

BUILDING LOADS GENERAL NOTES 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 DESIGN LOADS: FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, ASTM DESIGNATION MIN. YIELD STRENGTH I.2 MATERIALS HOT ROLLED STEEL SHAPES (W. & C) HOT ROLLED STEEL ANGLES (L) STEEL PIPES A572 Fy = 50 KSIFy = 36 KSI Fy = 42 KSI Fy = 42 KSI 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S. STRUCTURAL TUBING A500 STRUCTURAL TUBING A500 STRUCTURAL STEEL WEB PLATE A572/A1011 STRUCTURAL STEEL FLANCE PLATES/BARS A529/A572 COLD FORMED LIGHT GAGE A653/A1011 ROOF & WALL SHEETS A792/A653 CABLE BRACE A475 — TYPE ROD BRACE A36 Fy = 55 KSIFv = 55 KSLA792/A653 A475 - TYPE 1 Fy = 50, 80 KSI EXTRA HIGH STRENGTH Fy = 36 KSI2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION. THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD MIN. TENSILE STRENGTH MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1"% & LESS) HIGH STRENGTH BOLTS (>1"% TO 1 1/2"%) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554 Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI Fu = 60 KSIPRACTICE, LATEST EDITION.) 9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE PRIMER SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION). NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT DOCUMENTS. CONCRETE. CONCRETE. UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROMSIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MAINIAI) 1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS SYSTEMS MANUAL) 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION) 1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT : A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325 NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING 1.6 A325 BOLT TIGHTENING REQUIREMENTS

- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>WARNING</u> IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY, MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002
EDITION. SECTION AB.4. PAGE XI-AB-2

DESIGN LOADS:		MEZ
DESIGN CODE / WIND CODE	: IBC-18	
OCCUPANCY / RISK CATEGORY ENCLOSURE	: II—Normal : Closed	DATE 0/27/22 10/28/22
ROOF DEAD LOAD (D) (PSF)	: 2.00	
ROOF COLLATERAL LOAD (C) (PSF)	:1.00	_ _
WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH)	:115.00	APPROVAL PERMIT
WIND EXPOSURE CATEGORY	: 115.00 : C	APPROVI
INTERNAL PRESSURE COEFFICIENT, (GCpi)	: 0.18/-0.18	
WALL PANEL DESIGN WIND PRESSURE (PSF) WIND ENCLOSURE CLASSIFICATION	: 28.30/-30.70 : Closed	
LIVE LOAD	: Closeu	
PRIMARY FRAMING (PSF)	: 20.00	FLOOR INGS.CC
TRIB. AREA REDUCTION	: No	
SECONDARY FRAMING (PSF) SNOW LOAD	: 20.00	12 F 2135 UILDIN
GROUND SNOW LOAD, (Pg) (PSF)	:15.00	
ROOF SNOW LOAD, (Pf) (PSF)	:15.00	ROB 14 1 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
SNOW EXPOSURE FACTOR, (Ce)	:1.00	KER RE 80014 90014 SSTERNE
SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct)	: 1.00 : 1.00	DARKER R CO 80014 800)-497 (TWESTERN
SEISMIC LOAD		3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 WWW.GREATWESTERNBUILDINGS.COM
SEISMIC IMPORTANCE FACTOR, (Ie)	:1.00	S. S. (2) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4
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SPECTRAL RESPONSE COEFFICIENTS	3S = 0.307 $31 = 0.09331 = 0.318$ $31 = 0.149$	3033 AUROI PHONI WWW.C
SEISMIC DESIGN CATEGORY	:C	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
BASIC SEISMIC FORCE RESISTING SYSTEM	: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE	OVER!
	:RIGID FRAMES (OMF)	
TOTAL DECICAL DACE CHEAD (V) (VIDE)	:BRACED FRAMES (OCBF/OMF)	
TOTAL DESIGN BASE SHEAR, (V) (KIPS)	:LONGITUDINAL = 1.23 :TRANSVERSE = 1.28	
RESPONSE MODIFICATION FACTORS, (R)	:RIGID FRAMES = 3.00 Ω = 3.00	
SEISMIC RESPONSE COEFFICIENTS, (Cs)	:RIGID FRAMES = 0.1061	
ANALYSIS PROCEDURE USED		
OTHER LOADS/REQUIREMENTS	: EQUIVALENT LATERAL FORCE PROCEDURE	S. T.
Office Edition Regulation		
	THIS BUILDING IS DESIGNED USING PANEL SHEAR BRACING.	ISTOMER NAME: TOLECT LOCATION TOLECT COUNTY: TOLECT COUNTY: TOLECT TOLUGE: TOLECT TOLUGE: TOLECT NUMBER: TOLECT NUMBER: TOLECT TOLUGE: TOLUGE: TOLECT TOLUGE: TOLU
	NO ADDITIONAL OPENINGS ARE TO BE ADDED WITHOUT CONSULTING THE ORIGINAL DESIGN ENGINEER OR A LOCAL	TI IN IN IN IT IN
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ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION. STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWNOS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS
OUTSIDE— BETWEEN ROOF PANELS & RIDGE CAP
— BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

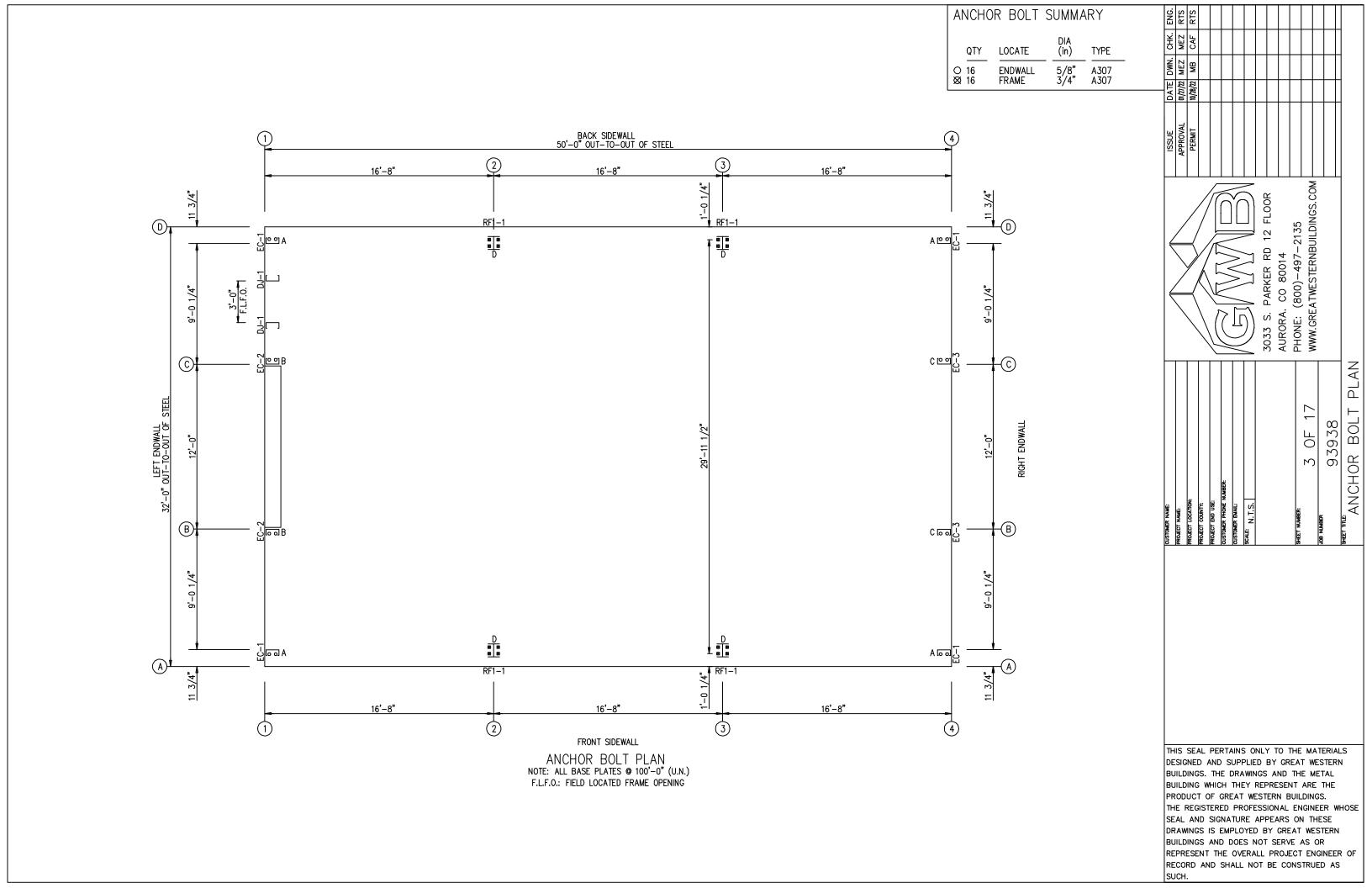
1.9 ERECTION AND UNLOADING NOT BY G.W.B.

