax	ns(k) _oad Hm Id H		V min	Bolt(in QTY) DIA	Bas Width	e_Plate(Length		Grout (in)	_	
1	[)		0.4 0.3	1.2 2.0	9 -:	2.8	-2.2	2 0.	625	3.500	8.000	0.250	0.0		
1	(0	8	0.9 0.7	-2.2 3.6	9 -	o.8 -	-4.0	2 0.	625	3.500	8.000	0.250	0.0		
1	E	3	12	1.0 0.0	-4.5 2.4		0.9 .0 -	0.6 - 4 .5	2 0.	625	3.500	8.000	0.250	0.0		
1	,	Ą	13	0.5 0.0	-0.8 1.3	9 -).4 -	-0.3 -0.8	2 0.	625	3.500	8.000	0.250	0.0		
3		A	1 (0.5 0.0	-0.9 1.5	8 0).5 -	-0.4 -0.9		625	3.500	8.000	0.250			
3	E		1 (1.0 0.0	-2.4 2.9	8 1	.0 -	-2.2 -2.4		625	3.500	8.000	0.250			
3) 1	14 (0.9 0.7 n.4	-2.7 3.2 -0.7	13 ().9 -	-1.4 -2.7 -0.5		625 625	3.500	8.000	0.250			

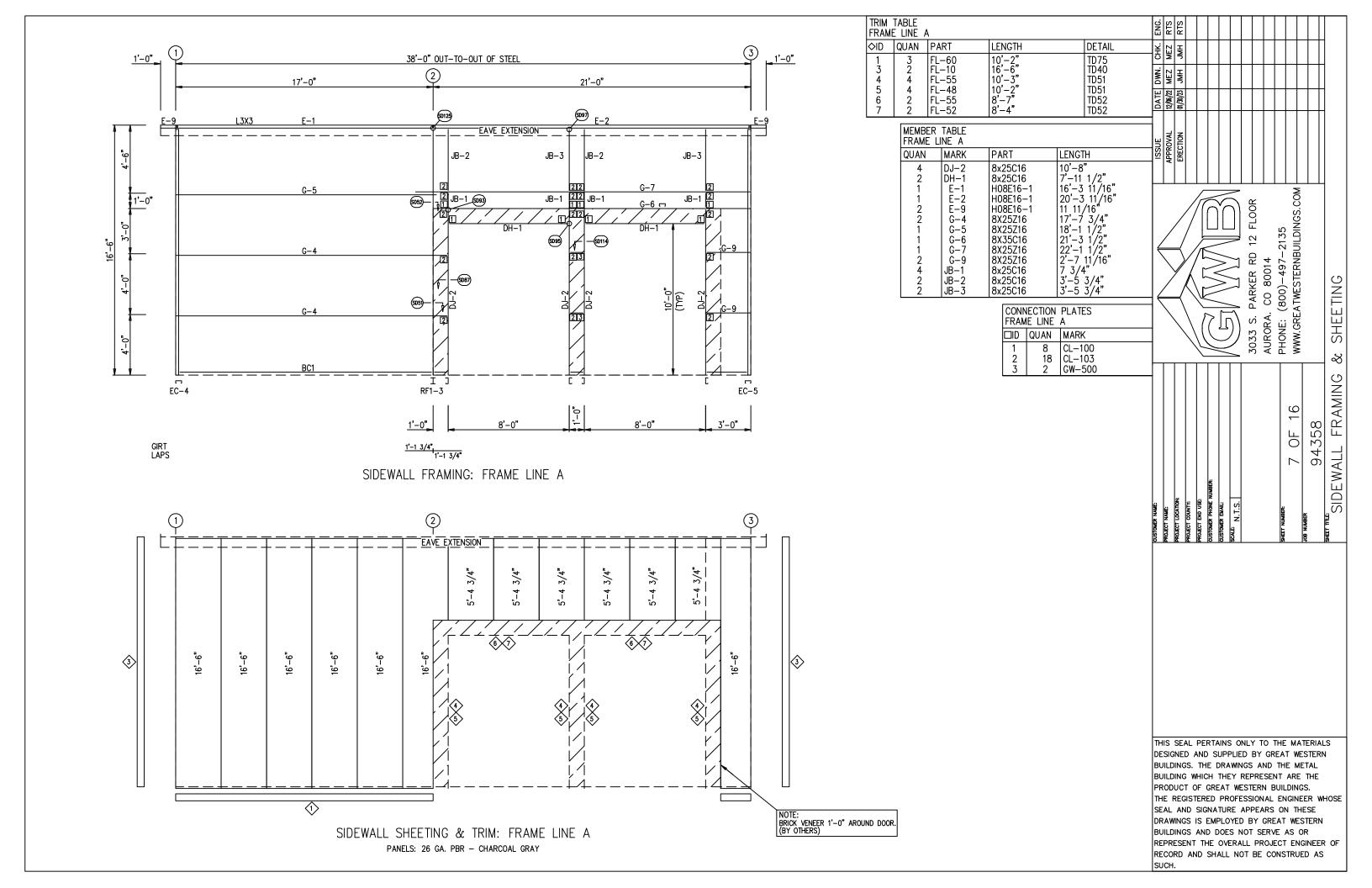
-0.3 0.4

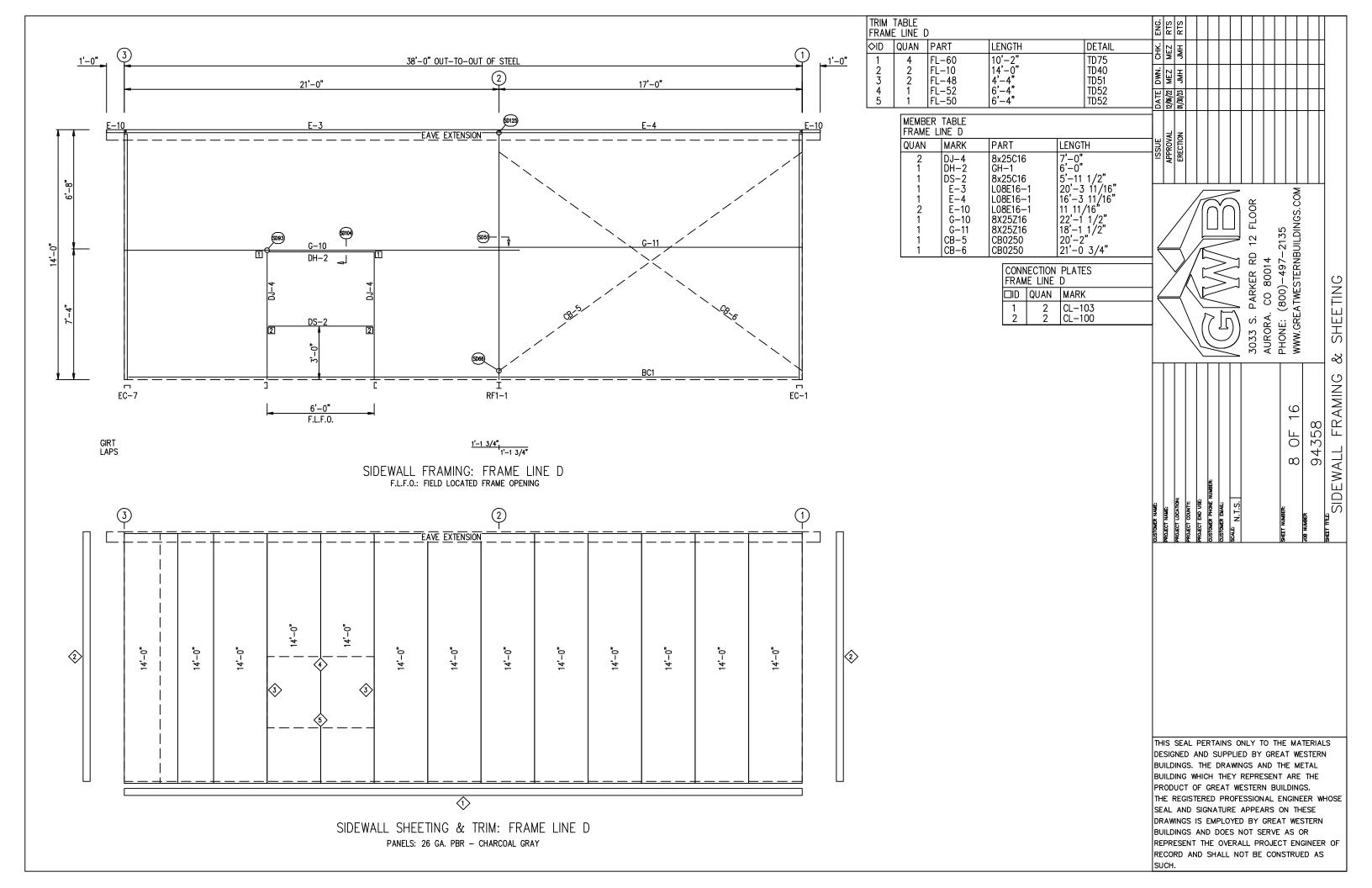
9 13 2 0.625 3.500 8.000 0.250 0.0

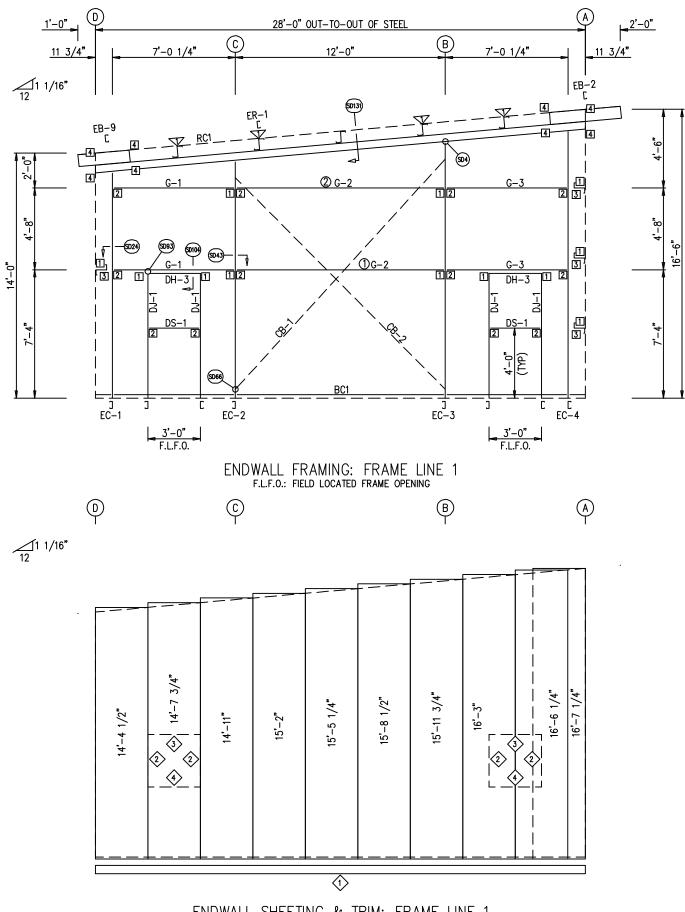


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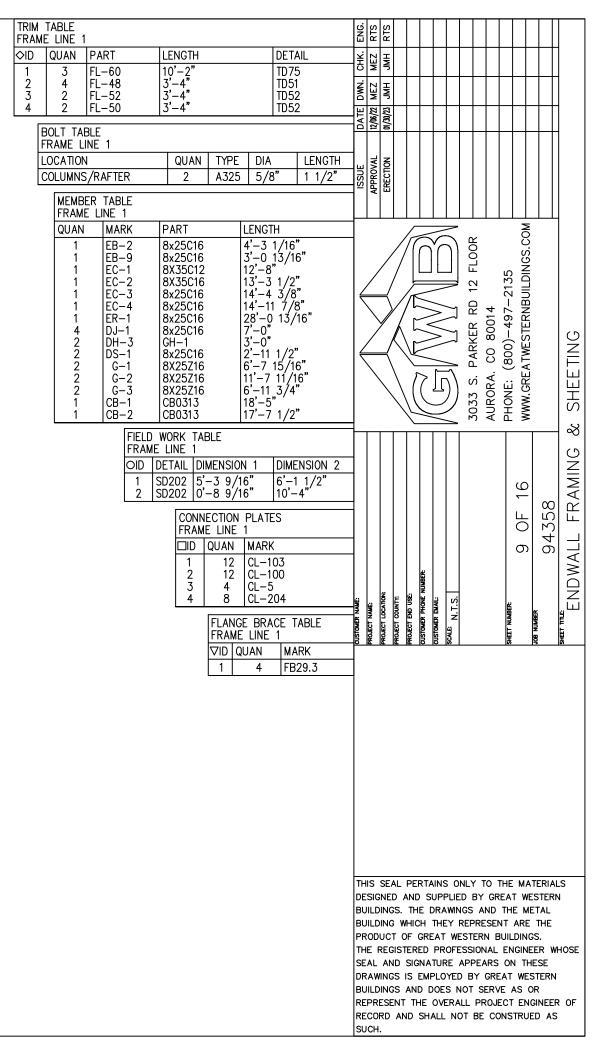


