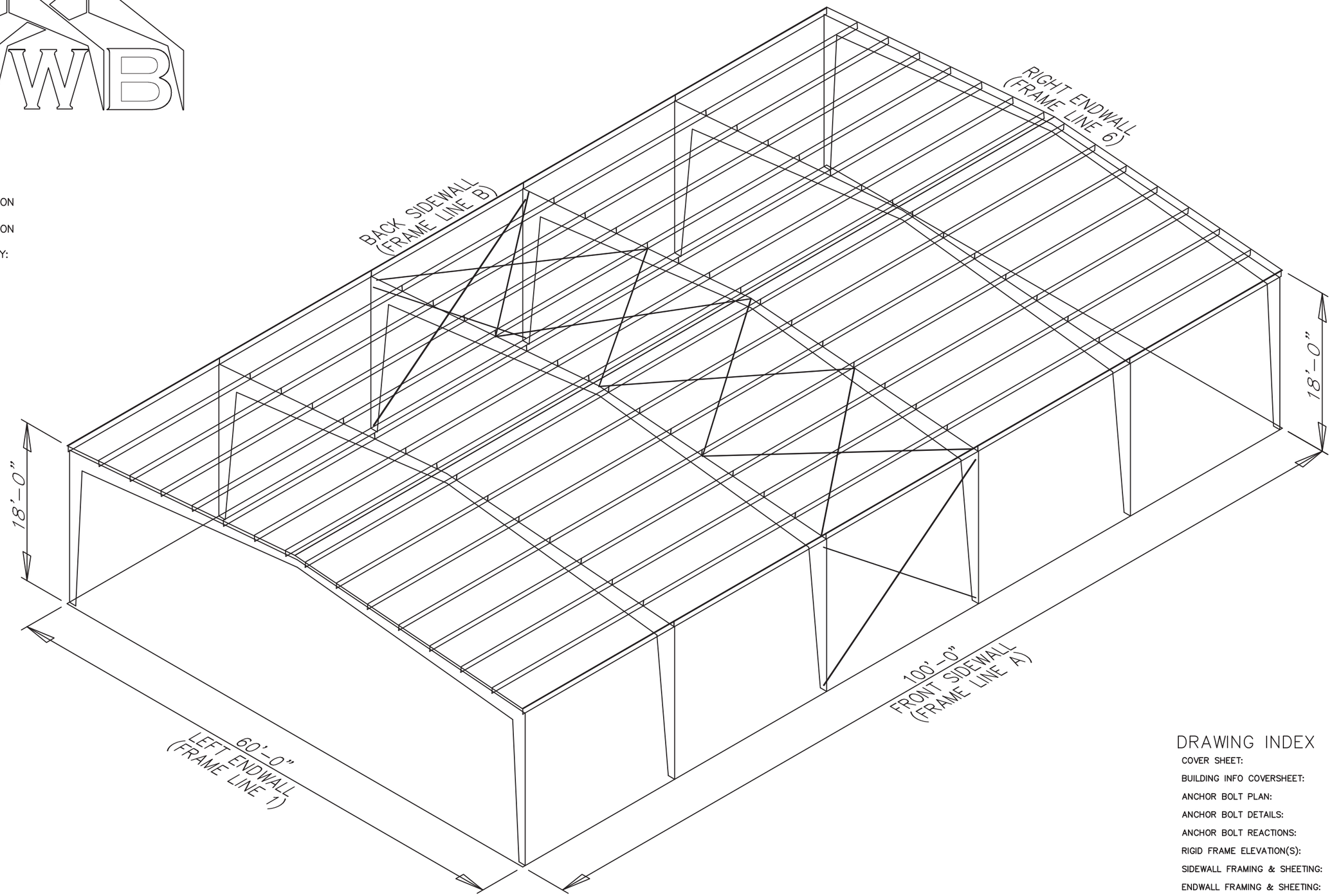
JOB NUMBER:

PROJECT NAME:

PROJECT LOCATION

PROJECT LOCATION

PROJECT COUNTY:




## DRAWING INDEX

## GENERAL NOTES

1.1 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2 MATERIALS

| | ASTM DESIGNATION | MIN. YIELD STRENGTH |
|---|---|---|
| HOT ROLLED STEEL SHAPES (W, & C) | A572 | Fy = 50 KSI |
| HOT ROLLED STEEL ANGLES (L) | A36 | Fy = 36 KSI |
| STEEL PIPES | A500 | Fy = 42 KSI |
| STRUCTURAL TUBING | A500 | Fy = 42 KSI |
| STRUCTURAL STEEL WEB PLATE | A572/A1011 | Fy = 50 KSI |
| STRUCTURAL STEEL FLANGE PLATES/BARS | A529/A572 | Fy = 55 KSI |
| COLD FORMED LIGHT GAGE | A653/A1011 | Fy = 55 KSI |
| ROOF & WALL SHEETS | A792/A653 | Fy = 50, 80 KSI |
| CABLE BRACE | A475 – TYPE 1 | EXTRA HIGH STRENGTH |
| ROD BRACE | A36 | Fy = 36 KSI |
| | | MIN. TENSILE STRENGTH |
| MACHINE BOLTS & NUTS | A307 | Fu = 60 KSI |
| HIGH STRENGTH BOLTS (1"ø & LESS) | A325–TYPE 1 | Fu = 120 KSI |
| HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) | A325–TYPE 1 | Fu = 105 KSI |
| ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) | A36/A307/F1554 | Fu = 60 KSI |

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS:
SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'–1 1/4" A307 EXCEPT :
A) ENDWALL RAFTER SPLICE – 5/8"ø x 0'–1 3/4" A325–N
B) ENDWALL COLUMN TO RAFTER CONNECTION – (SEE WALL ELEVATION)
C) MAIN FRAME CONNECTIONS – SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS – 1/2"ø x 0'–1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325–N UNLESS SPECIFICALLY NOTED OTHERWISE.
HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN–OF–THE–NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325–N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
INSIDE– UNDER ROOF PANELS & BASE OF WALL PANELS
OUTSIDE – BETWEEN ROOF PANELS & RIDGE CAP
– BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

## BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING SUCH.

2.2 METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.) STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE, METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

2.3 IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

2.4 APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

2.5 ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST EDITION)

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE.
UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 WARNING IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN–OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.

2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

| ROOF SNOW LOAD (IN PSF) | EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES) | RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES) |
|---|---|---|
| 20 | 16.60 | 8.30 |
| 25 | 17.25 | 8.62 |
| 30 | 17.90 | 8.95 |
| 35 | 18.55 | 9.28 |
| 40 | 19.20 | 9.60 |
| 45 | 19.85 | 9.92 |
| 50 | 20.50 | 10.25 |
| 55 | 21.15 | 10.58 |
| 60 | 21.80 | 10.90 |
| 65 | 22.45 | 11.22 |
| 70 | 23.10 | 11.55 |
| 75 | 23.75 | 11.88 |
| 80 | 24.40 | 12.20 |

NOTE:
FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI–A8–2

## BUILDING LOADS

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

| | |
|---|---|
| DESIGN CODE / WIND CODE | : IBC–21 |
| OCCUPANCY / RISK CATEGORY | : II – Normal |
| ENCLOSURE | : Open |
| ROOF DEAD LOAD (D) (PSF) | : 1.74 |
| ROOF COLLATERAL LOAD (C) (PSF) | : 1.00 |
| WIND LOAD | |
| ULTIMATE WIND SPEED, (VULT) (MPH) | : 115.0 |
| WIND EXPOSURE CATEGORY | : C |
| INTERNAL PRESSURE COEFFICIENT, (GCpi) | : 0.00/0.00 |
| WALL PANEL DESIGN WIND PRESSURE (PSF) | : 22.44/–24.69 |
| WIND ENCLOSURE CLASSIFICATION | : Open |
| LIVE LOAD | |
| PRIMARY FRAMING (PSF) | : 20.00 |
| TRIB. AREA REDUCTION | : No |
| SECONDARY FRAMING (PSF) | : 20.00 |
| SNOW LOAD | |
| GROUND SNOW LOAD, (Pg) (PSF) | : 30.0 |
| ROOF SNOW LOAD, (Pf) (PSF) | : 30.0 |
| SNOW EXPOSURE FACTOR, (Ce) | : 1.00 |
| SNOW IMPORTANCE FACTOR, (Is) | : 1.00 |
| THERMAL FACTOR, (Ct) | : 1.20 |
| SEISMIC LOAD | |
| SEISMIC IMPORTANCE FACTOR, (Ie) | : 1.00 |
| SITE CLASSIFICATION | : D |
| SPECTRAL RESPONSE ACCELERATION | : Ss = 0.470 :S1 = 0.171 |
| SPECTRAL RESPONSE COEFFICIENTS | : Sds = 0.446 :Sd1 = 0.257 |
| SEISMIC DESIGN CATEGORY | : D |
| BASIC SEISMIC FORCE RESISTING SYSTEM | : STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE<br>: RIGID FRAMES (OMF)<br>: BRACED FRAMES (OCBF/OMF) |
| TOTAL DESIGN BASE SHEAR, (V) (KIPS) | : LONGITUDINAL = 4.35<br>: TRANSVERSE = 4.71 |
| RESPONSE MODIFICATION FACTORS, (R) | : RIGID FRAMES = 3.25 Ω= 3.00<br>: SW X–BRACING = 3.25 Ω= 2.00 |
| SEISMIC RESPONSE COEFFICIENTS, (Cs) | : RIGID FRAMES = 0.1374<br>: SW X–BRACING = 0.1374 |
| ANALYSIS PROCEDURE USED | : EQUIVALENT LATERAL FORCE PROCEDURE |
| OTHER LOADS/REQUIREMENTS | |

BUILDING DESCRIPTION:

| | |
|---|---|
| WIDTH (FT) | : 60.0 |
| LENGTH (FT) | : 100.0 |
| EAVE HEIGHT AT BSW (FT) | : 18.0 |
| EAVE HEIGHT AT FSW (FT) | : 18.0 |
| ROOF SLOPE AT BSW | : 1.0:12 |
| ROOF SLOPE AT FSW | : 1.0:12 |
| BAY SPACING (FT) | : 5 AT 20 |

COVERING AND TRIMS:

| | |
|---|---|
| ROOF PANELS & TRIMS | |
| PANEL TYPE | : 26 GA. PBR |
| PANEL COLOR | : GALVALUME |
| TRIM COLORS | |
| GABLE/EAVE | : SOLAR WHITE |

| | ENG. | CHK. | DWN. | DATE | ISSUE |
|---|---|---|---|---|---|
| | RTS | MEZ | MEZ | 01/31/23 | APPROVAL |
| | RTS | MEZ | AA | 02/27/23 | PERMIT |
| | RTS | CAF | JMM | 10/25/23 | ERECTION |
| | RTS | JMM | JMM | | |