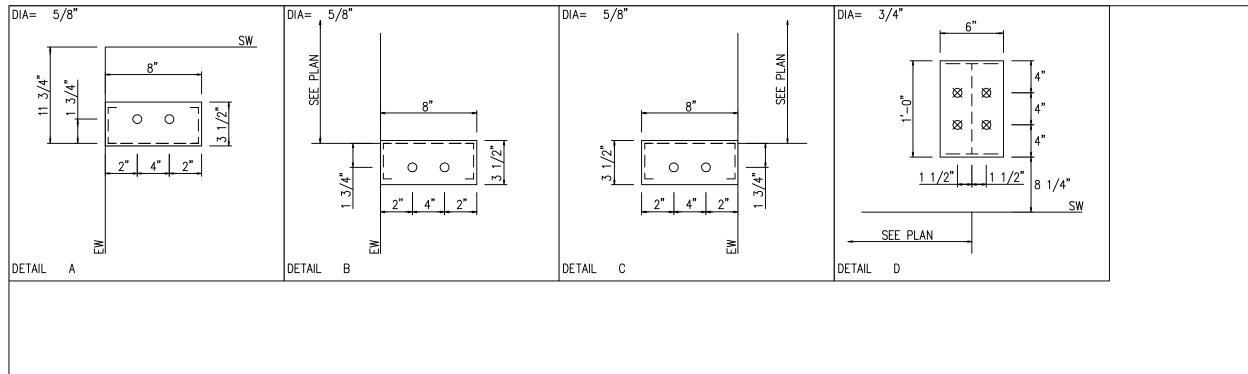
1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF TRAD ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- 2.2 METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- 2.3 IN CASE OF DISCREPANCES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST FOLION)
- 2.4 APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MEMA 3.3.3)
- 2.5 ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.



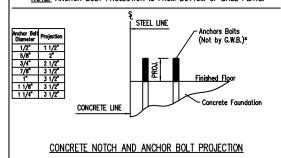


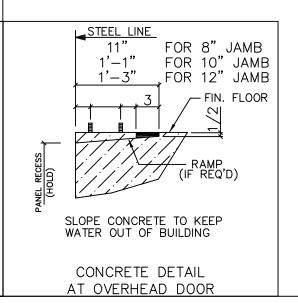
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

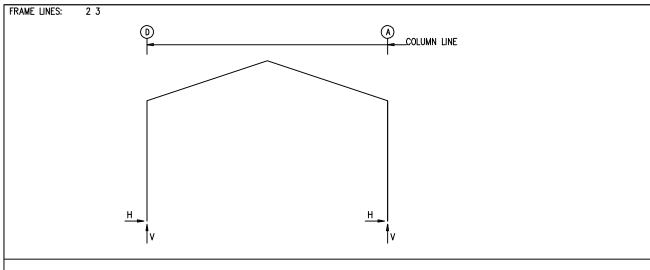
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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RIGID	FRAME:		MAXIMUM	REACTION	IS, ANC	HOR BOLT	S, & BAS	E PLATI	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	3 1	2.1 1.5	4.2 7.3	6	-2.2 -1.8	-2.5 -3.9	4	0.750	6.000	12.00	0.375	0.0
2*	Α	7 1	2.2 -1.5	-2.5 7.3	2 5	-2.1 1.8	4.2 -3.9	4	0.750	6.000	12.00	0.375	0.0
2*	FRAME li	nes:	2 3										

RIGID FRAME: BASIC COLUM					IMN REACTIONS (k)									
FRAME Line 2* 2*	Column Line D A	Horiz 0.2 -0.2	-Dead Vert 1.0 1.0		oteral- Vert 0.3 0.3	Horiz 1.2 -1.2	Live Vert 6.0 6.0	Horiz 0.9 -0.9	-Snow Vert 4.5 4.5	Wind Horiz -3.2 -2.1	_Left1- Vert -7.6 -3.6	-Wind_ Horiz 2.1 3.2	Right1— Vert -3.6 -7.6	
FRAME Line 2* 2*	Column Line D A	Wind Horiz -3.9 -1.4	_Left2- Vert -5.3 -1.3	-Wind_ Horiz 1.4 3.9	Right2- Vert -1.3 -5.3	Wind Horiz 1.4 -0.8	_Long1- Vert -4.7 -4.4	Wind Horiz 0.8 -1.4	I_Long2- Vert -4.4 -4.7	-Seism Horiz -0.2 -0.2	ic_Left Vert -0.2 0.2	Seismic Horiz 0.2 0.2	_Right Vert 0.2 -0.2	
FRAME Line 2* 2*	Column Line D A	F1UNB_ Horiz 0.8 -0.8	SL_L- Vert 4.3 2.4	F1UNB_ Horiz 0.8 -0.8	SL_R- Vert 2.4 4.3									
2* F	FRAME lir	nes:	2 3											

IOTES	FOR	REACTIO	NS		
	followir Width Length Eave I Roof S Dead Collate Live L Snow Ultimo Wind C Expose Import Import Seismi	r (ft) Height (ft) Slope (rise/12 Load (psf) eral Load (psf) boad (psf) Load (psf) te Wind Spee Code	ta: ;) d (mph)		Closed 1.00 1.00
ID	Descrip	otion			
1 2 3 4 5 6 7 8 9 10	Dead 0.6D 0.6D 0.6D 0.6D 0.6D 0.6D 0.6D 0.6D	H+Collateral+Cead+0.6Wind_ead+0.6Wind_ead+0.6Wind_ead+0.6Wind_ead+0.6Wind_ead+0.6Wind_ead+0.6Wind_ead+0.6Wind_	.75Live+0.45Wind .75Live+0.45Wind Left1 Right1 Left2	d_Right Suction d_Long	y1L 1

BUILDING BR	ACING REACTIONS
	the Horz Vert Horz Vert Panel_Shear (lb/ft) Horz Vert Horz Vert Horz Vert Horz Vert

