

# GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50  KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36  KSI
	STEEL PIPES	A500	Fy = 42  KSI
	STRUCTURAL TUBING	A500	Fy = 42  KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	$F_y = 50 \text{ KSI}$
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	$F_y = 55 \text{ KSI}$
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55  KSI
	ROOF & WALL SHEETS	A792/A653	$F_{y} = 50, 80 \text{ KSI}$
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI

MIN. TENSILE STRENGTH Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI A325-TYPE 1 A325-TYPE 1 HIGH STRENGTH BOLTS (1" & LESS) HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554 Fu = 60 KSI

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF MESPONSIBLE FOR ANY DELEMENTATION OF THE STOP FRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).

NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT

.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT:

A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N

B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION)

C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION D) FLANGE BRACECONNECTIONS — 1/2"ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

### .6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION. STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

## 1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS

BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:

ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED

## 1.9 ERECTION AND UNLOADING NOT BY G.W.B.

## 1.10 SHORTAGES

ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISTITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECIEVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

# BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
  STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
  DOCUMENTS, M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
  METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
  TO THE CONTRARY NOTWITHSTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
  BUYER/FUND USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
  EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

  N. CASE OF DISCPEDANCES BETWEEN M.B.S. STUILDINGLA, STEPL JANS AND DIANS FOR
- 2.3 IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY AFFROYAL OF MIS.3. BARWINGS AND CALCULATIONS INDUCATE THE M.B.S. HAS CORRECTED INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/
  END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

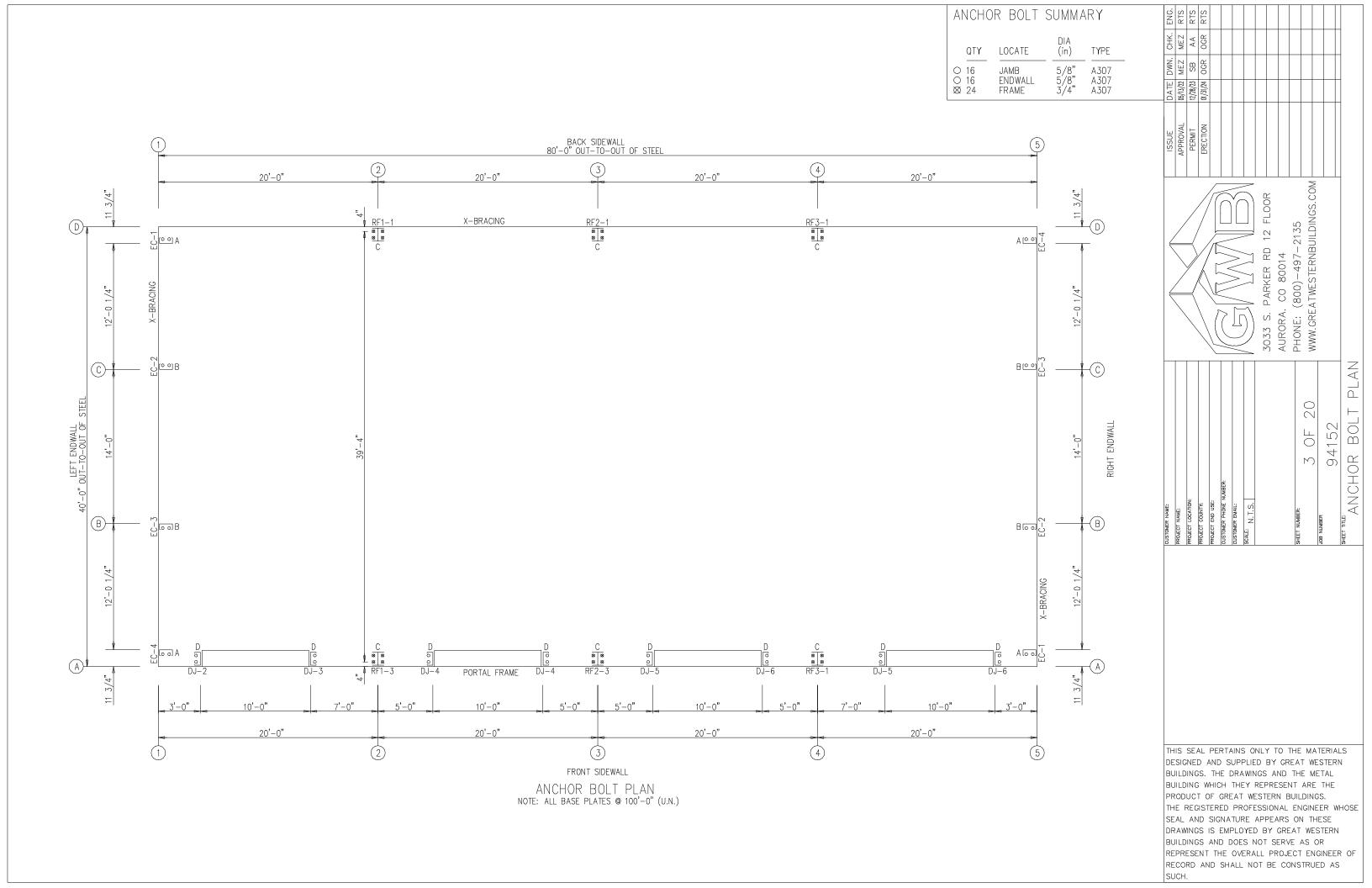
- 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION.
  ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS
  CONCERNING ANY MATERIALS NOT FIRNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE
  CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA
  CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR ABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE FRECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE FRECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS, USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD
- 9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE CONCRETE.