GWB
3033 S. PARKER RD 12 FLOOR
AURORA, CO 80014
PHONE: (800)–497–2135
WWW.GREATWESTERNBUILDINGS.COM

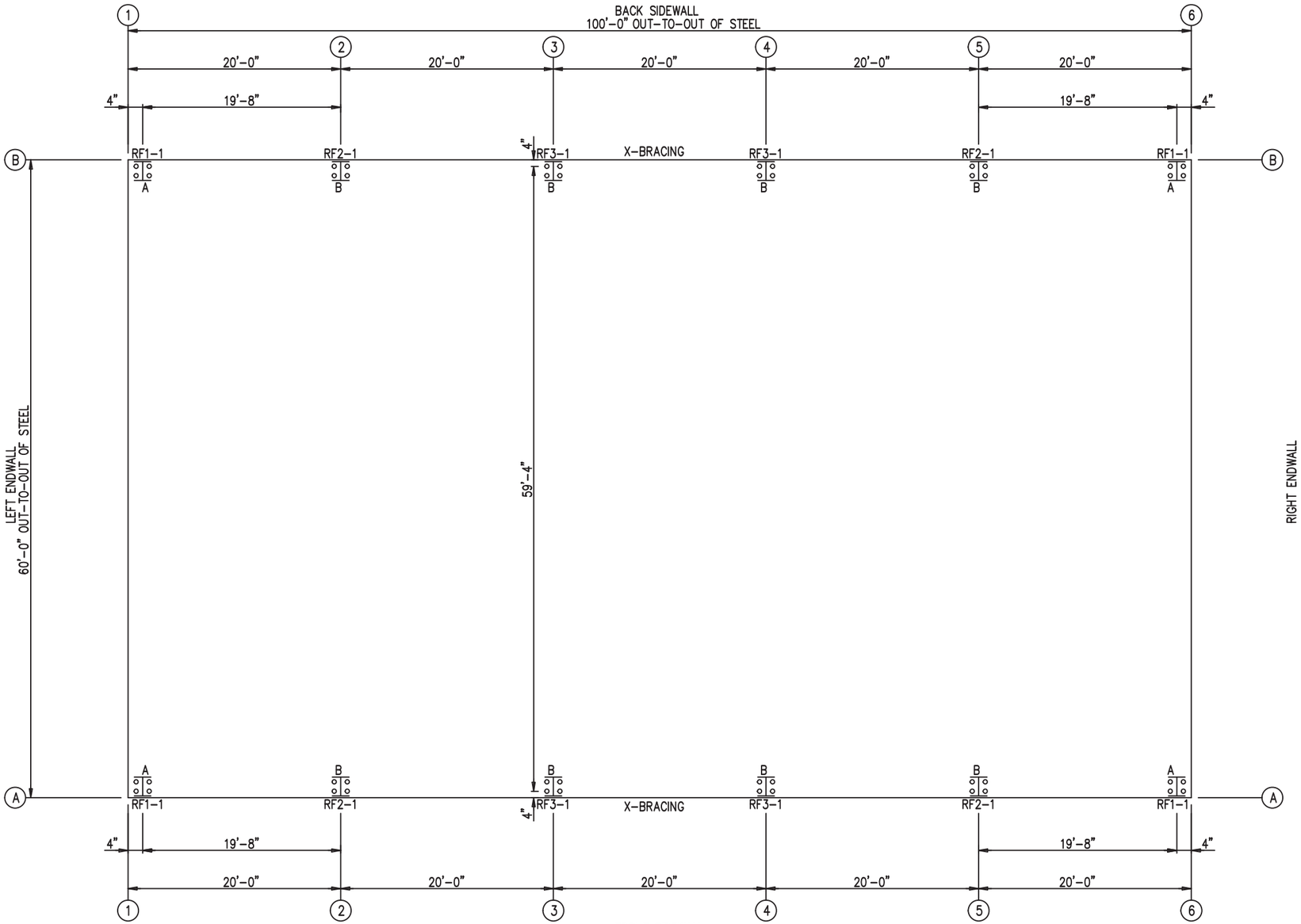

CUSTOMER NAME:
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE #:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 2 OF 15
JOB NUMBER 94431
SHEET TITLE: BUILDING INFO COVERSHEET

THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

## ANCHOR BOLT SUMMARY

| | QTY | LOCATE | DIA (in) | TYPE |
|---|---|---|---|---|
| ○ | 48 | FRAME | 3/4" | A307 |




## ANCHOR BOLT PLAN

NOTE: ALL BASE PLATES @ 100'-0" (U.N.)

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/9/21 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/21 | JMM | JMM | RTS |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CUSTOMER NAME:
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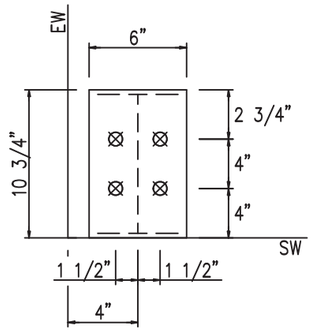
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE
CUSTOMER EMAIL:
SCALE: N.T.S.

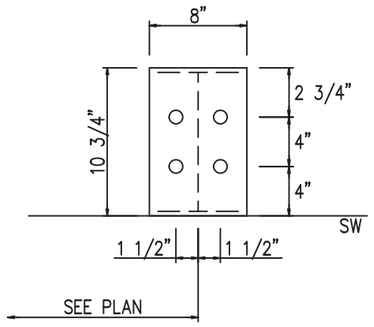
SHEET NUMBER: 3 OF 15
JOB NUMBER 94431
SHEET TITLE: ANCHOR BOLT PLAN

DIA= 3/4"




DETAIL A

DIA= 3/4"




DETAIL B

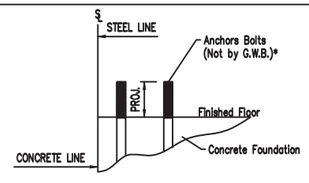
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

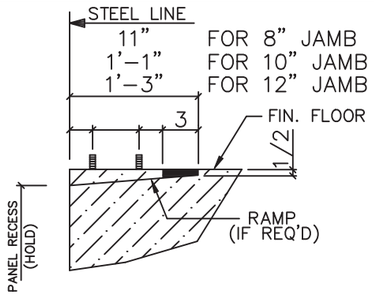
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

| Anchor Bolt Diameter | Projection |
|---|---|
| 1/2" | 1 1/2" |
| 5/8" | 2" |
| 3/4" | 2 1/2" |
| 7/8" | 3 1/2" |
| 1" | 3 1/2" |
| 1 1/8" | 3 1/2" |
| 1 1/4" | 3 1/2" |




CONCRETE NOTCH AND ANCHOR BOLT PROJECTION




SLOPE CONCRETE TO KEEP WATER OUT OF BUILDING

CONCRETE DETAIL AT OVERHEAD DOOR

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/31/23 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/23 | JMM | JMM | RTS |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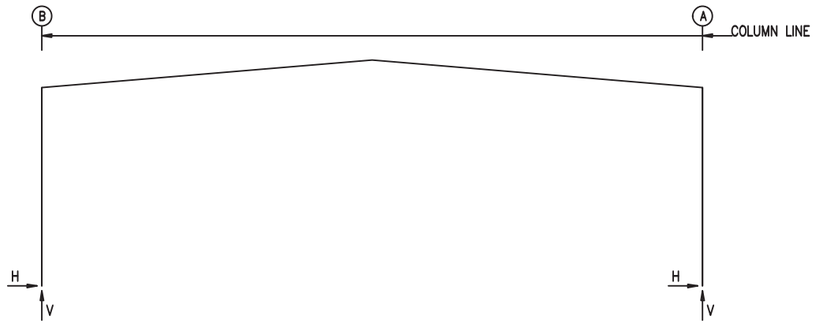
PROJECT END USE:
CUSTOMER PHONE:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 4 OF 15
JOB NUMBER 944331
SHEET TITLE: ANCHOR BOLT DETAILS

FRAME LINES: 1 2 3 4 5 6




## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) QTY | Bolt(in) DIA | Base_Plate(in) Width | Base_Plate(in) Length | Base_Plate(in) Thick | Grout (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Column_Reactions(k ) | | | | | | | | | | | |
| 1* | B | 1 | 6.4 | 10.8 | 6 | −2.5 | −3.8 | 4 | 0.750 | 6.000 | 10.75 | 0.375 | 0.0 |
| 1* | A | 6 | 2.5 | −3.8 | 1 | −6.4 | 10.8 | 4 | 0.750 | 6.000 | 10.75 | 0.375 | 0.0 |
| | | 1 | −6.4 | 10.8 | 6 | 2.5 | −3.8 | | | | | | |

1* FRAME lines: 1 6

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) QTY | Bolt(in) DIA | Base_Plate(in) Width | Base_Plate(in) Length | Base_Plate(in) Thick | Grout (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Column_Reactions(k ) | | | | | | | | | | | |
| 2* | B | 1 | 12.7 | 20.7 | 6 | −4.3 | −6.6 | 4 | 0.750 | 8.000 | 10.75 | 0.375 | 0.0 |
| 2* | A | 6 | 4.3 | −6.6 | 1 | −12.7 | 20.7 | 4 | 0.750 | 8.000 | 10.75 | 0.375 | 0.0 |
| | | 1 | −12.7 | 20.7 | 6 | 4.3 | −6.6 | | | | | | |

2* FRAME lines: 2 5

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) QTY | Bolt(in) DIA | Base_Plate(in) Width | Base_Plate(in) Length | Base_Plate(in) Thick | Grout (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Column_Reactions(k ) | | | | | | | | | | | |
| 3* | B | 1 | 12.7 | 20.7 | 3 | −3.7 | −4.0 | 4 | 0.750 | 8.000 | 10.75 | 0.375 | 0.0 |
| | | 2 | 11.4 | 21.6 | 5 | −2.9 | −8.5 | | | | | | |
| 3* | A | 4 | 3.7 | −4.0 | 1 | −12.7 | 20.7 | 4 | 0.750 | 8.000 | 10.75 | 0.375 | 0.0 |
| | | 2 | −11.4 | 21.6 | 5 | 2.9 | −8.5 | | | | | | |

3* FRAME lines: 3 4

## NOTES FOR REACTIONS

Building reactions are based on the following building data:

| | |
|---|---|
| Width (ft) | = 60.0 |
| Length (ft) | = 100.0 |
| Eave Height (ft) | = 18.0/18.0 |
| Roof Slope (rise/12) | = 1.0:12/1.0:12 |
| Dead Load (psf) | = 1.74 |
| Collateral Load (psf) | = 1.00 |
| Live Load (psf) | = 20.00 |
| Snow Load (psf) | = 30.0 |
| Wind Speed (mph) | = 115.0 |
| Wind Code | = IBC−21 |
| Exposure | = C |
| Closed/Open | = Open |
| Importance Wind | = 1.00 |
| Importance Seismic | = 1.00 |
| Seismic Zone | = D |
| Seismic Coeff (Fa*Ss) | = 0.67 |

| ID | Description |
|---|---|
| 1 | Dead+Collateral+Snow+Slide_Snow |
| 2 | Dead+Collateral+0.75Snow+0.45Wind_Long2R+0.75Slide_Snow |
| 3 | 0.6Dead+0.6Wind_Left1 |
| 4 | 0.6Dead+0.6Wind_Right1 |
| 5 | 0.6Dead+0.6Wind_Long1L |
| 6 | 0.6Dead+0.6Wind_Long1R |