ANCHO	R BOLT	SUMMA	\RY		
QTY	LOCATE	DIA (in)	TYPE		
O 16 Ø 16	ENDWALL FRAME	5/8" 3/4"	A307 A307		

ENDV	VALL	COLU	JMN:		BASIC CO	DLUMN R	EACTIONS										
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.2 0.4 0.4 0.2		Collat Vert 0.1 0.1 0.1 0.1	Live Vert 1.3 2.5 2.5 1.3	Snow Vert 1.0 1.9 1.9	ı Le	.2	Wind Right1 Vert -1.2 -2.4 -3.2 -2.1	Wind Left2 Vert -1.5 -2.3 -1.6 -0.6	Wir Rig Ver -0.0 -1.6 -2.3	int2 f rt f 6 –	Mind Press Horz 0.9 2.2 2.2 0.9	Wind Suct Horz 1.0 2.4 2.4 1.0	Wind Long1 Vert -1.5 -3.0 -1.6 -1.2	Wind Long2 Vert -1.2 -1.6 -3.0 -1.5	
Frm Line 1 1 1	Col Line D C B A	Seis Left Vert 0.0 0.0 -0.1 0.1	-	Seis Right Vert 0.1 -0.1 0.0	E1UNB_ Horz 0.0 0.0 0.0 0.0	SL_L- Vert 1.0 2.2 0.9 0.2	E1UNB_ Horz 0.0 0.0 0.0 0.0	_SL_R- Vert 0.2 0.9 2.2 1.0									
Frm Line 4 4 4 4	Col Line A B C D	Dead Vert 0.2 0.4 0.4 0.2		Collat Vert 0.1 0.1 0.1 0.1	Live Vert 1.3 2.5 2.5 1.3	Snow Vert 1.0 1.9 1.9	ı Le	ind eft1 ert .1 .2 .4	Wind Right1 Vert -1.2 -2.4 -3.2 -2.1	Wind Left2 Vert -1.5 -2.3 -1.6 -0.6	Wir Rig Ver -0.4 -1.6 -2.3	ht2 f rt f 6 -	Wind Press Horz 0.9 2.2 2.2 0.9	Wind Suct Horz 1.0 2.4 2.4 1.0	Wind Long1 Vert -1.5 -3.0 -1.6 -1.2	Wind Long2 Vert -1.2 -1.6 -3.0 -1.5	
Frm Line 4 4 4 4	Col Line A B C D	Seis Left Vert 0.0 0.0 -0.1 0.1		Seis Right Vert 0.1 -0.1 0.0 0.0	E2UNB_ Horz 0.0 0.0 0.0 0.0	SL_L- Vert 1.0 2.2 0.9 0.2	E2UNB Horz 0.0 0.0 0.0 0.0	_SL_R- Vert 0.2 0.9 2.2 1.0									
ENDV	VALL	COLU	JMN:		MAXIMUM	REACTIO	NS, ANCH	OR BOLT	rs, & bas	E PLATES							
Frr Lin			Load Id	Hmax H	umn_Reac V Vmax	tions(k) Load Id	Hmin H	V Vmin	— Bolt QTY	(in) DIA	Base Width	:_Plate(in) Length	Thick	Grout (in)	_		
1	[)	8	0.6 0.0	-1.2 1.6	9 8	-0.5 0.6	-0.8 -1.2	2	0.625	3.500	8.000	0.250	0.0			
1	(8 1	1.4 0.0	-1.7 3.0	9	-1.3 1.4	-1.5 -1.7	2	0.625	3.500	8.000	0.250	0.0			
1	E	3	10 1	1.4 0.0	-1.7 3.0	11 10	-1.3 1.4	-1.5 -1.7	2	0.625	3.500	8.000	0.250	0.0			
1	,	4	10 1	0.6 0.0	−1.2 1.6	11 10	-0.5 0.6	−0.8 −1.2	2	0.625	3.500	8.000	0.250	0.0			
4		4	8 1	0.6 0.0	-1.2 1.6	9 8	-0.5 0.6	-0.8 -1.2	2	0.625	3.500	8.000	0.250	0.0			
4	E		8 1	1.4 0.0	-1.7 3.0	9 8	-1.3 1.4	-1.5 -1.7	2	0.625	3.500	8.000	0.250	0.0			
4	(10 1	1.4 0.0	-1.7 3.0	11 10	-1.3 1.4	-1.5 -1.7	2	0.625	3.500	8.000	0.250	0.0			
4	[,	10 1	0.6 0.0	-1.2 1.6	11 10	-0.5 0.6	−0.8 −1.2	2	0.625	3.500	8.000	0.250	0.0			