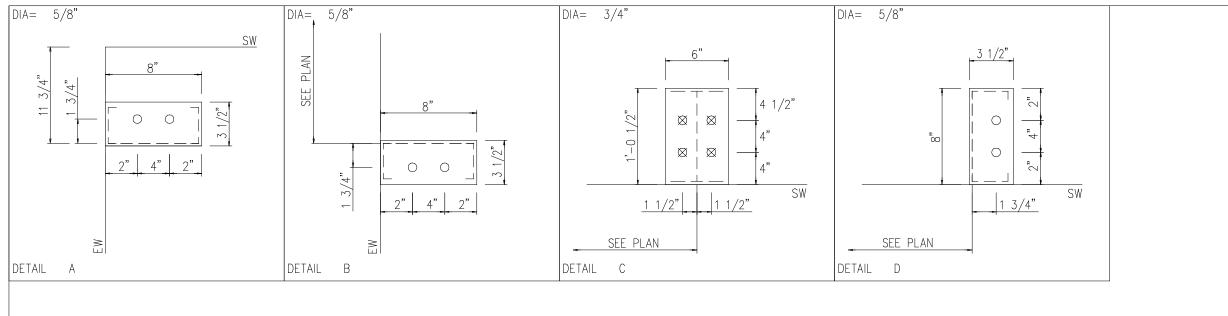
  UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANULA!)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE NORMAL ERECTION OFERATIONS INCLODE THE CORRECTIONS OF MINOR MISTERS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD DELECTOR AFTER ENTERING AND EXCHAPACTED FOR THE MOST EFFICIENT AND EXCHAPACT EXCHAPACTED FOR THE MOST EFFICIENT AND EXCHAPACT EXCHAPACTED FOR THE MOST EFFICIENT AND EXCHAPACT EXCHAPACT
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWSE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>WARNING</u> IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKES SAFETY, MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE. ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP, SSE TABLE BELOW.

20         16.60         8.30           25         17.25         8.62           30         17.90         8.95           35         18.55         9.28           40         19.20         9.60           45         19.85         9.92           50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88           80         24.40         12.20		(IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	WHEN SNOW REMOVAL SHOULD START (IN INCHES)
30         17.90         8.95           35         18.55         9.28           40         19.20         9.60           45         19.85         9.92           50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88	Ī	20	16.60	8.30
35         18.55         9.28           40         19.20         9.60           45         19.85         9.92           50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88		25	17.25	8.62
40         19.20         9.60           45         19.85         9.92           50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88		30	17.90	8.95
45         19.85         9.92           50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88		35	18.55	9.28
50         20.50         10.25           55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88		40		
55         21.15         10.58           60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88	L	45	19.85	9.92
60         21.80         10.90           65         22.45         11.22           70         23.10         11.55           75         23.75         11.88	L	50	20.50	
65 22.45 11.22 70 23.10 11.55 75 23.75 11.88		55	21.15	10.58
70 23.10 11.55 75 23.75 11.88			21.80	
75 23.75 11.88		65	22.45	
			23.10	11.55
80 24.40 12.20	L			
		80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