## BUILDING BRACING REACTIONS

| Wall Loc | Wall Line | Col Line | ± Reactions(k ) Wind Horz | Wind Vert | Seismic Horz | Seismic Vert | Panel_Shear (lb/ft) Wind | Panel_Shear (lb/ft) Seis | Note |
|---|---|---|---|---|---|---|---|---|---|
| L_EW | 1 | | | | | | | | (h) |
| F_SW | A | 3,4 | 8.6 | 7.0 | 2.8 | 2.3 | | | |
| R_EW | 6 | | | | | | | | (h) |
| B_SW | B | 4,3 | 8.6 | 7.0 | 2.8 | 2.3 | | | |

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh

## ANCHOR BOLT SUMMARY

| | QTY | LOCATE | DIA (in) | TYPE |
|---|---|---|---|---|
| O | 48 | FRAME | 3/4" | A307 |

## RIGID FRAME: BASIC COLUMN REACTIONS (k )

| FRAME Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1* | B | 0.6 | 1.4 | 0.2 | 0.3 | 3.7 | 6.1 | 5.6 | 9.2 | −3.5 | −4.5 | −3.2 | −6.5 |
| 1* | A | −0.6 | 1.4 | −0.2 | 0.3 | −3.7 | 6.1 | −5.6 | 9.2 | 3.2 | −6.5 | 3.5 | −4.5 |

| FRAME Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1* | B | −3.2 | −6.3 | −3.5 | −4.8 | −4.7 | −7.8 | 2.0 | 3.3 | −0.4 | −0.2 | 0.4 | 0.2 |
| 1* | A | 3.5 | −4.8 | 3.2 | −6.3 | 4.7 | −7.8 | −2.0 | 3.3 | −0.4 | 0.2 | 0.4 | −0.2 |

| FRAME Line | Column Line | MIN_SNOW Horiz | MIN_SNOW Vert | F1UNB_SL_L Horiz | F1UNB_SL_L Vert | F1UNB_SL_R Horiz | F1UNB_SL_R Vert |
|---|---|---|---|---|---|---|---|
| 1* | B | 3.7 | 6.1 | 4.3 | 8.6 | 4.3 | 4.9 |
| 1* | A | −3.7 | 6.1 | −4.3 | 4.9 | −4.3 | 8.6 |

| FRAME Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | B | 1.0 | 2.1 | 0.4 | 0.6 | 7.5 | 12.0 | 11.3 | 18.0 | −7.1 | −8.8 | −6.4 | −12.9 |
| 2* | A | −1.0 | 2.1 | −0.4 | 0.6 | −7.5 | 12.0 | −11.3 | 18.0 | 6.4 | −12.9 | 7.1 | −8.8 |

| FRAME Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | B | −6.5 | −12.3 | −7.0 | −9.4 | −8.2 | −13.2 | 4.0 | 6.4 | −0.6 | −0.3 | 0.6 | 0.3 |
| 2* | A | 7.0 | −9.4 | 6.5 | −12.3 | 8.2 | −13.2 | −4.0 | 6.4 | −0.6 | 0.3 | 0.6 | −0.3 |

| FRAME Line | Column Line | MIN_SNOW Horiz | MIN_SNOW Vert | F2UNB_SL_L Horiz | F2UNB_SL_L Vert | F2UNB_SL_R Horiz | F2UNB_SL_R Vert |
|---|---|---|---|---|---|---|---|
| 2* | B | 7.5 | 12.0 | 8.8 | 17.0 | 8.8 | 9.7 |
| 2* | A | −7.5 | 12.0 | −8.8 | 9.7 | −8.8 | 17.0 |

| FRAME Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 3* | B | 1.0 | 2.1 | 0.4 | 0.6 | 7.5 | 12.0 | 11.3 | 18.0 | −7.1 | −8.8 | −6.4 | −12.9 |
| 3* | A | −1.0 | 2.1 | −0.4 | 0.6 | −7.5 | 12.0 | −11.3 | 18.0 | 6.4 | −12.9 | 7.1 | −8.8 |

| FRAME Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 3* | B | −6.5 | −12.3 | −7.0 | −9.4 | −5.8 | −16.3 | 3.1 | −2.1 | −0.6 | −0.3 | 0.6 | 0.3 |
| 3* | A | 7.0 | −9.4 | 6.5 | −12.3 | 5.8 | −16.3 | −3.1 | −2.1 | −0.6 | 0.3 | 0.6 | −0.3 |

| FRAME Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert | F3UNB_SL_L Horiz | F3UNB_SL_L Vert | F3UNB_SL_R Horiz | F3UNB_SL_R Vert |
|---|---|---|---|---|---|---|---|---|---|
| 3* | B | 0.0 | −2.3 | 7.5 | 12.0 | 8.8 | 17.0 | 8.8 | 9.7 |
| 3* | A | 0.0 | −2.3 | −7.5 | 12.0 | −8.8 | 9.7 | −8.8 | 17.0 |

1* FRAME lines: 1 6
2* FRAME lines: 2 5
3* FRAME lines: 3 4

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/23 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/23 | JMM | JMM | RTS |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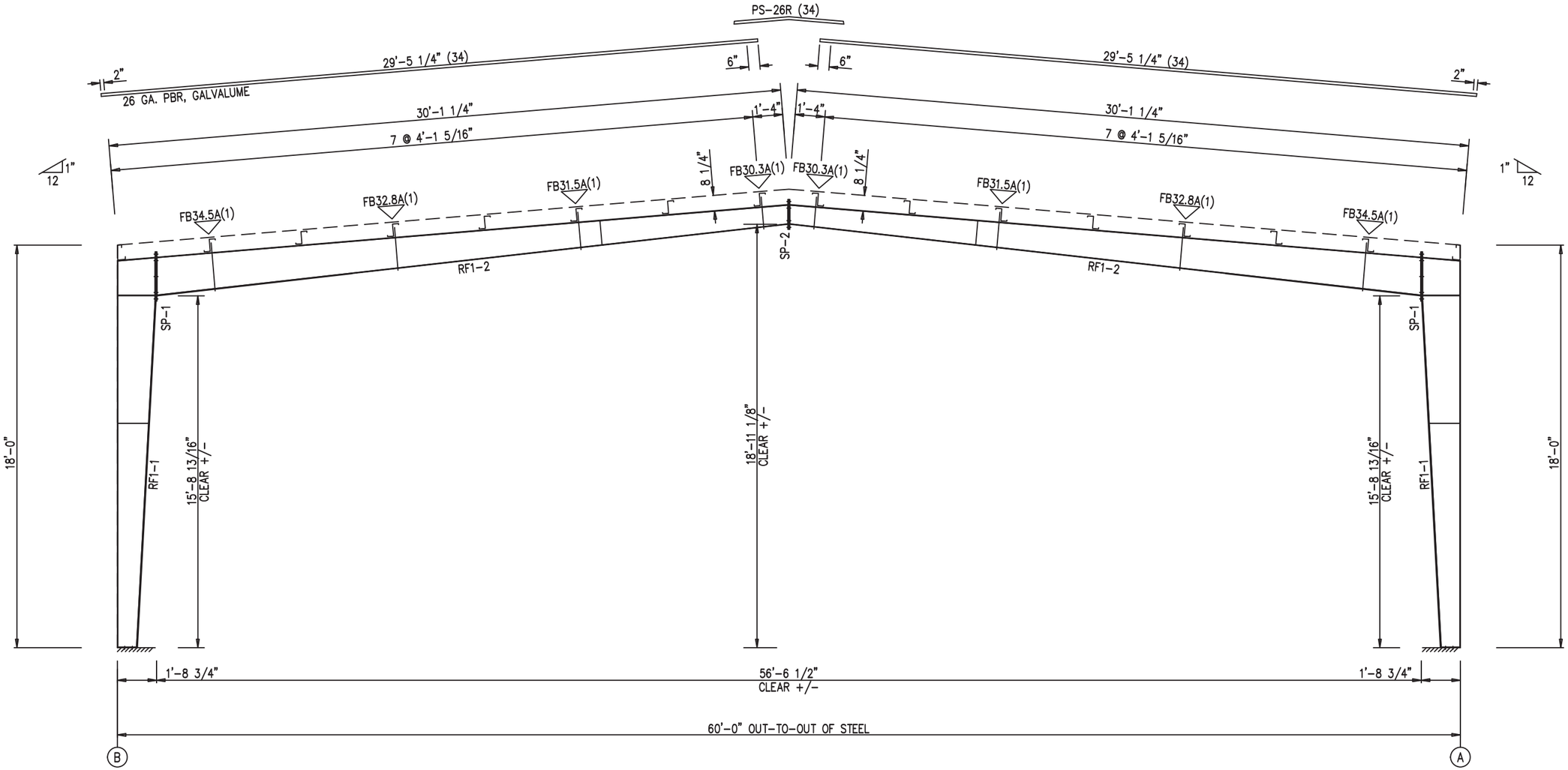
SHEET NUMBER: 5 OF 15
JOB NUMBER 94431
SHEET TITLE: ANCHOR BOLT REACTIONS

| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 2 | A325 | 3/4" | 2" |
| SP-2 | 4 | 4 | 0 | A325 | 5/8" | 2" |

| MEMBER TABLE | | | | | |
|---|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Web Plate Length | Outside Flange W x Thk x Length | Inside Flange W x Thk x Length |
| RF1-1 | 10.0/16.5 | 0.135 | 10'-0" | 6 x 1/4" x 17'-3 1/8" | 6 x 1/2" x 15'-5 3/4" |
| | 16.5/20.0 | 0.188 | 7'-4 13/16" | 6 x 1/4" x 1'-8 5/16" | |
| RF1-2 | 20.0/12.9 | 0.135 | 20'-0" | 5 x 1/4" x 28'-3 3/8" | 5 x 3/8" x 8'-9 1/8" |
| | 12.9/10.0 | 0.135 | 8'-5 1/16" | | 5 x 1/4" x 19'-7 1/4" |