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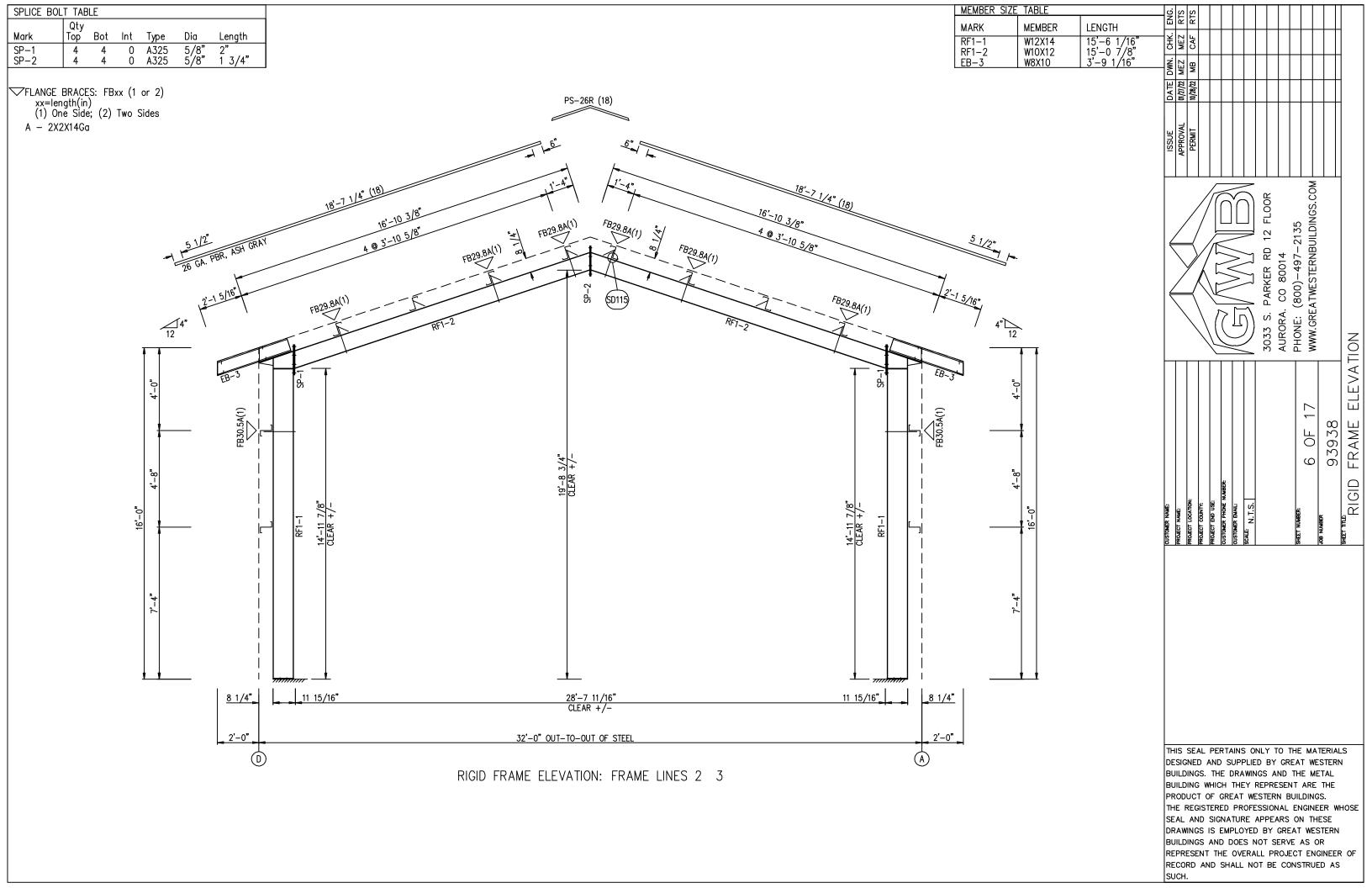
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PHONE: (800)-497-2135
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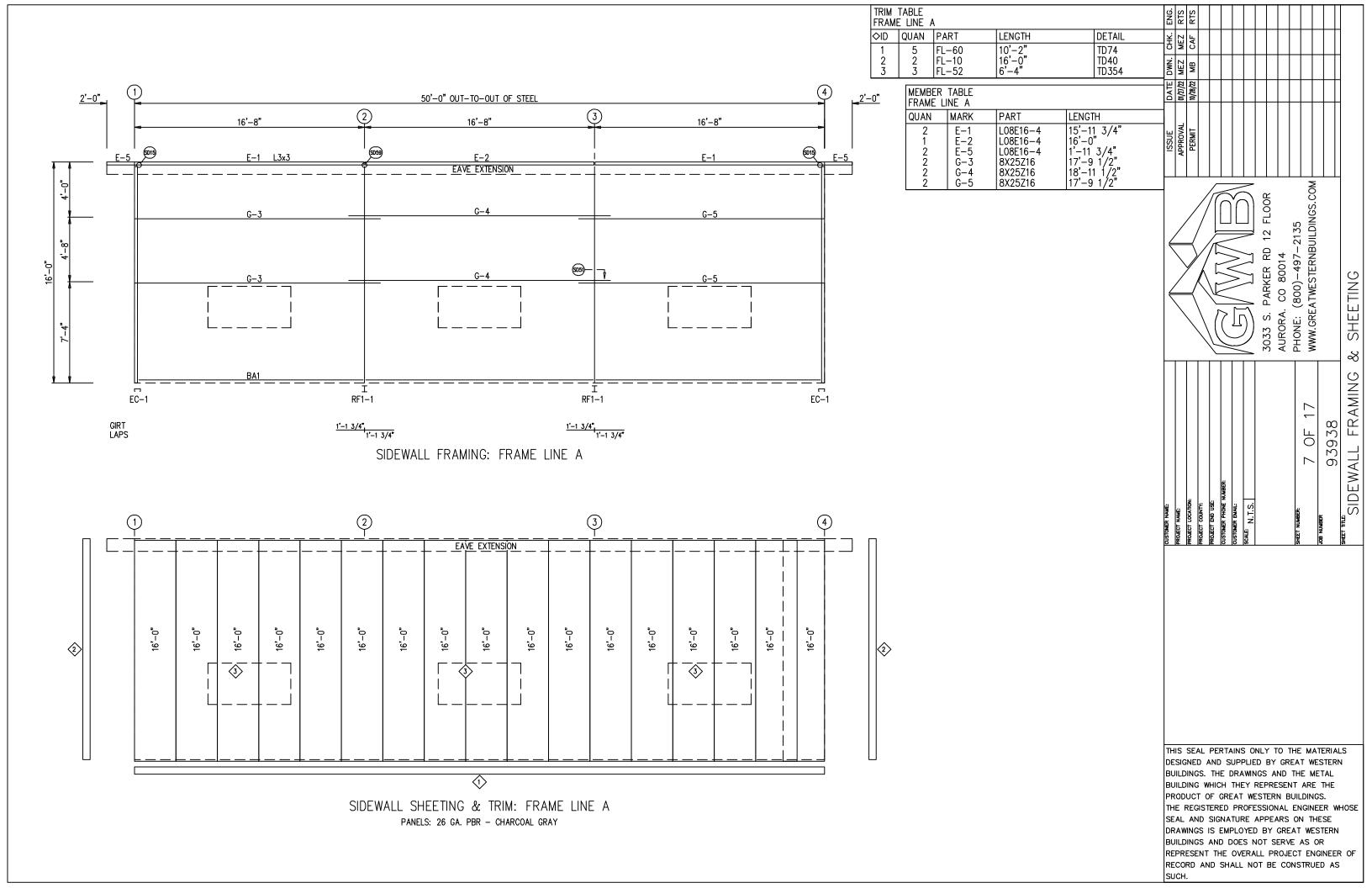
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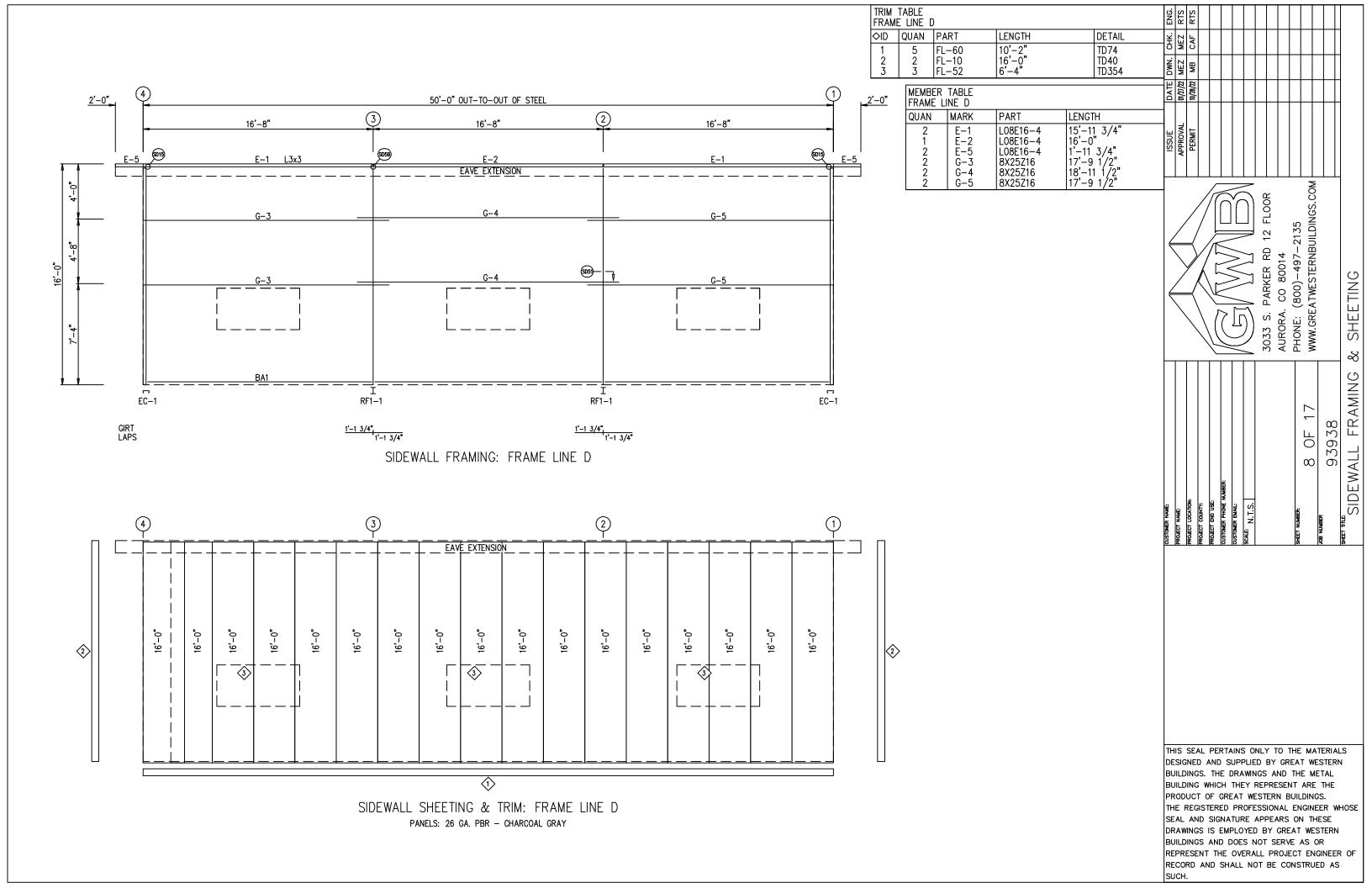