### ENG. RTS RTS BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: DESIGN LOADS: DESIGN CODE / WIND CODE : IBC-21 OCCUPANCY / RISK CATEGORY : II-Normal **ENCLOSURE** : Enclosed ROOF DEAD LOAD (D) (PSF) : 2.00 ROOF COLLATERAL LOAD (C) (PSF) :1.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) : 115.00 WIND EXPOSURE CATEGORY : C INTERNAL PRESSURE COEFFICIENT, (GCpi) :0.18/-0.18WALL PANEL DESIGN WIND PRESSURE (PSF) : 23.78/-25.80 WIND ENCLOSURE CLASSIFICATION : Enclosed LIVE LOAD PRIMARY FRAMING (PSF) : 20.00 TRIB. AREA REDUCTION : No SECONDARY FRAMING (PSF) : 20.00 2135 TWESTERNBUILDIN 12 SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) . 20 00 80014 ROOF SNOW LOAD, (Pf) (PSF) : 20.00 SNOW EXPOSURE FACTOR, (Ce) PARKER : 1.00 SNOW IMPORTANCE FACTOR, (Is) :1.00 (800) THERMAL FACTOR, (Ct) :1.00 SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) :1.00 GRE ഗ് Ш PHONE: SITE CLASSIFICATION : D RSHE SPECTRAL RESPONSE ACCELERATION : Ss = 0.175 : S1 = 0.059SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.186 : Sd1 = 0.093SEISMIC DESIGN CATEGORY ·R BASIC SEISMIC FORCE RESISTING SYSTEM :STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE Ó : RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) TOTAL DESIGN BASE SHEAR, (V) (KIPS) :LONGITUDINAL = 1.19 $\bigcirc$ :TRANSVERSE = 1.20 RESPONSE MODIFICATION FACTORS, (R) :RIGID FRAMES = 3.00 $\Omega = 3.00$ $\bigcirc$ :SW X-BRACING = 3.00 $\Omega = 3.00$ 4 DING $\cdot$ SW WIND BENT = 3.00 $\Omega = 3.00$ $\sim$ Ó SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.0619:SW X-BRACING = 0.0619:SW WIND BENT = 0.0619 $\bar{m}$ ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS **BUILDING DESCRIPTION:** WIDTH (FT) : 40.00 LENGTH (FT) .80.00EAVE HEIGHT AT BSW (FT):12.00 EAVE HEIGHT AT FSW (FT): 12.00 ROOF SLOPE AT BSW : 5.0:12 ROOF SLOPE AT FSW : 5.0:12 BAY SPACING (FT) :4 AT 20.00 **COVERING AND TRIMS: ROOF PANELS & TRIMS** PANEL TYPE :26 GA. PBR PANEL COLOR : GALVALUME TRIM COLORS GABLE /EAVE : CHARCOAL GRAY EAVE GUTTER : CHARCOAL GRAY WALL PANELS & TRIMS PANEL TYPE · 26 GA PRR PANEL COLOR : ASH GRAY THIS SEAL PERTAINS ONLY TO THE MATERIALS TRIM COLORS DESIGNED AND SUPPLIED BY GREAT WESTERN : CHARCOAL GRAY CORNER BUILDINGS. THE DRAWINGS AND THE METAL : CHARCOAL GRAY FRAMED OPENING BUILDING WHICH THEY REPRESENT ARE THE : CHARCOAL GRAY DOWNSPOUTS PRODUCT OF GREAT WESTERN BUILDINGS. : CHARCOAL GRAY RASE THE REGISTERED PROFESSIONAL ENGINEER WHOSE WAINSCOT PANELS & TRIMS SEAL AND SIGNATURE APPEARS ON THESE PANEL TYPE :26 GA. PBR DRAWINGS IS EMPLOYED BY GREAT WESTERN PANEL COLOR : CHARCOAL GRAY BUILDINGS AND DOES NOT SERVE AS OR TRIM COLORS : CHARCOAL GRAY REPRESENT THE OVERALL PROJECT ENGINEER OF INSULATION ROOF INSULATION :6" (R-19) WMP-VR RECORD AND SHALL NOT BE CONSTRUED AS WALL INSULATION : 6" (R-19) WMP-VR