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – 2X2X14Ga




RIGID FRAME ELEVATION: FRAME LINES 1 6

| | | | | |
|---|---|---|---|---|
| ENG. | RTS | | | |
| CHK. | MEZ | RTS | CAF | JMM |
| DWN. | MEZ | MEZ | AA | JMM |
| DATE | 11/9/21 | 11/9/21 | 12/27/23 | 10/26/21 |
| ISSUE | APPROVAL | | PERMIT | ERECTION |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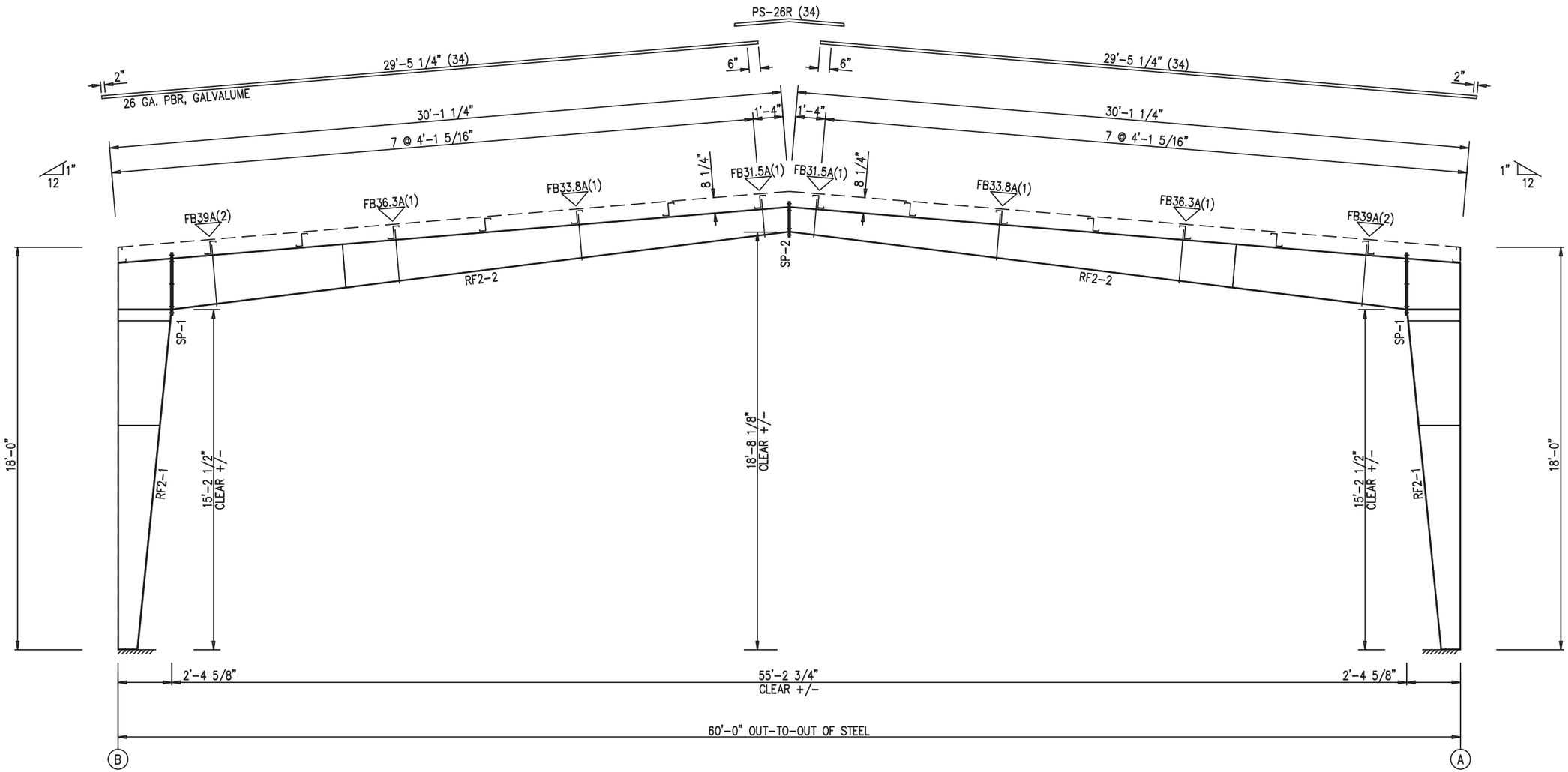
SHEET NUMBER: 6 OF 15
JOB NUMBER 94431
SHEET TITLE: RIGID FRAME ELEVATION

| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 2 | A325 | 3/4" | 2 1/4" |
| SP-2 | 4 | 4 | 0 | A325 | 5/8" | 2" |

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – 2X2X14Ga

| MEMBER TABLE | | | | | |
|---|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Web Plate Length | Outside Flange W x Thk x Length | Inside Flange W x Thk x Length |
| RF2-1 | 10.0/22.1 | 0.135 | 10'-0" | 8 x 1/4" x 17'-3 1/8" | 8 x 3/8" x 15'-0 1/16" |
| | 22.1/27.7 | 0.164 | 4'-8 3/16" | 8 x 1/4" x 2'-4 5/16" | |
| | 27.7/28.0 | 0.250 | 2'-9 1/4" | | |
| RF2-2 | 27.0/23.1 | 0.188 | 7'-9 3/8" | 6 x 1/4" x 27'-7 1/8" | 6 x 5/16" x 8'-1 1/2" |
| | 23.1/13.0 | 0.135 | 20'-0" | | 6 x 1/4" x 19'-7 1/8" |




RIGID FRAME ELEVATION: FRAME LINES 2 5

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/21 | MEZ | MEZ | RTS |
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| ERECTION | 10/25/21 | JMM | JMM | RTS |


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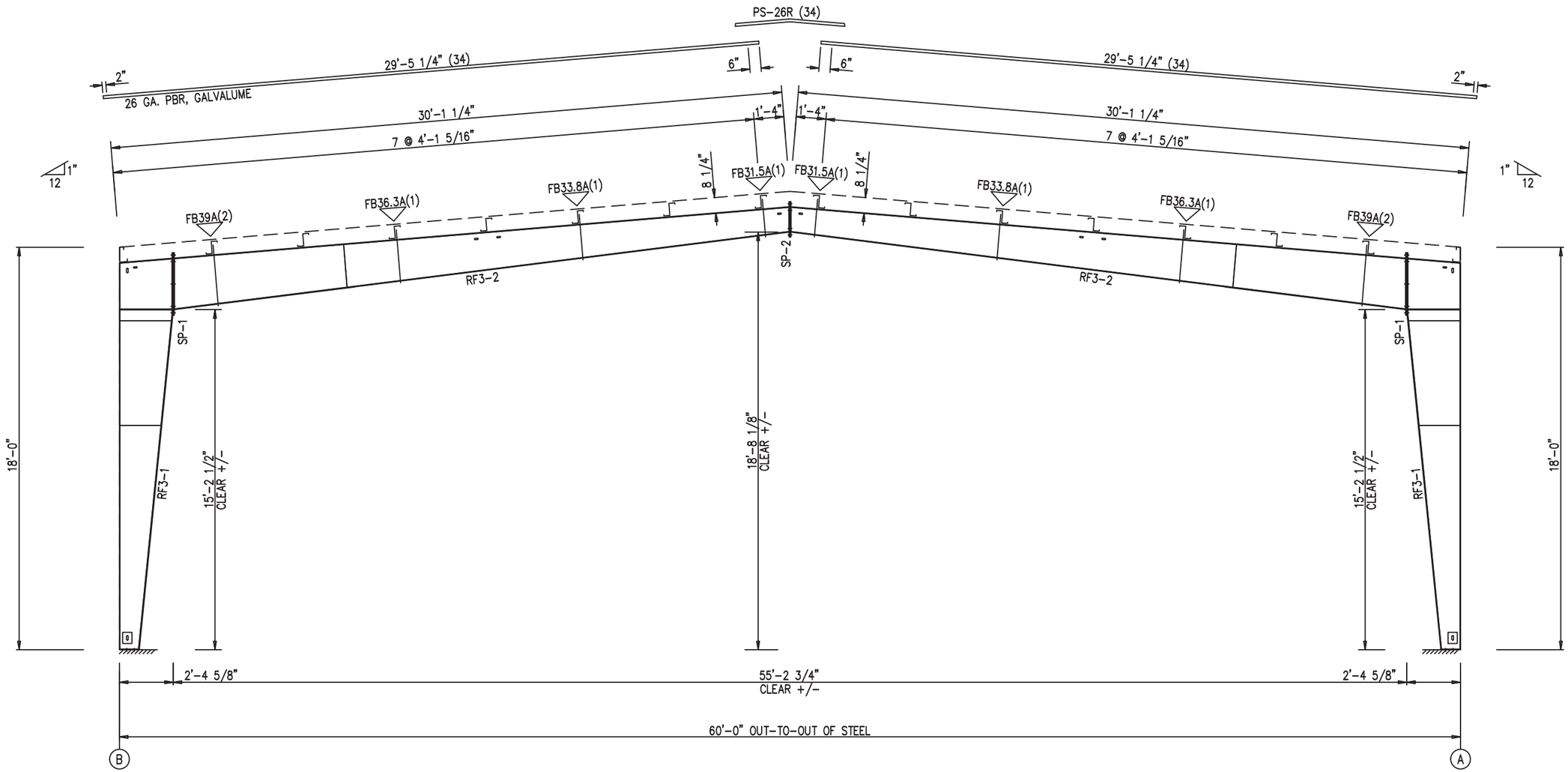
SHEET NUMBER: 7 OF 15
JOB NUMBER 94431
SHEET TITLE: RIGID FRAME ELEVATION

| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 2 | A325 | 3/4" | 2 1/4" |
| SP-2 | 4 | 4 | 0 | A325 | 5/8" | 2" |

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – 2X2X14Ga

| MEMBER TABLE | | | | | |
|---|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Web Plate Length | Outside Flange W x Thk x Length | Inside Flange W x Thk x Length |
| RF3-1 | 10.0/22.1 | 0.135 | 10'-0" | 8 x 1/4" x 17'-3 1/8" | 8 x 3/8" x 15'-0 1/16" |
| | 22.1/27.7 | 0.164 | 4'-8 3/16" | 8 x 1/4" x 2'-4 5/16" | |
| | 27.7/28.0 | 0.250 | 2'-9 1/4" | | |
| RF3-2 | 27.0/23.1 | 0.188 | 7'-9 3/8" | 6 x 1/4" x 27'-7 1/8" | 6 x 5/16" x 8'-1 1/2" |
| | 23.1/13.0 | 0.135 | 20'-0" | | 6 x 1/4" x 19'-7 1/8" |




RIGID FRAME ELEVATION: FRAME LINES 3 4

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/21 | MEZ | MEZ | RTS |
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| ERECTION | 10/25/21 | JMM | JMM | RTS |


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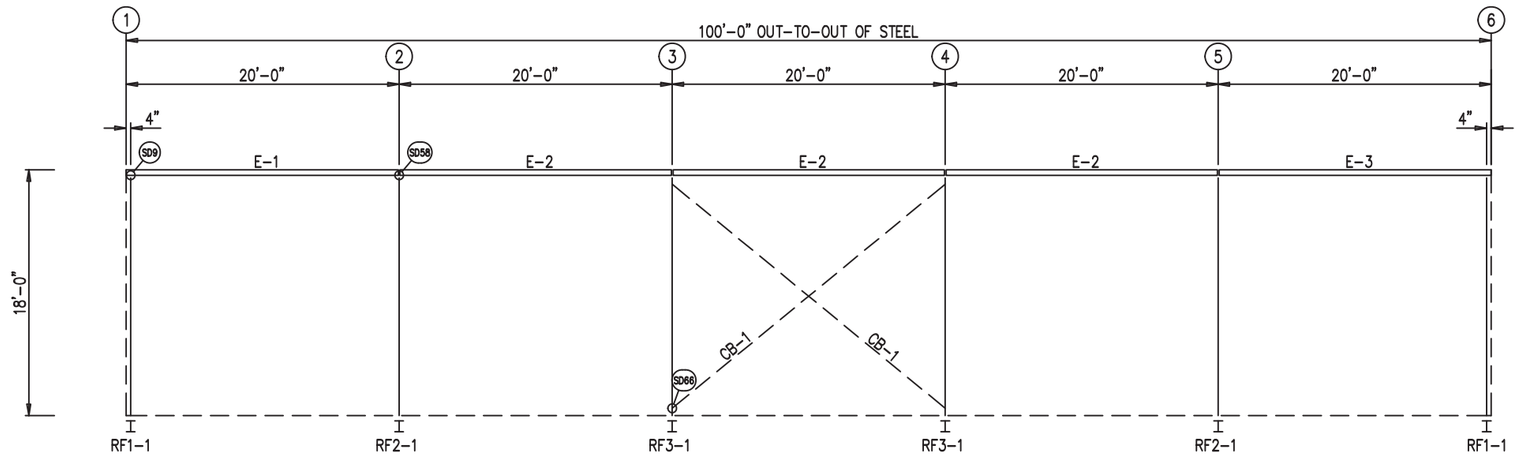
SHEET NUMBER: 8 OF 15
JOB NUMBER 94431
SHEET TITLE: RIGID FRAME ELEVATION