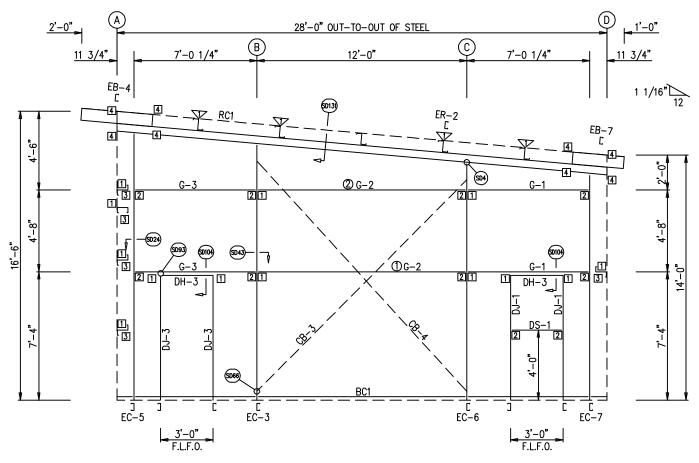




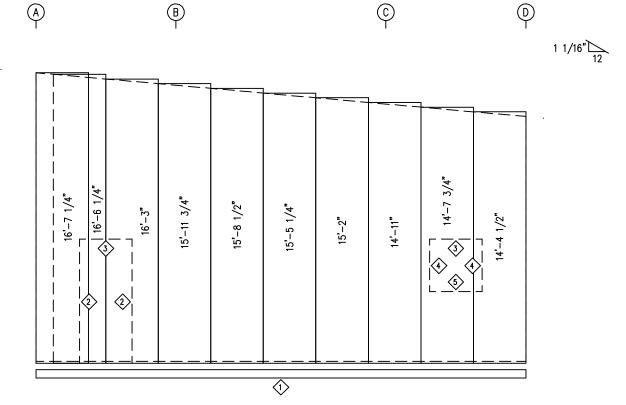


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - CHARCOAL GRAY



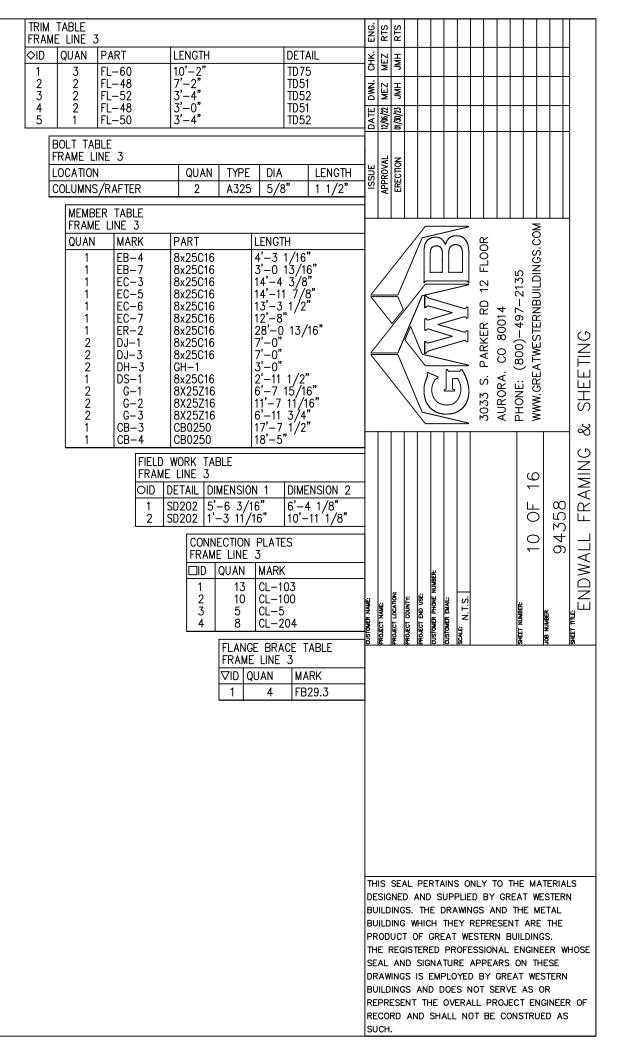


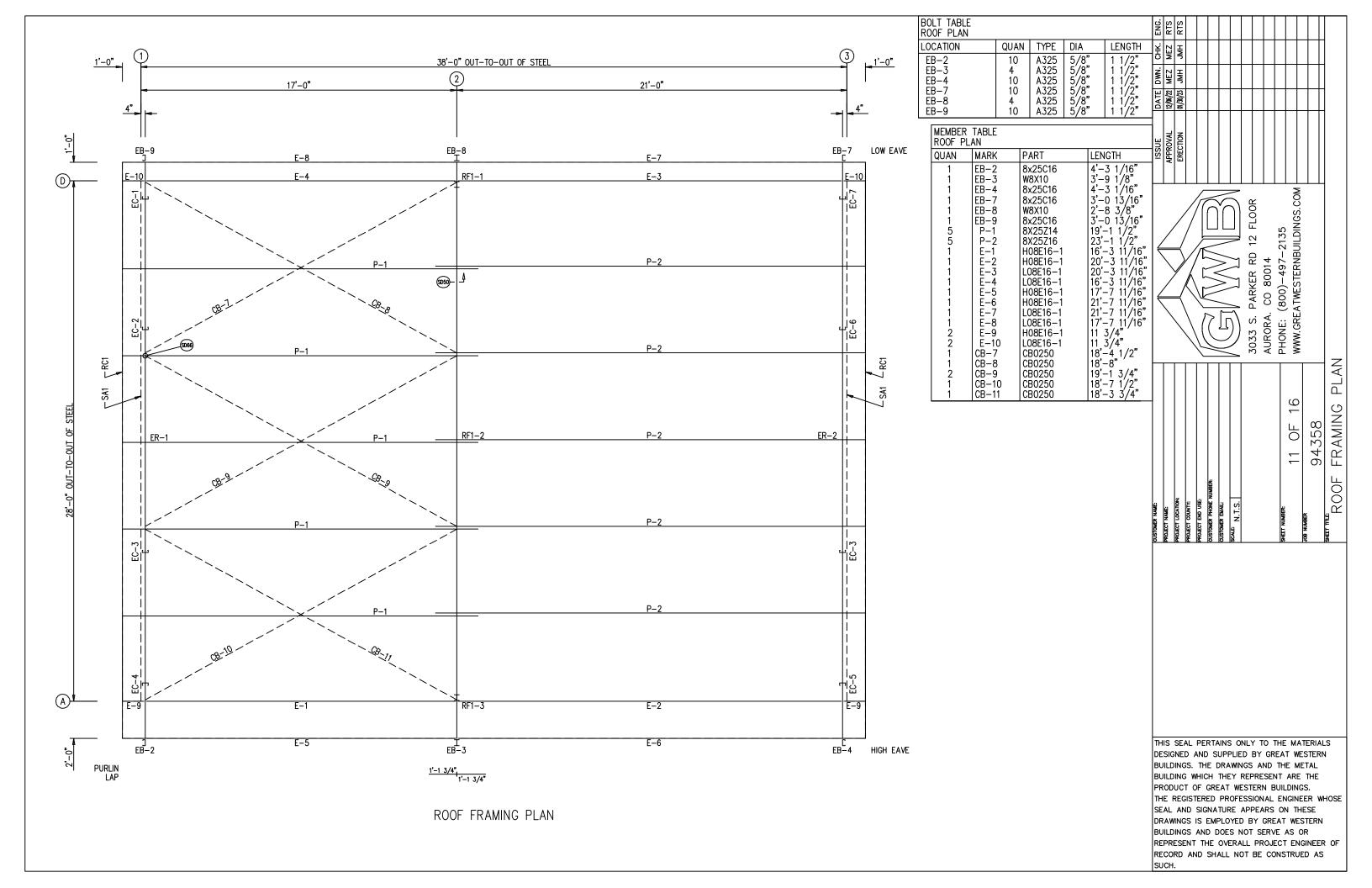
ENDWALL FRAMING: FRAME LINE 3 F.L.F.O.: FIELD LOCATED FRAME OPENING

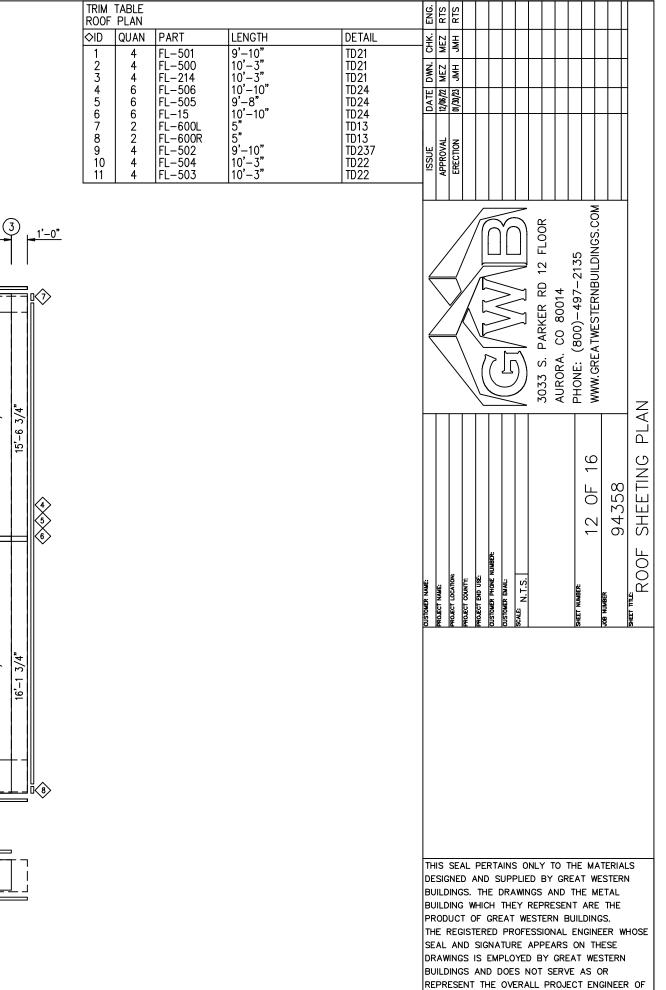


ENDWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 26 GA. PBR - CHARCOAL GRAY







RECORD AND SHALL NOT BE CONSTRUED AS

SUCH.

D	1'-0"-	1					38'-0"	0UT-T0-0U	_	L 						_1'-0"
	\$\displaystyle{4}\$\displaystyle{5}\$\displaystyle{4}\$	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"	15'-6 3/4"		15'-6 3/4"	\$\frac{1}{\sqrt{5}}\cdot\frac{1}{\sqrt{5}}\cd
28'-0" 0UT-T0-0UT OF STEEL		16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'–1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"	16'-1 3/4"		16'-1 3/4"	6
A 2,-0,	®0	- <u>-</u>						3>						- 		D\8
- 1	(13) 1'-10 3/4" [A]							<u>\$</u>							- - - - -	

ROOF SHEETING PLAN

PANELS: 26 GA. PBR - GALVALUME

[A] SOFFIT PANELS: 26 GA. PBR - COAL BLACK