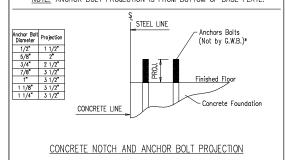


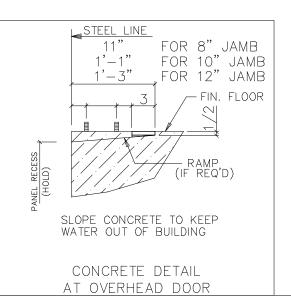
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR

FIELD WORK. ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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DETAILS

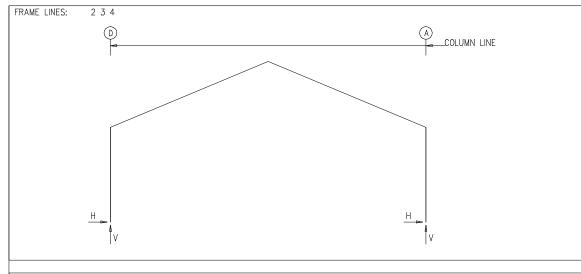
BOLT

ANCHOR

94152

ENG. RTS RTS RTS

THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS



RIGI	D F	RAME:		MAXIMUM	REACTION	IS, ANCH	HOR BOLT	S, & BASE	E PLATE	S				
	rm ine	Col Line	Load Id	Colu Hmax H	ımn_React V Vmax		Hmin H	V Vmin	Bolt QTY	(in) 	Base Width	_Plate(in) Length	Thick	Grout (in)
2		D	1	4.1	9.7	4 6	-2.2 0.4	-1.0 -3.6	4	0.750	6.000	12.50	0.375	0.0
2		А	5 1	2.2 -4.1	-1.0 9.7	1 7	-4.1 -0.4	9.7 -3.6	4	0.750	6.000	12.50	0.375	0.0

RIGID	FRAME:		MAXIMUM	REACTION	NS, ANCH	HOR BOL	TS, & BASI	E PLATE	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax		Hmin H	V Vmin	Bol- QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
3	D	1	4.1	9.7	4 6	-2.2 0.4	-1.0 -3.6	4	0.750	6.000	12.50	0.375	0.0
3	Α	5 1	2.2 -4.1	-1.0 9.7	1 7	-4.1 -0.4	9.7 -3.6	4	0.750	6.000	12.50	0.375	0.0

RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BASE	PLATE	:S				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol: QTY	(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
4	D	1	4.1	9.7	4 2	-2.2 -2.2	−1.0 −2.8	4	0.750	6.000	12.50	0.375	0.0
4	Α	5 1	2.2 -4.1	-1.0 9.7	1 3	-4.1 2.2	9.7 -2.8	4	0.750	6.000	12.50	0.375	0.0

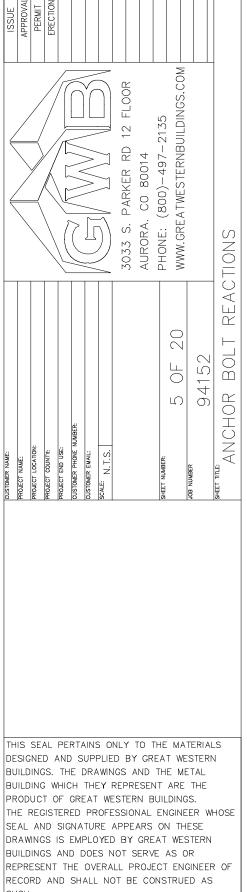
Wa	II —	– Col	±	Reacti	ons(k ) - —Sei	smic -	Panel_ - (lb/		
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Not-
L_EW F_SW R_EW B_SW	1 A 5 D	D,C 2,3 A,B 3,2	1.7 1.3 1.7 2.6	2.2 1.4 2.2 1.3	0.2 0.3 0.2 0.6	0.2 0.3 0.2 0.3			(b