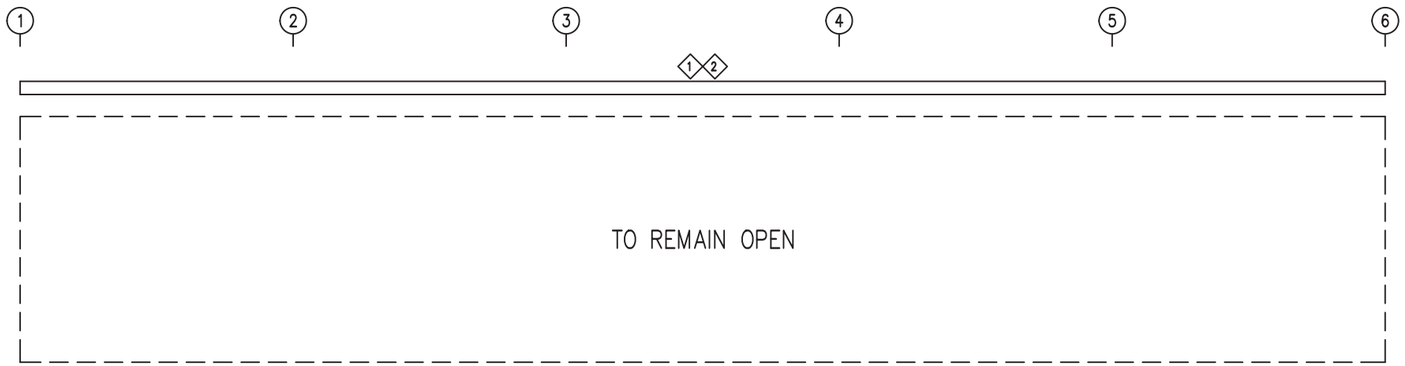

SIDEWALL FRAMING: FRAME LINE A

TRIM TABLE
FRAME LINE A

| ◇ID | QUAN | PART | LENGTH | DETAIL |
|---|---|---|---|---|
| 1 | 10 | FL-214 | 10'-3" | TD21 |
| 2 | 10 | FL-80 | 10'-3" | TD21 |

MEMBER TABLE
FRAME LINE A

| QUAN | MARK | PART | LENGTH |
|---|---|---|---|
| 1 | E-1 | L08E16-1 | 19'-11 1/2" |
| 3 | E-2 | L08E16-1 | 19'-11 1/2" |
| 1 | E-3 | L08E16-1 | 19'-11 1/2" |
| 2 | CB-1 | RD0750 | 26'-4 1/2" |




SIDEWALL SHEETING & TRIM: FRAME LINE A

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 01/31/23 | MEZ | MEZ | RTS |
| PERMIT | 02/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/23 | JMM | JMM | RTS |

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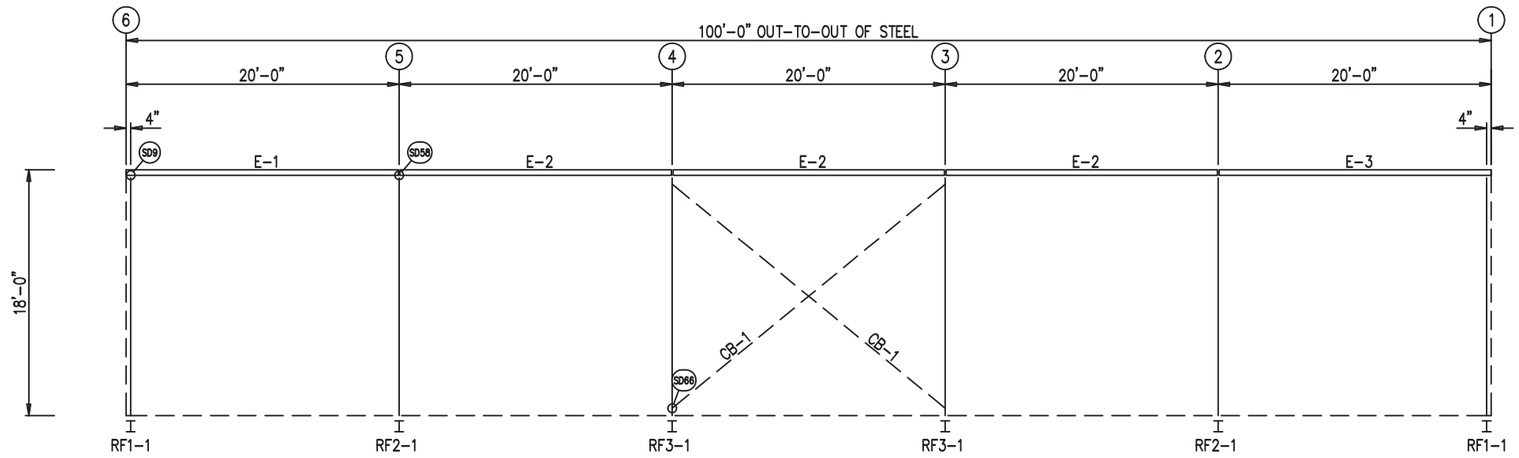
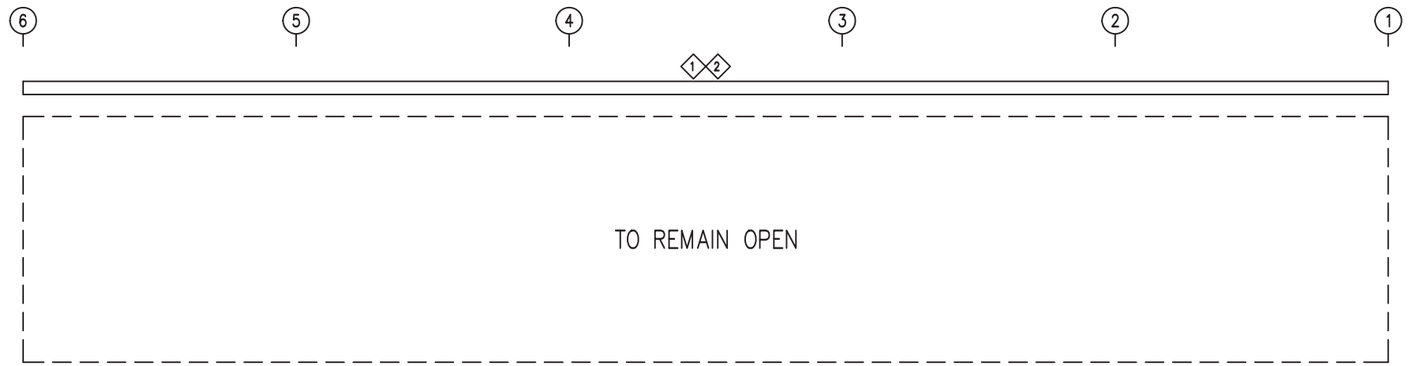
SHEET NUMBER: 9 OF 15
JOB NUMBER 94431
SHEET TITLE: SIDEWALL FRAMING & SHEETING




SIDEWALL FRAMING: FRAME LINE B




SIDEWALL SHEETING & TRIM: FRAME LINE B

TRIM TABLE
FRAME LINE B

| ◇ID | QUAN | PART | LENGTH | DETAIL |
|---|---|---|---|---|
| 1 | 10 | FL-214 | 10'-3" | TD21 |
| 2 | 10 | FL-80 | 10'-3" | TD21 |

MEMBER TABLE
FRAME LINE B

| QUAN | MARK | PART | LENGTH |
|---|---|---|---|
| 1 | E-1 | L08E16-1 | 19'-11 1/2" |
| 3 | E-2 | L08E16-1 | 19'-11 1/2" |
| 1 | E-3 | L08E16-1 | 19'-11 1/2" |
| 2 | CB-1 | RD0750 | 26'-4 1/2" |

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
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| ERECTION | 10/26/21 | JMM | JMM | RTS |

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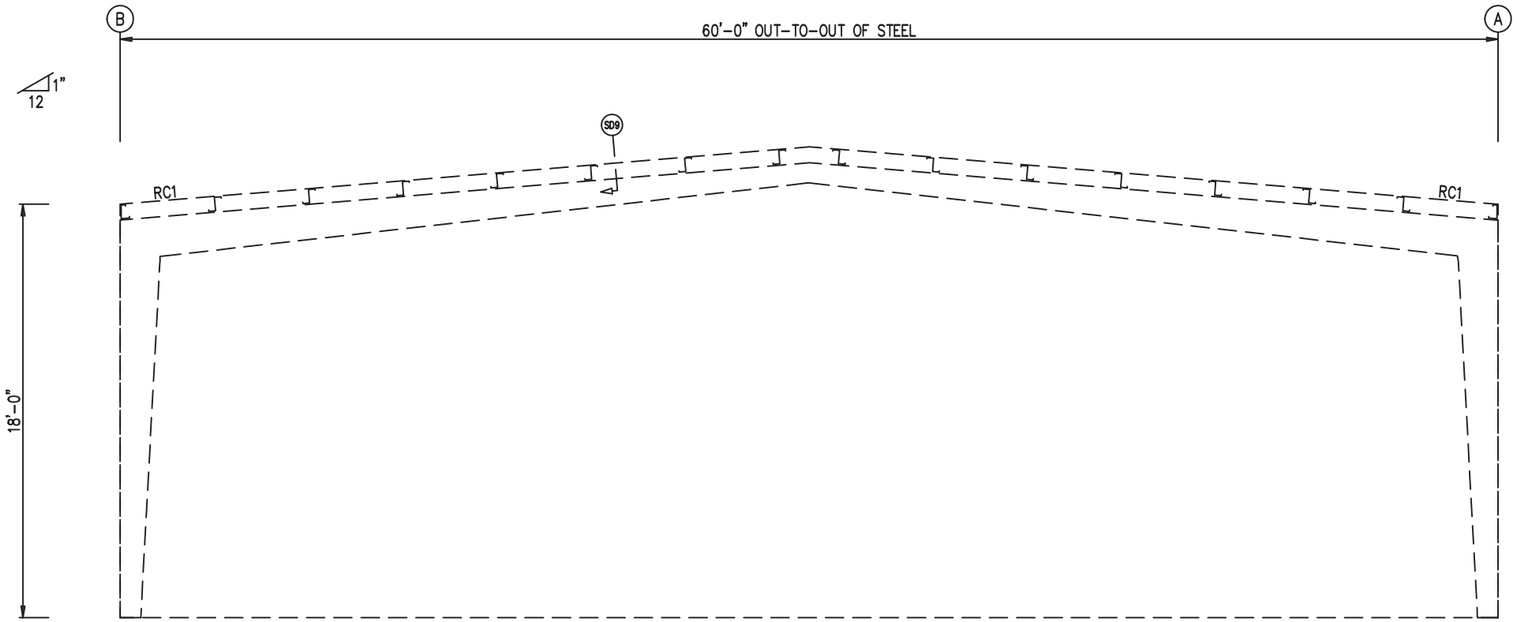
SHEET NUMBER: 10 OF 15
JOB NUMBER 94431
SHEET TITLE: SIDEWALL FRAMING & SHEETING