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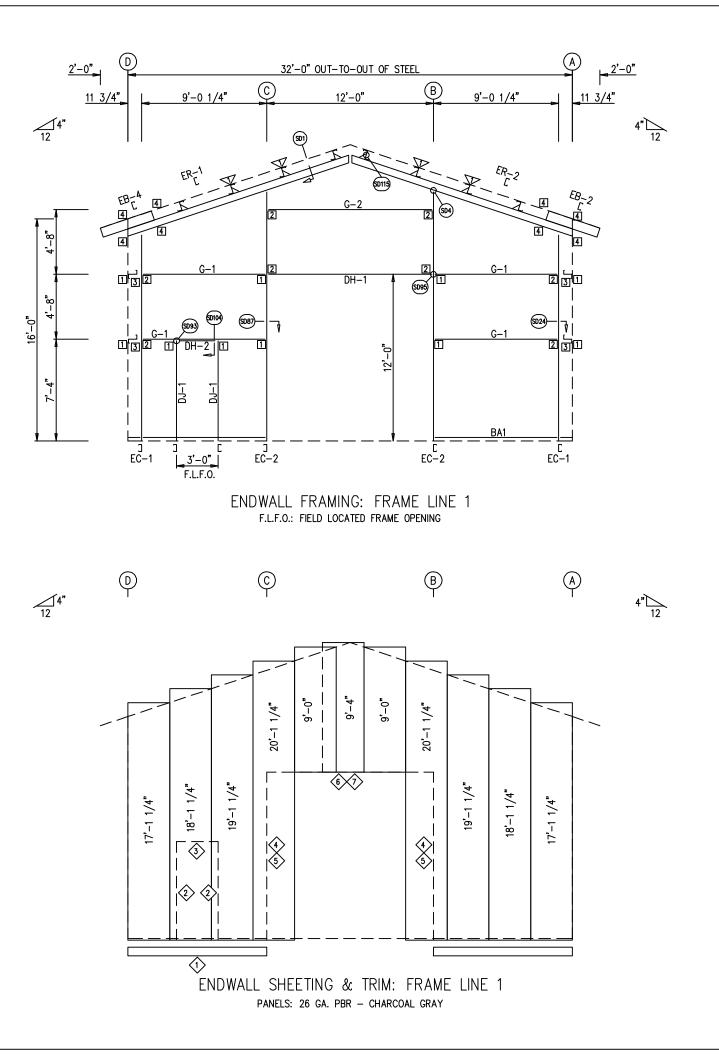
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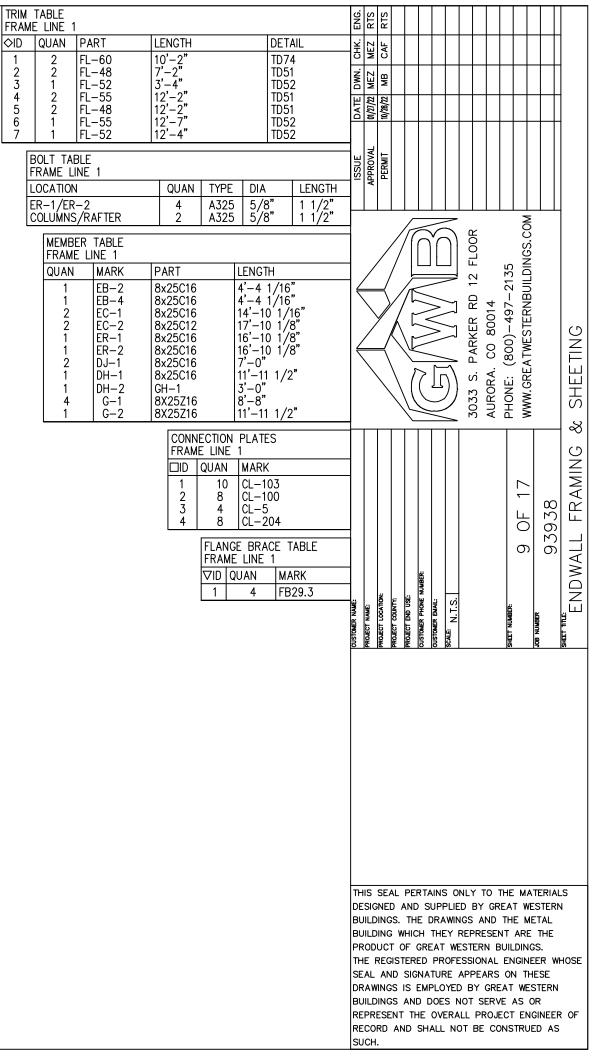
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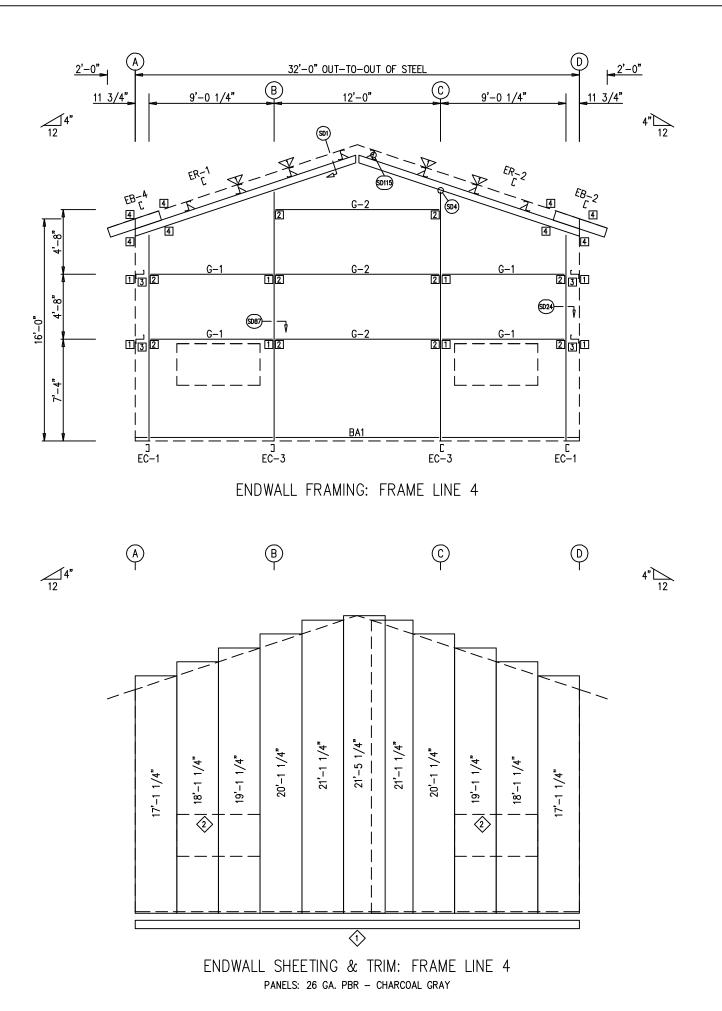