ANCHO	R BOLT	SUMMA	\RY	
QTY ○ 16 ○ 16 ○ 24	LOCATE JAMB ENDWALL FRAME	DIA (in) 5/8" 5/8" 3/4"	A307 A307 A307 A307	

Reactions for seismic represent shear force, Eh

RIGIE	FRAM	1E:	BASI	IC COLUM	IN REACT	IONS (k	)						
	Column Line D A	 Horiz 0.5 -0.5			oteral— Vert 0.4 0.4	Horiz	Vert	Horiz		Wind Horiz -4.2 -1.0	_Left1- Vert -5.9 -4.4	Horiz 1.0	Right1- Vert -4.4 -5.9
FRAME Line 2 2	Column Line D A	Wind Horiz -4.2 -1.0	_Left2- Vert -3.0 -1.5	-Wind_ Horiz 1.0 4.2	Vert	Horiz 0.1	Vert	Wind Horiz -1.0 -0.1	I_Long2- Vert -6.8 -7.3	-Seismi Horiz -0.1 -0.1	ic_Left Vert -0.1 0.1	Seismic Horiz 0.1 0.1	Vert
Line 2	Column Line D A		Vert	Horiz	SL_L- Vert 7.3 4.4		SL_R- Vert 4.4 7.3						
FRAME Line 3	Column Line D A	Horiz	Vert	Horiz	Vert	Horiz 3.4	Vert	Horiz	-Snow Vert 8.0 8.0	Horiz -4.2	_Left1- Vert -5.9 -4.4	Horiz 1.0	Right1- Vert -4.4 -5.9
	Line D		_Left2- Vert -3.0 -1.5	Horiz 1.0	Right2- Vert -1.5 -3.0	Horiz 0.1	Vert	Horiz -1.0	I_Long2- Vert -6.8 -7.3	-Seismi Horiz -0.1 -0.1	Vert -0.1	Seismic Horiz 0.1 0.1	Vert
	Column Line D A	-Seismi Horiz 0.0 0.0	ic_Long Vert -0.3 -0.3		_SL_L- Vert 7.3 4.4	F2UNB_ Horiz 2.6 –2.6	_SL_R- Vert 4.4 7.3						
Line 4	Column Line D A		Vert		oteral— Vert 0.4 0.4		-Live Vert 8.0 8.0	Horiz	-Snow Vert 8.0 8.0		_Left1- Vert -5.9 -4.4	Horiz 1.0	Right1- Vert -4.4 -5.9
	Column Line D A	Horiz -4.2	_Left2- Vert -3.0 -1.5	Horiz	Vert −1.5	Horiz 0.1	I_Long1- Vert -5.9 -5.5	Horiz	Vert −5.5	-Seismi Horiz -0.1 -0.1	ic_Left Vert -0.1 0.1	Seismic Horiz 0.1 0.1	Vert 0.1
Line 4		F3UNB_ Horiz 2.6 -2.6	Vert	F3UNB_ Horiz 2.6 -2.6	Vert								