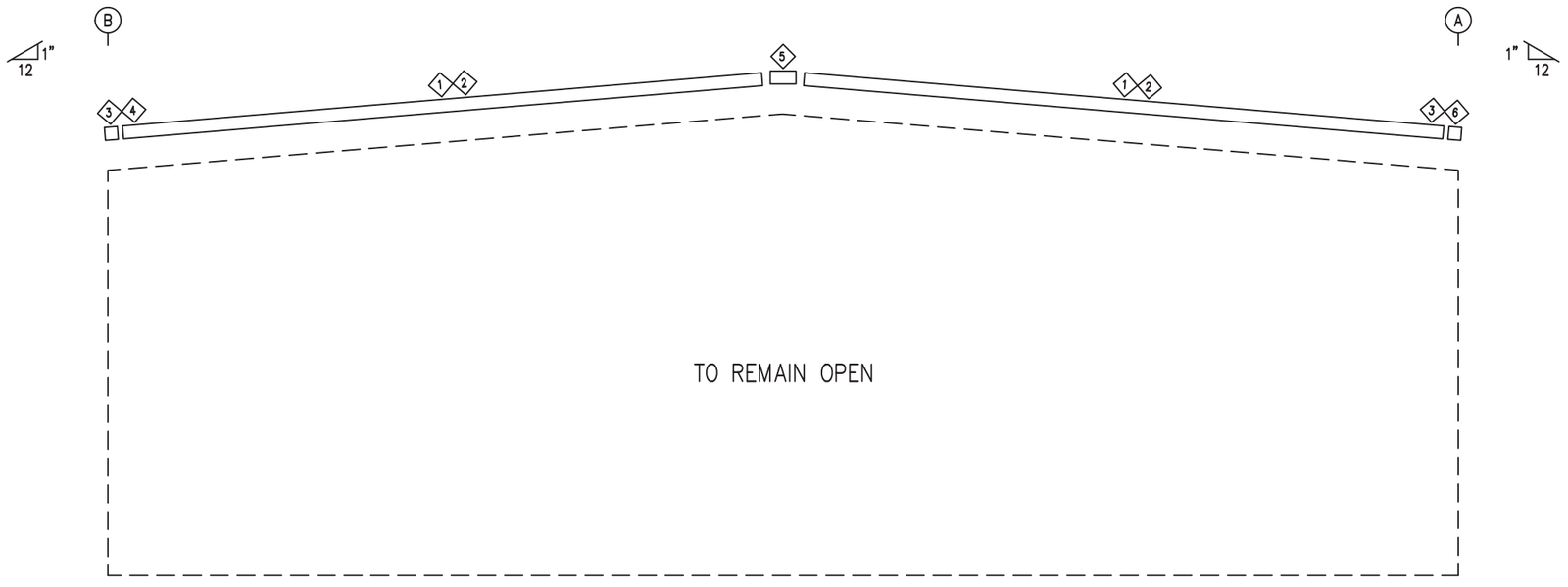



ENDWALL FRAMING: FRAME LINE 1

TRIM TABLE
FRAME LINE 1

| ◇ID | QUAN | PART | LENGTH | DETAIL |
|---|---|---|---|---|
| 1 | 6 | FL-15 | 10'-6" | TD25 |
| 2 | 6 | FL-78 | 10'-6" | TD25 |
| 3 | 2 | FL-601B | 7 3/4" | TD85 |
| 4 | 1 | FL-600L | 5 5/8" | TD12 |
| 5 | 1 | FL-17 | 1'-4" | |
| 6 | 1 | FL-600R | 5 5/8" | TD12 |

1" / 12




ENDWALL SHEETING & TRIM: FRAME LINE 1

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/21 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/25/21 | JMM | JMM | RTS |

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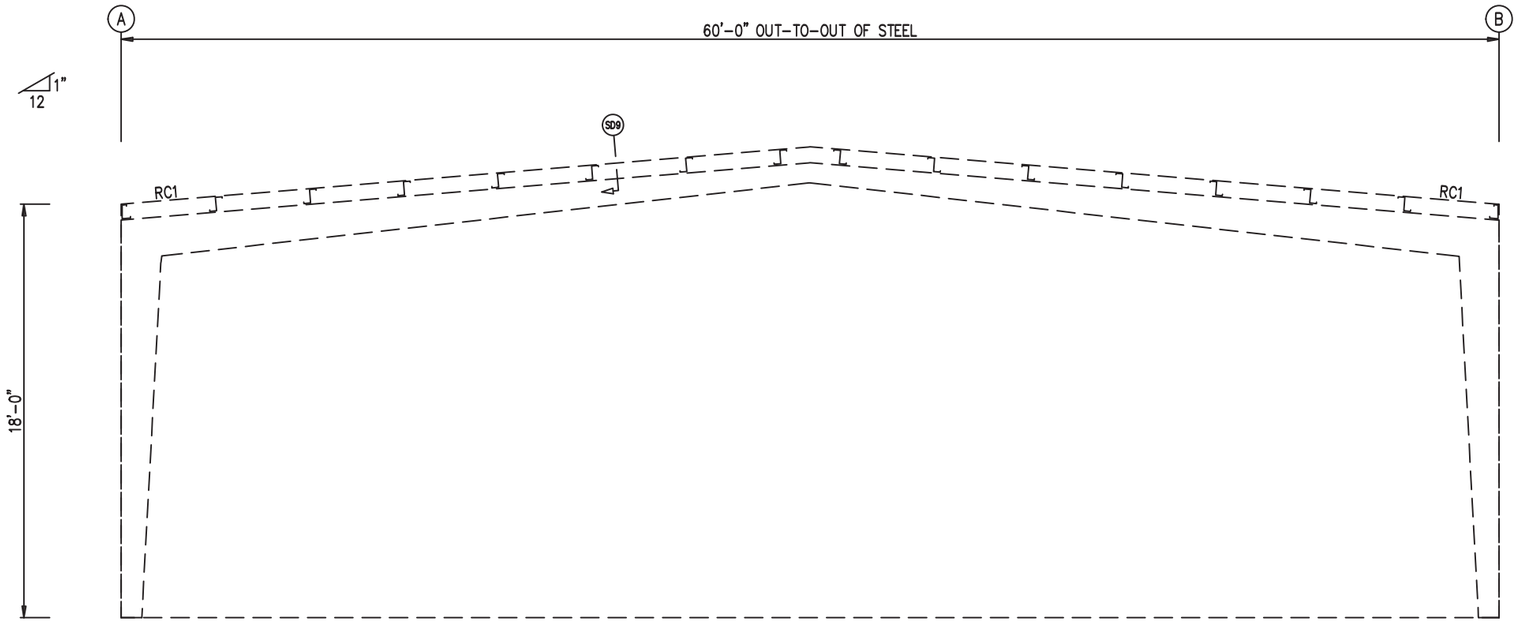
SHEET NUMBER: 11 OF 15
JOB NUMBER 94431
SHEET TITLE: ENDWALL FRAMING & SHEETING

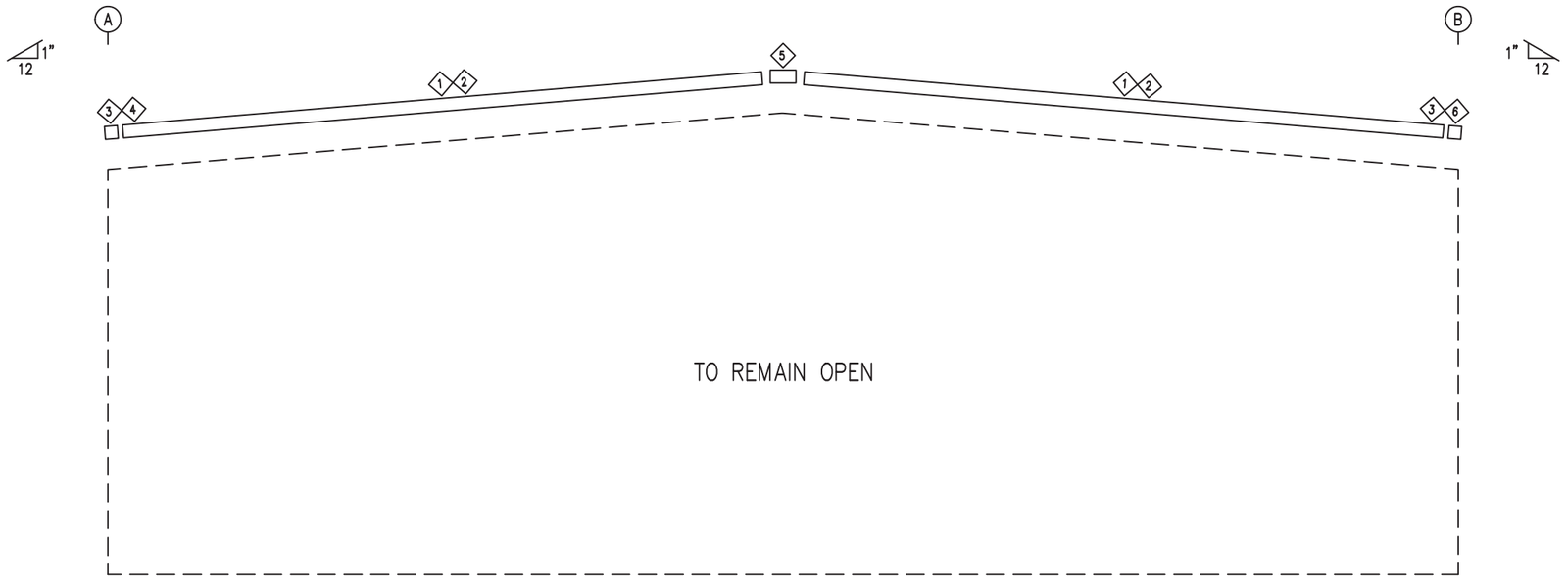



ENDWALL FRAMING: FRAME LINE 6

TRIM TABLE
FRAME LINE 6

| ◇ID | QUAN | PART | LENGTH | DETAIL |
|---|---|---|---|---|
| 1 | 6 | FL-15 | 10'-6" | TD25 |
| 2 | 6 | FL-78 | 10'-6" | TD25 |
| 3 | 2 | FL-601B | 7 3/4" | TD85 |
| 4 | 1 | FL-600L | 5 5/8" | TD12 |
| 5 | 1 | FL-17 | 1'-4" | |
| 6 | 1 | FL-600R | 5 5/8" | TD12 |