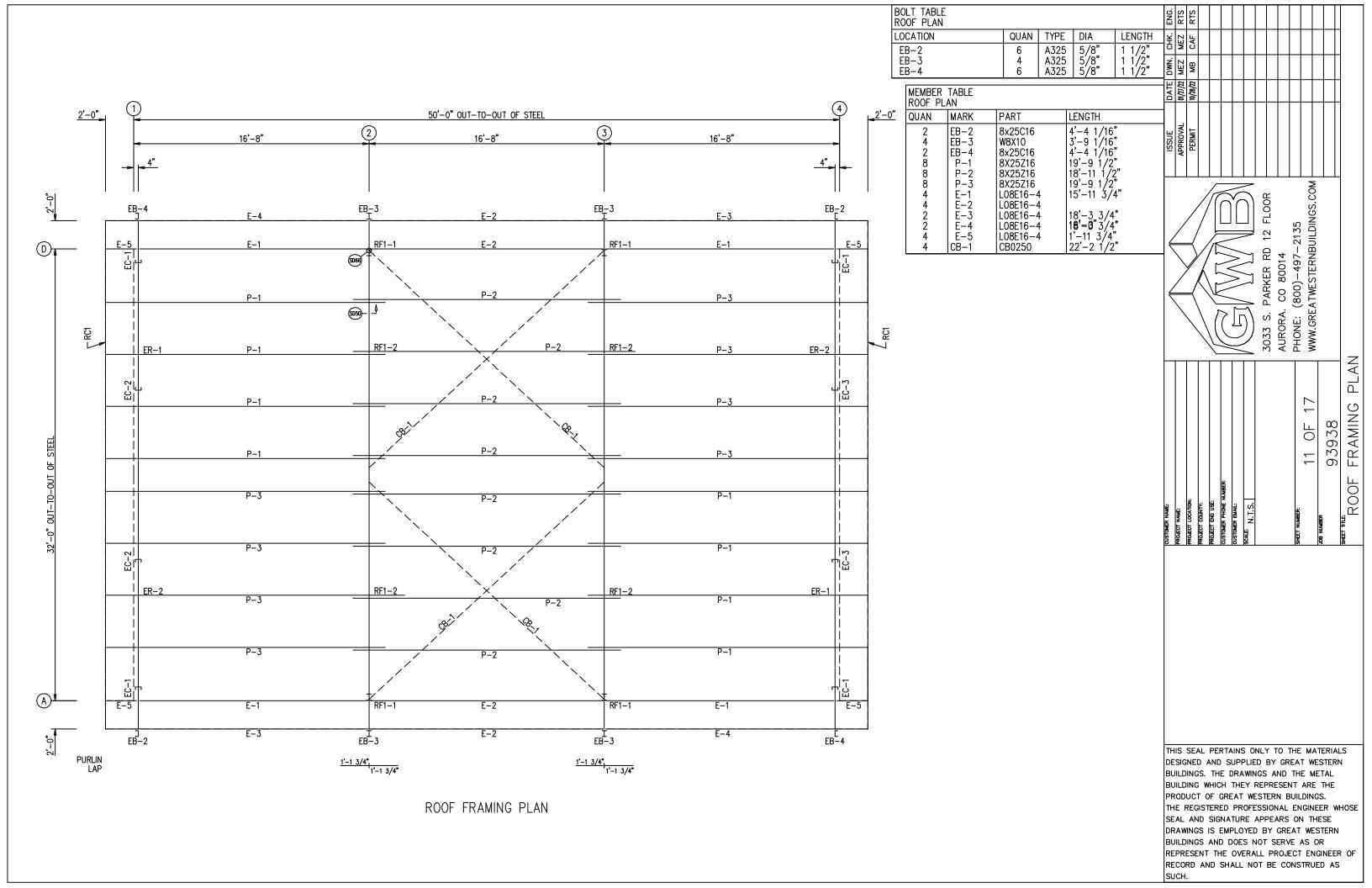


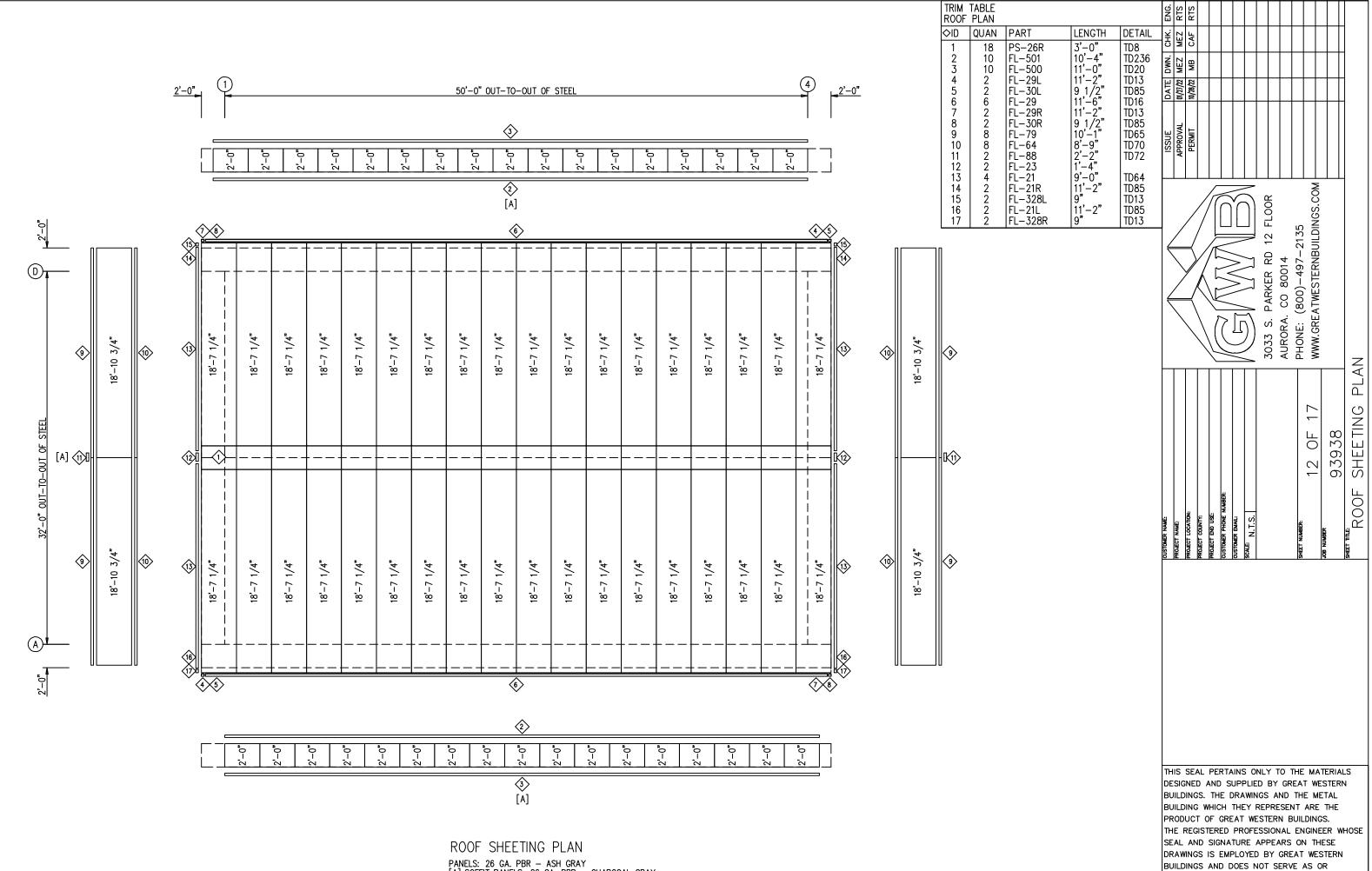


TRIM TABLE FRAME LINE 4 ◇ID QUAN PART LENGTH DETAIL 10'-2" 6'-4" TD74 TD354 FL-60 FL-52 BOLT TABLE FRAME LINE 4 LOCATION QUAN TYPE DIA LENGTH A325 A325 5/8" 5/8" ER-1/ER-2 COLUMNS/RAFTER MEMBER TABLE FRAME LINE 4 QUAN MARK PART LENGTH EB-2 EB-4 EC-1 EC-3 ER-1 ER-2 G-1 G-2 4'-4 1/16" 4'-4 1/16" 14'-10 1/16" 17'-10 1/8" 16'-10 1/8" 8'-8" 8x25C16 8x25C16 8x25C16 8x25C12 8x25C16 8x25C16 AURORA. CO 80014 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM 8X25Z16 8X25Z16 SHEETING CONNECTION PLATES FRAME LINE 4 □ID QUAN MARK 8 CL-103 10 CL-100 4 CL-5 8 CL-204 FLANGE BRACE TABLE FRAMING FRAME LINE 4 VID QUAN MARK 4 FB29.3 93938 OF ENDWALL 10 THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF

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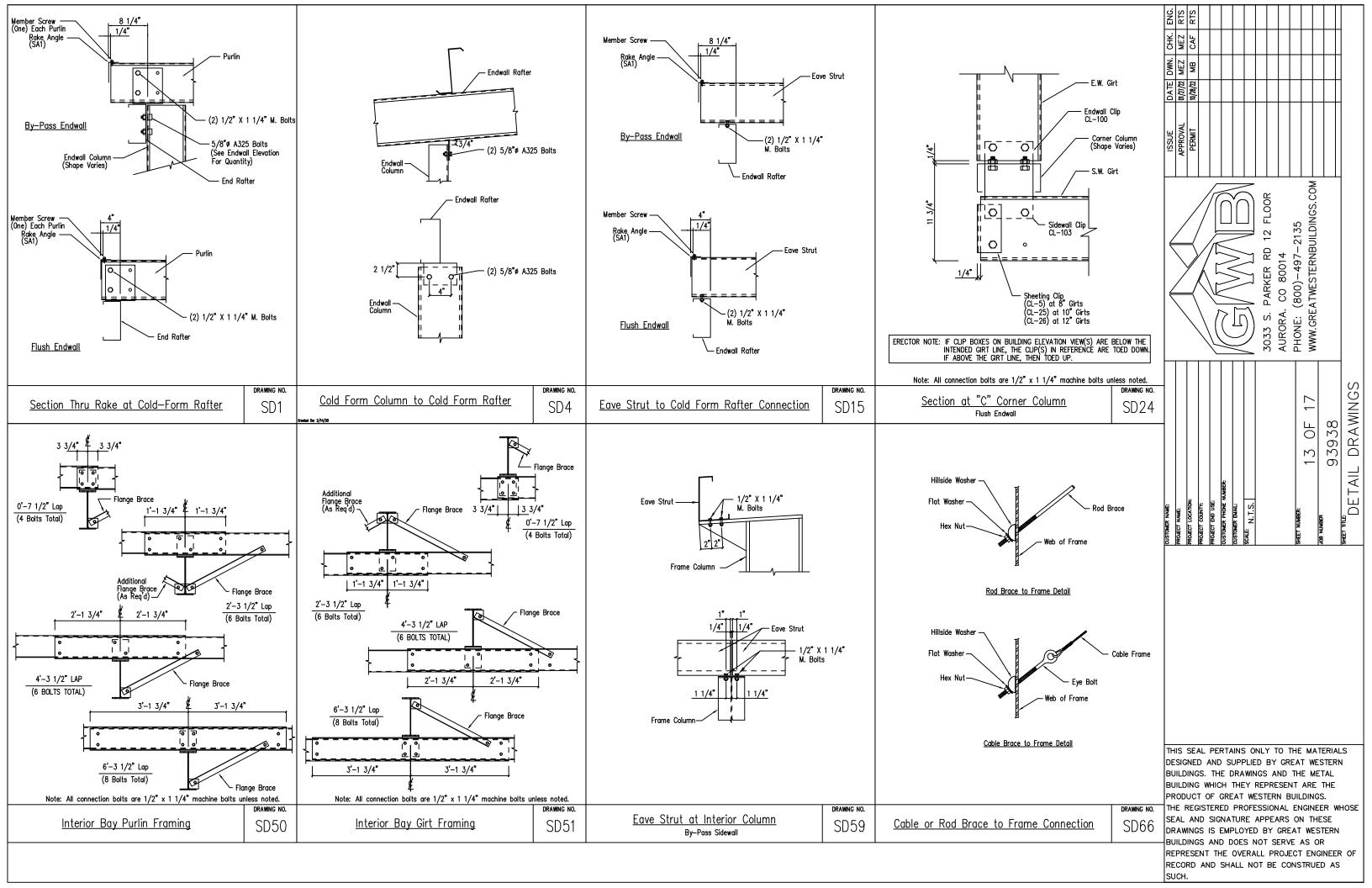
RECORD AND SHALL NOT BE CONSTRUED AS