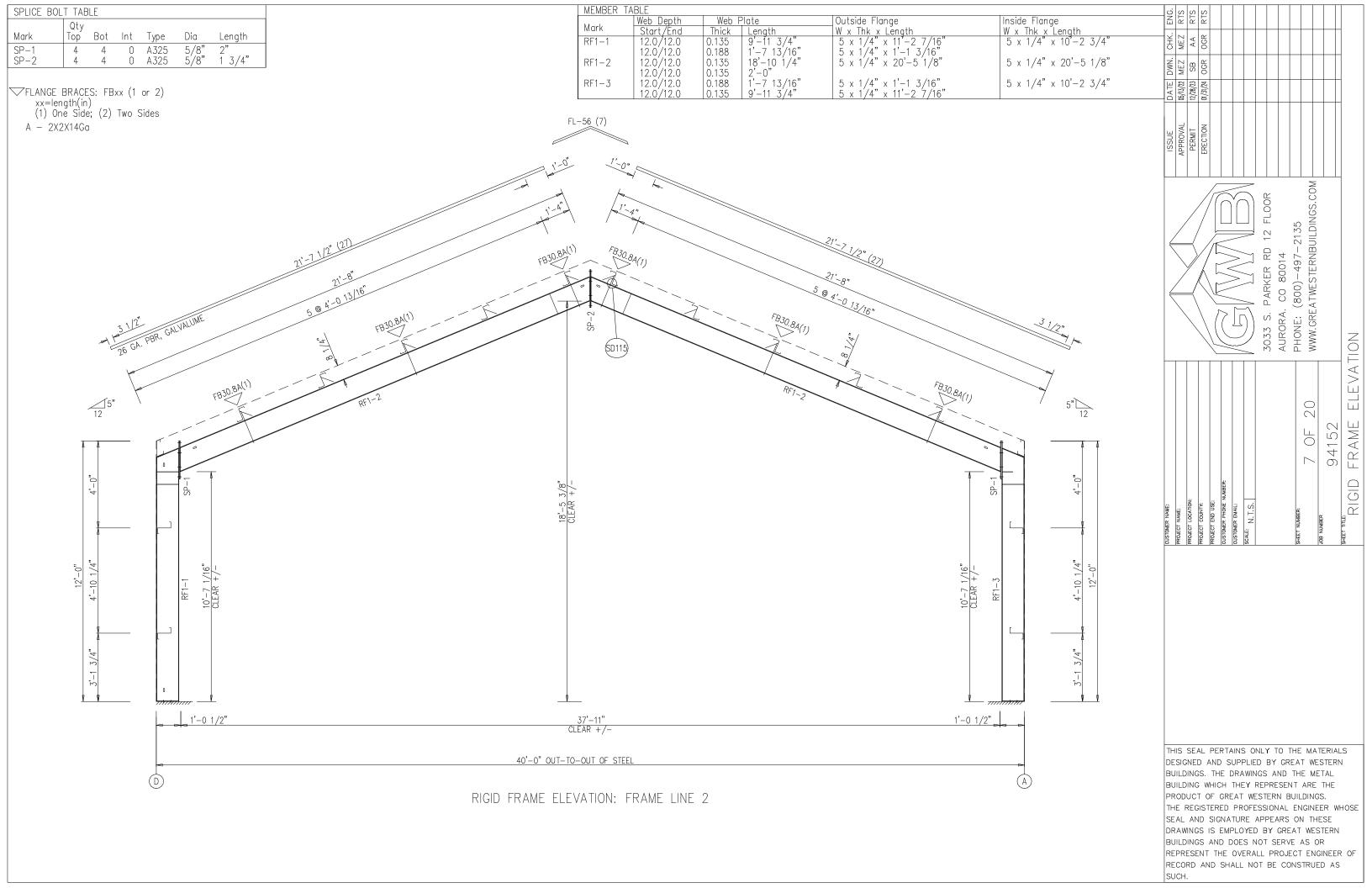
NOTES	FOR REACTIONS		
the ·	ling reactions are based on following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise)(12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= = = = = = = = = = = = = = = = = = =	40.00 80.00 12.00/12.00 5.0:12/5.0:12 2.00 1.00 20.00 20.00 115.00 IBC-21 C Enclosed 1.00 1.00 1.00
ID —	Description		
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16	0.6Dead+0.6Wind_Right1+0.6' 0.6Dead+0.6Wind_Pressure+C Dead+Collateral+E1UNB_SL_L 0.6Dead+0.6Wind_Suction+0.1 Dead+Collateral+E1UNB_SL_F	ind_Suction 0.6Wind_Long .45Wind_Rig Wind_Suction 0.6Wind_Long 6Wind_Long 1.6Wind_Long	g2L pht2+0.45Wind_Suction+0.75Slide_Snov in g1L