1" 12




ENDWALL SHEETING & TRIM: FRAME LINE 6

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/21 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/25/21 | JMM | JMM | RTS |

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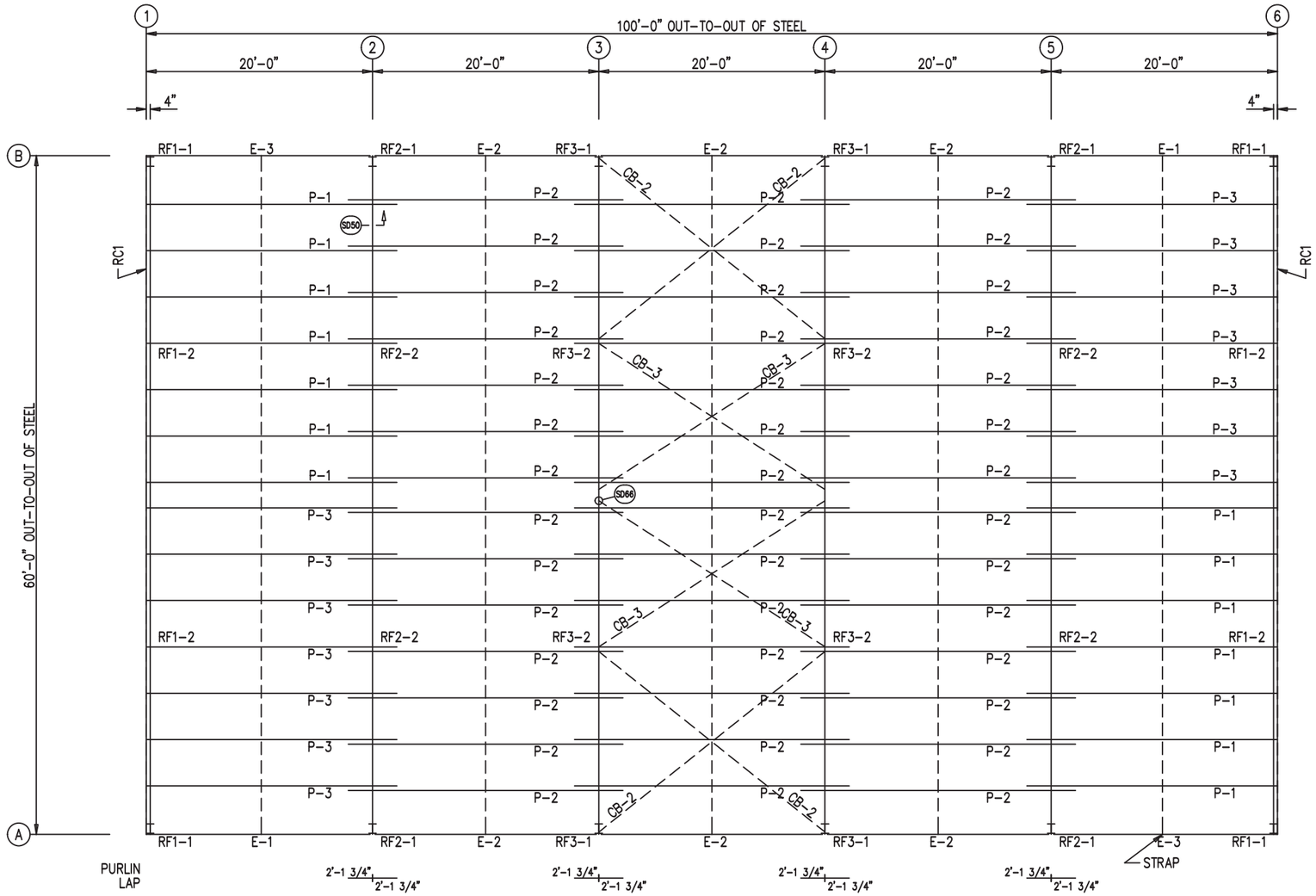
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE
CUSTOMER EMAIL:
SCALE: N.T.S.

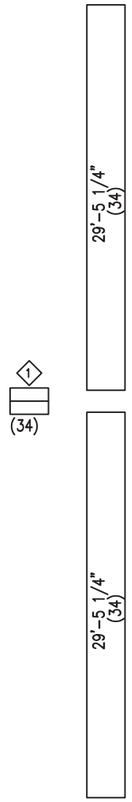
SHEET NUMBER: 12 OF 15
JOB NUMBER 94431
SHEET TITLE: ENDWALL FRAMING & SHEETING

| TRIM TABLE ROOF PLAN | | | | |
|---|---|---|---|---|
| ◇ID | QUAN | PART | LENGTH | DETAIL |
| 1 | 34 | PS-26R | 3'-0" | TD8 |

| MEMBER TABLE ROOF PLAN | | | |
|---|---|---|---|
| QUAN | MARK | PART | LENGTH |
| 14 | P-1 | 8X25Z14 | 22'-1 1/2" |
| 42 | P-2 | 8X25Z16 | 24'-3 1/2" |
| 14 | P-3 | 8X25Z14 | 22'-1 1/2" |
| 2 | E-1 | L08E16-1 | 19'-11 1/2" |
| 6 | E-2 | L08E16-1 | 19'-11 1/2" |
| 2 | E-3 | L08E16-1 | 19'-11 1/2" |
| 4 | CB-2 | RD0500 | 25'-8 1/4" |
| 4 | CB-3 | RD0500 | 24'-1 3/4" |




ROOF FRAMING PLAN




ROOF SHEETING

PANELS: 26 GA. PBR
GALVALUME

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/9/21 | MEZ | MEZ | RTS |
| PERMIT | 12/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/21 | JMM | JMM | RTS |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| | |
|---|---|
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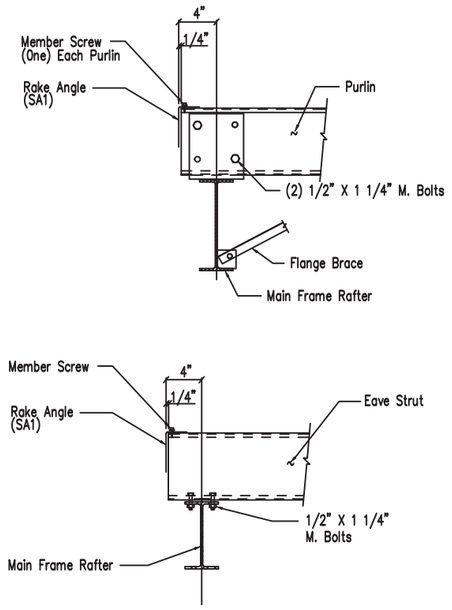
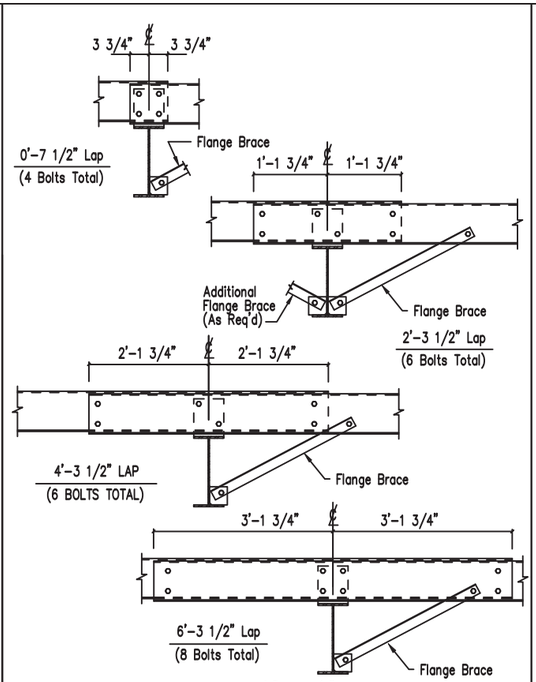
| PROJECT NAME: | |
| PROJECT LOCATION: | |
| PROJECT COUNTY: | |
| PROJECT END USE: | |
| CUSTOMER PHONE | |
| CUSTOMER EMAIL: | |
| SCALE: | N.T.S. |
| SHEET NUMBER: | 13 OF 15 |
| JOB NUMBER | 94431 |
| SHEET TITLE: | ROOF FRAMING & SHEETING PLAN |




## Main Frame Rafter Connection
PURLIN & EAVE STRUT CONNECTION

DRAWING NO. SD9

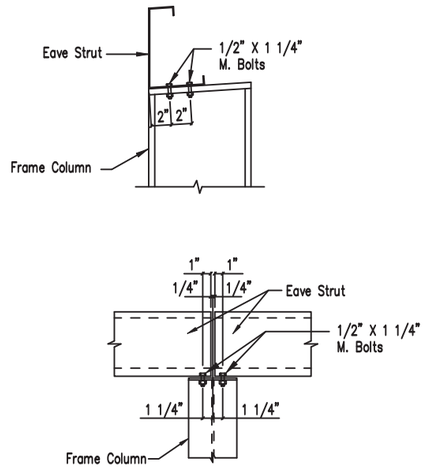
Created On: 7/22/11




Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

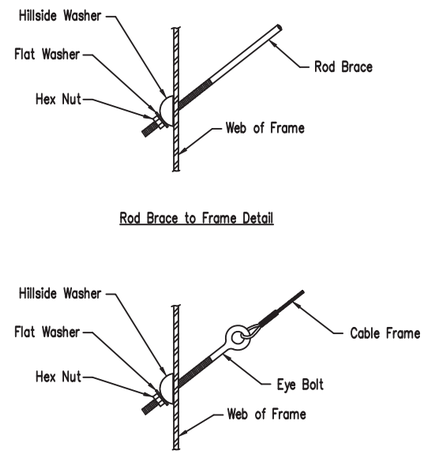
## Interior Bay Purlin Framing

DRAWING NO. SD50




## Eave Strut at Interior Column
Flush Sidewall

DRAWING NO. SD58



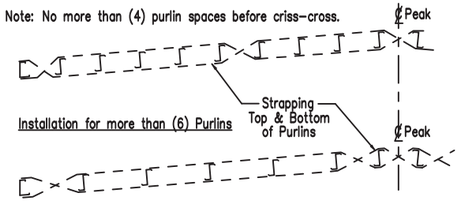

Rod Brace to Frame Detail

Cable Brace to Frame Detail

## Cable or Rod Brace to Frame Connection

DRAWING NO. SD66

Note: No more than (4) purlin spaces before criss-cross.

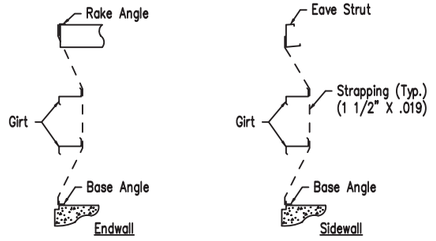



Installation for more than (6) Purlins

Installation for (6) or less Purlins

Roof Uplift Strap Installation
(Refer to Roof Plan for Locations)

Note: 1) Attach straps w/#10-16 x 1" pancake self driller (RF1) at purlins or girts.
2) No criss-cross straps in walls.