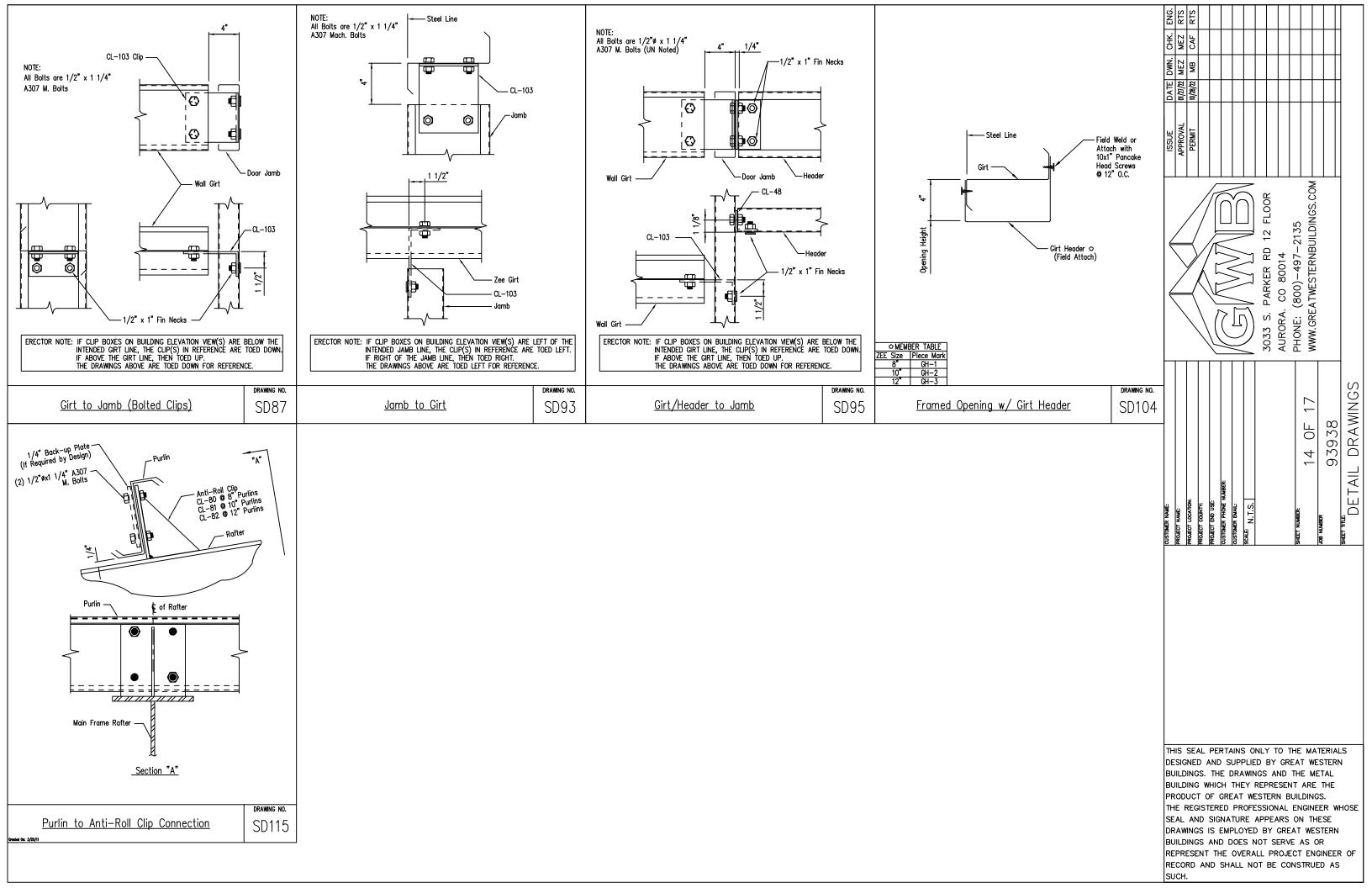


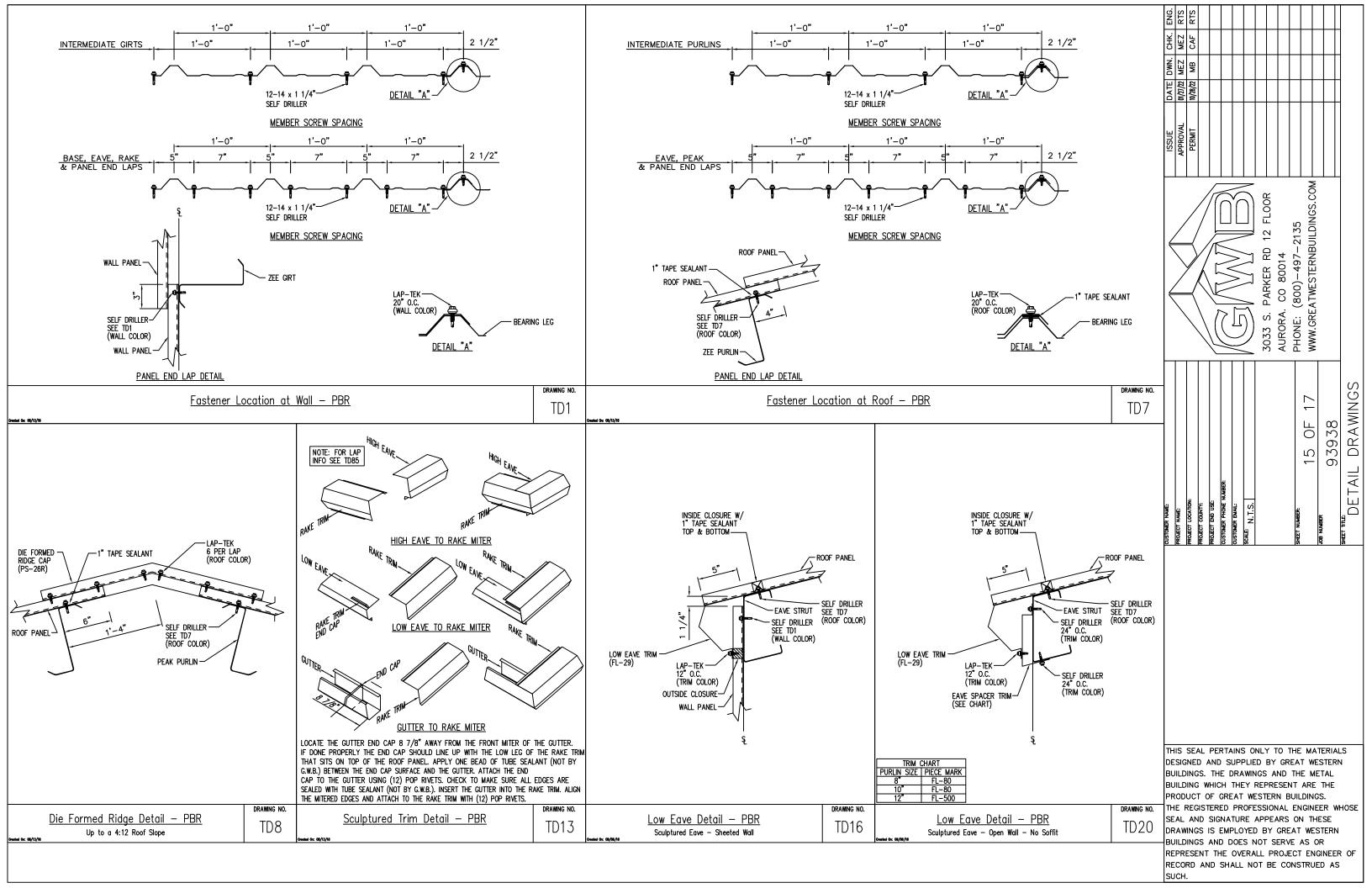


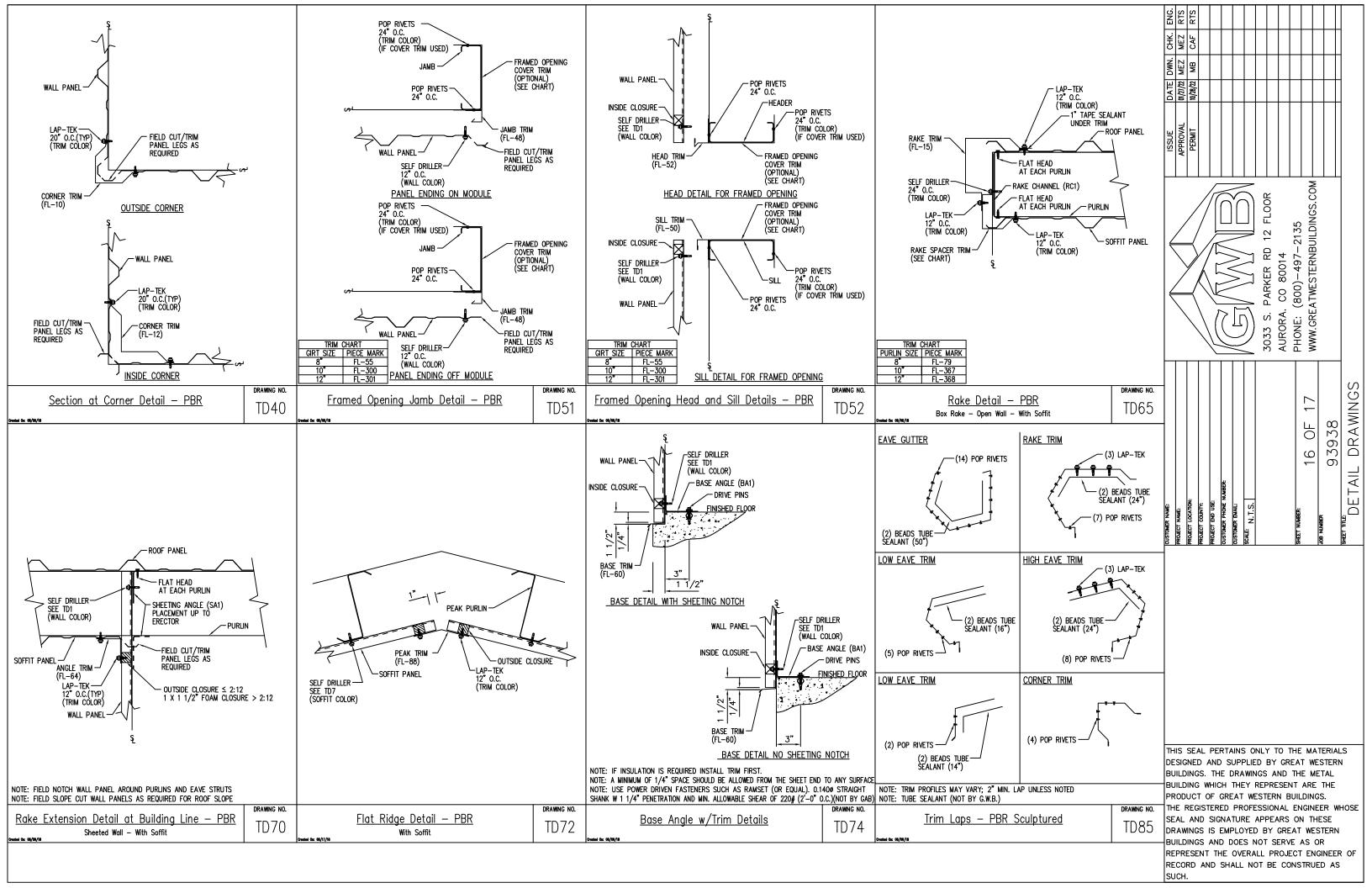
PANELS: 26 GA. PBR — ASH GRAY [A] SOFFIT PANELS: 26 GA. PBR — CHARCOAL GRAY

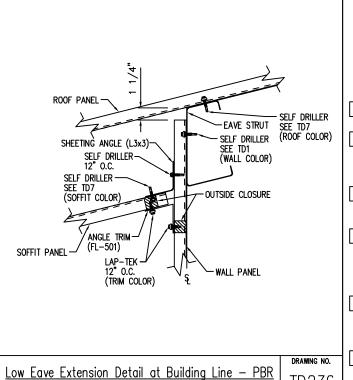
REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS



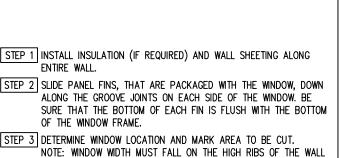


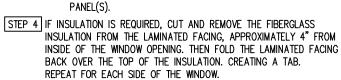




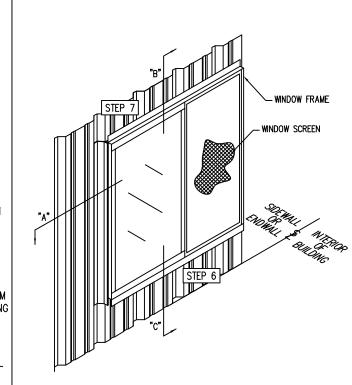


Flat Eave - Sheeted Wall - With Soffit





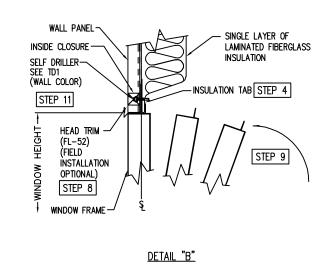
- STEP 5 STARTING AT THE TOP AND WORKING DOWN TOWARDS THE WINDOW HEIGHT DIMENSION PLUS AN ADDITIONAL 1/8". FIELD CUT THE WALL PANEL 1" FROM CENTER OF THE HIGH RIB, TOWARDS THE WINDOW FRAME, DOWN EACH SIDE OF THE WINDOW.
- STEP 6 FIELD CUT THE WALL PANEL ALONG THE BOTTOM OF THE WINDOW CONNECTING THE TWO PREVIOUS LINES.
- STEP 7 FIELD CUT THE WALL PANEL ALONG THE TOP OF THE WINDOW
- STEP 8 WITH THE WALL PANEL NOW REMOVED, INSTALL THE HEAD TRIM AND INSIDE CLOSURE ON THE BOTTOM OF THE WALL PANEL ALONG THE TOP OF THE WINDOW OPENING BY USING SOME MEANS OF TEMPORARY SUPPORT.
- STEP 9 INSTALL THE INSIDE CLOSURE ON THE WALL PANEL BELOW THE WINDOW. PLACE THE BOTTOM OF THE WINDOW FRAME ON TOP OF THE WALL PANEL. SANDWICH THE LAMINATED FACING TABS BETWEEN THE BOTTOM OF THE WINDOW FRAME AND WALL PANEL, WORKING YOUR WAY UP EACH SIDE WITH THE INSULATION TABS TOWARDS THE TOP OF THE WINDOW. WHILE ROTATING THE TOP OF WINDOW FRAME INTO PLACE.
- STEP 10 FASTEN THE BOTTOM OF THE WINDOW FRAME TO THE WALL PANEL FROM THE INTERIOR OF THE BUILDING.
- STEP 11 FASTEN THE TOP OF THE WINDOW FRAME TO THE WALL PANEL FROM THE EXTERIOR OF THE BUILDING. THEN REMOVE TEMPORARY SUPPORT
- STEP 12 FASTEN THE SIDES OF THE WINDOW FRAME TO THE WALL PANEL FROM THE EXTERIOR OF THE BUILDING.
- STEP 13 CAULK ALL SIDES OF WINDOW FRAMING AGAINST PANEL FOR WEATHER TIGHTNESS (NOT BY G.A.B.).

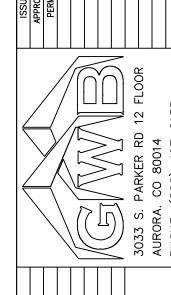


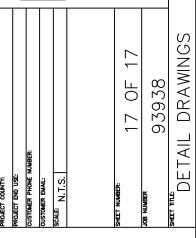
NOTE: WINDOW IS INSTALLED FROM INTERIOR OF THE BUILDING.

NOTE: INSULATION NOT SHOWN FOR CLARITY.

NOTE: IF FRAMED OPENINGS ARE REQUIRED INSTALL AFTER WINDOW IS INSTALLED.

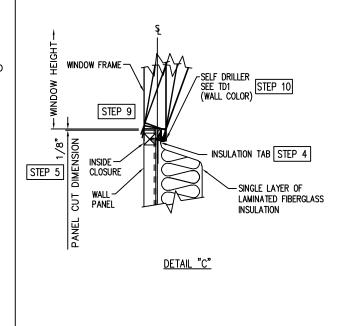






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WINDOW HEIGHT PANEL CUT DIMENSION SELF DRILLER 12" O.C. STEP 5 (WALL COLOR) STEP 12 - FIELD CUT/TRIM PANEL LEGS AS REQUIRED STEP 3 - WNDOW FRAME INSULATION TAB--PANEL FIN STEP 4 LAMINATED FIBERGLASS STEP 2 INSULATION DETAIL "A"



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