Wall Suction Strap Installation
(Refer to Wall Elevations for Location)

## Roof Uplift and Wall Suction Strap Details

DRAWING NO. SD102

Created On: 9/28/12

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/3/23 | MEZ | MEZ | RTS |
| PERMIT | 02/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/23 | JMM | JMM | RTS |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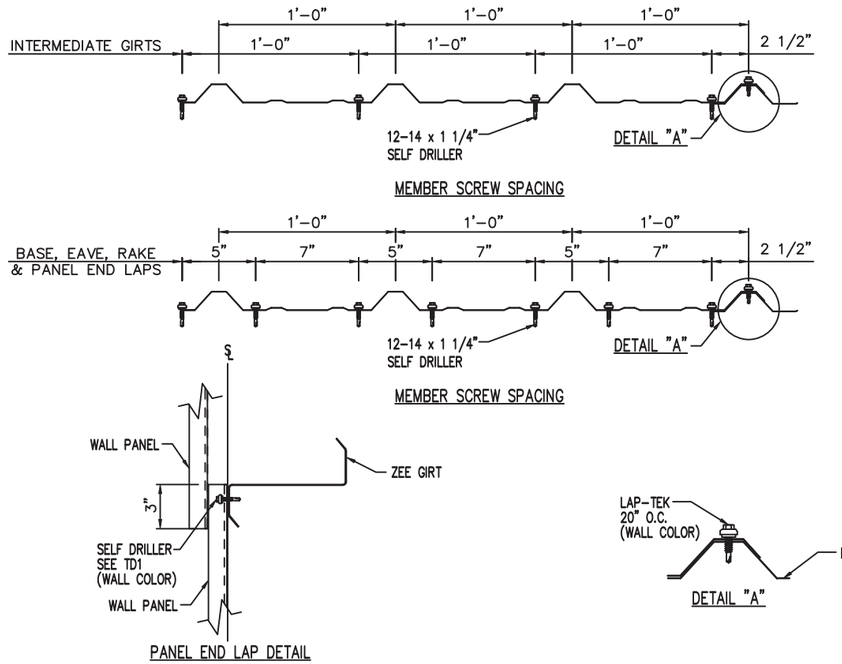
PROJECT END USE:
CUSTOMER PHONE:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 14 OF 15

JOB NUMBER 944331

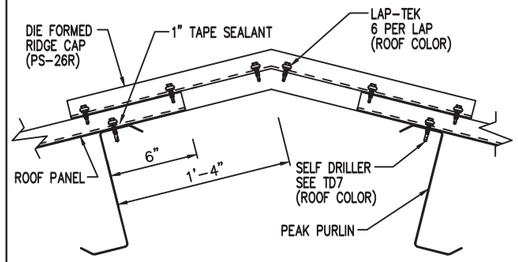
SHEET TITLE: DETAIL DRAWINGS




## Fastener Location at Wall – PBR

DRAWING NO. TD1

Created On: 08/13/18

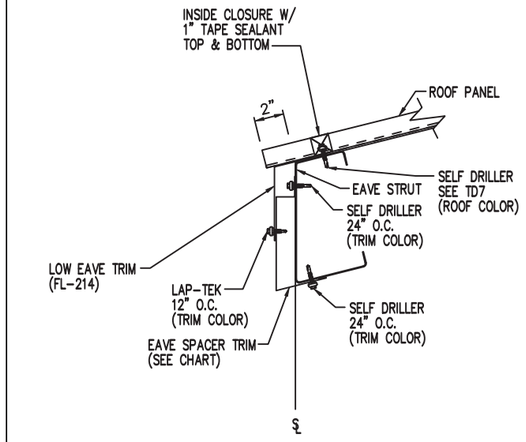



## Die Formed Ridge Detail – PBR

Up to a 4:12 Roof Slope

DRAWING NO. TD8

Created On: 08/13/18




| TRIM CHART | |
|---|---|
| PURLIN SIZE | PIECE MARK |
| 8" | FL-80 |
| 10" | FL-80 |
| 12" | FL-500 |

## Low Eave Detail – PBR

Simple Eave – Open Wall – No Soffit

DRAWING NO. TD21

Created On: 08/08/18

| ISSUE | DATE | DWN. | CHK. | ENG. |
|---|---|---|---|---|
| APPROVAL | 11/31/23 | MEZ | MEZ | RTS |
| PERMIT | 02/27/23 | AA | CAF | RTS |
| ERECTION | 10/26/23 | JMM | JMM | RTS |

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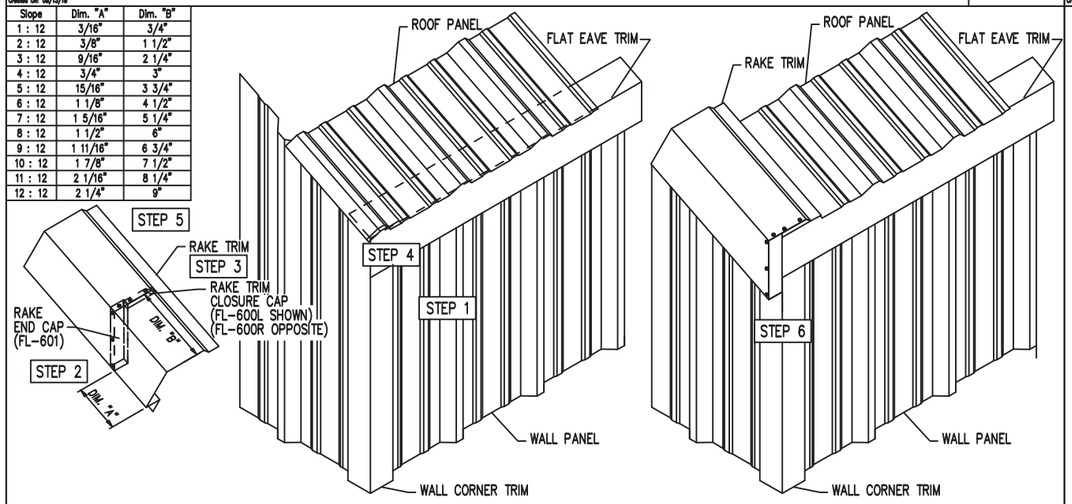
| Slope | Dim. "A" | Dim. "B" |
|---|---|---|
| 1 : 12 | 3/16" | 3/4" |
| 2 : 12 | 3/8" | 1 1/2" |
| 3 : 12 | 9/16" | 2 1/4" |
| 4 : 12 | 3/4" | 3" |
| 5 : 12 | 15/16" | 3 3/4" |
| 6 : 12 | 1 1/8" | 4 1/2" |
| 7 : 12 | 1 5/16" | 5 1/4" |
| 8 : 12 | 1 1/2" | 6" |
| 9 : 12 | 1 11/16" | 6 3/4" |
| 10 : 12 | 1 7/8" | 7 1/2" |
| 11 : 12 | 2 1/16" | 8 1/4" |
| 12 : 12 | 2 1/4" | 9" |




**STEP 1** INSTALL FLAT EAVE TRIM IN BETWEEN THE ROOF PANEL AND LOW EAVE MEMBER. BE SURE THE END OF THE FLAT EAVE TRIM IS FLUSH WITH THE WALL CORNER TRIM.

**STEP 2** INSTALL RAKE END CAP (FL-601), INTO RAKE TRIM USING (5) POP RIVETS. USE CHART TO DETERMINE HOW FAR THE RAKE END CAP IS POSITIONED INTO THE RAKE TRIM. WITH DIM. "A" BEING THE BOTTOM FACE OF THE RAKE TRIM & DIM. "B" BEING THE TOP FACE OF THE RAKE TRIM.

**STEP 3** INSTALL RAKE TRIM CLOSURE CAP (FL-600L/R), FLUSH WITH THE RAKE END CAP USING (3) POP RIVETS.

**STEP 4** FIELD CUT/NOTCH THE END OF THE ROOF PANEL BACK 1". THIS IS TO ALLOW THE RAKE TRIM CLOSURE CAP FROM HITTING THE ROOF PANEL.

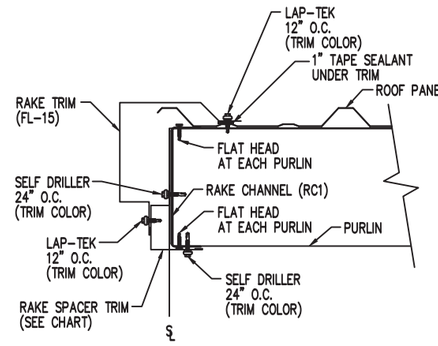
**STEP 5** INSTALL RAKE TRIM. BE SURE THE BOTTOM END OF THE RAKE TRIM, THAT ATTACHES TO THE HIGH RIBS OF THE WALL PANEL, IS FLUSH WITH THE WALL CORNER TRIM.

**STEP 6** FIELD CUT/NOTCH THE FACE OF THE RAKE TRIM TO BE FLUSH WITH THE WALL CORNER TRIM. BE SURE WHEN CUTTING/NOTCHING THE TOP OF THE RAKE TRIM, THAT SITS ON TOP OF THE ROOF PANEL, SHOULD BE LINED UP WITH THE END OF THE ROOF PANEL.

## Flat Eave Corner Trim Installation – PBR

DRAWING NO. TD12

Created On: 08/01/23




| TRIM CHART | |
|---|---|
| PURLIN SIZE | PIECE MARK |
| 8" | FL-78 |
| 10" | FL-365 |
| 12" | FL-366 |

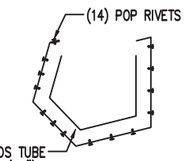
## Rake Detail – PBR

Box Rake – Open Wall – No Soffit

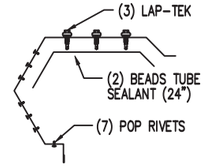
DRAWING NO. TD25

Created On: 08/08/18

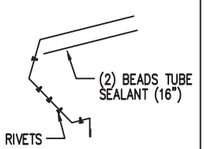
EAVE GUTTER



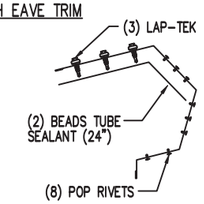

RAKE TRIM



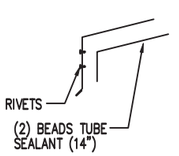

LOW EAVE TRIM



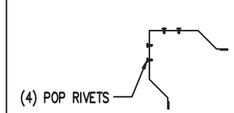

HIGH EAVE TRIM




LOW EAVE TRIM




CORNER TRIM




NOTE: TRIM PROFILES MAY VARY; 2" MIN. LAP UNLESS NOTED
NOTE: TUBE SEALANT (NOT BY G.W.B.)

## Trim Laps – PBR Sculptured

DRAWING NO. TD85

Created On: 08/08/18

CUSTOMER NAME:
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 15 OF 15
JOB NUMBER 94431
SHEET TITLE: DETAIL DRAWINGS