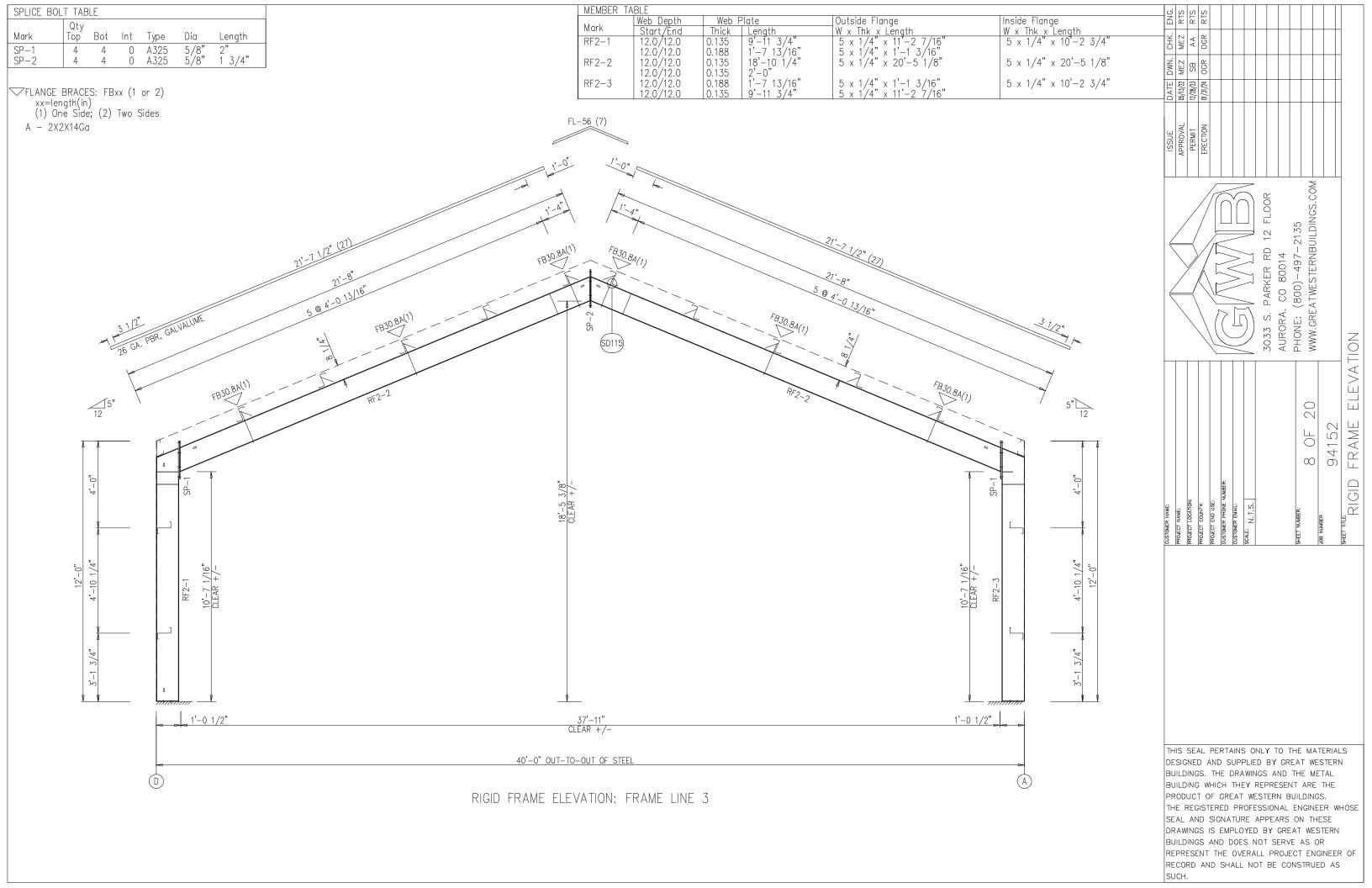


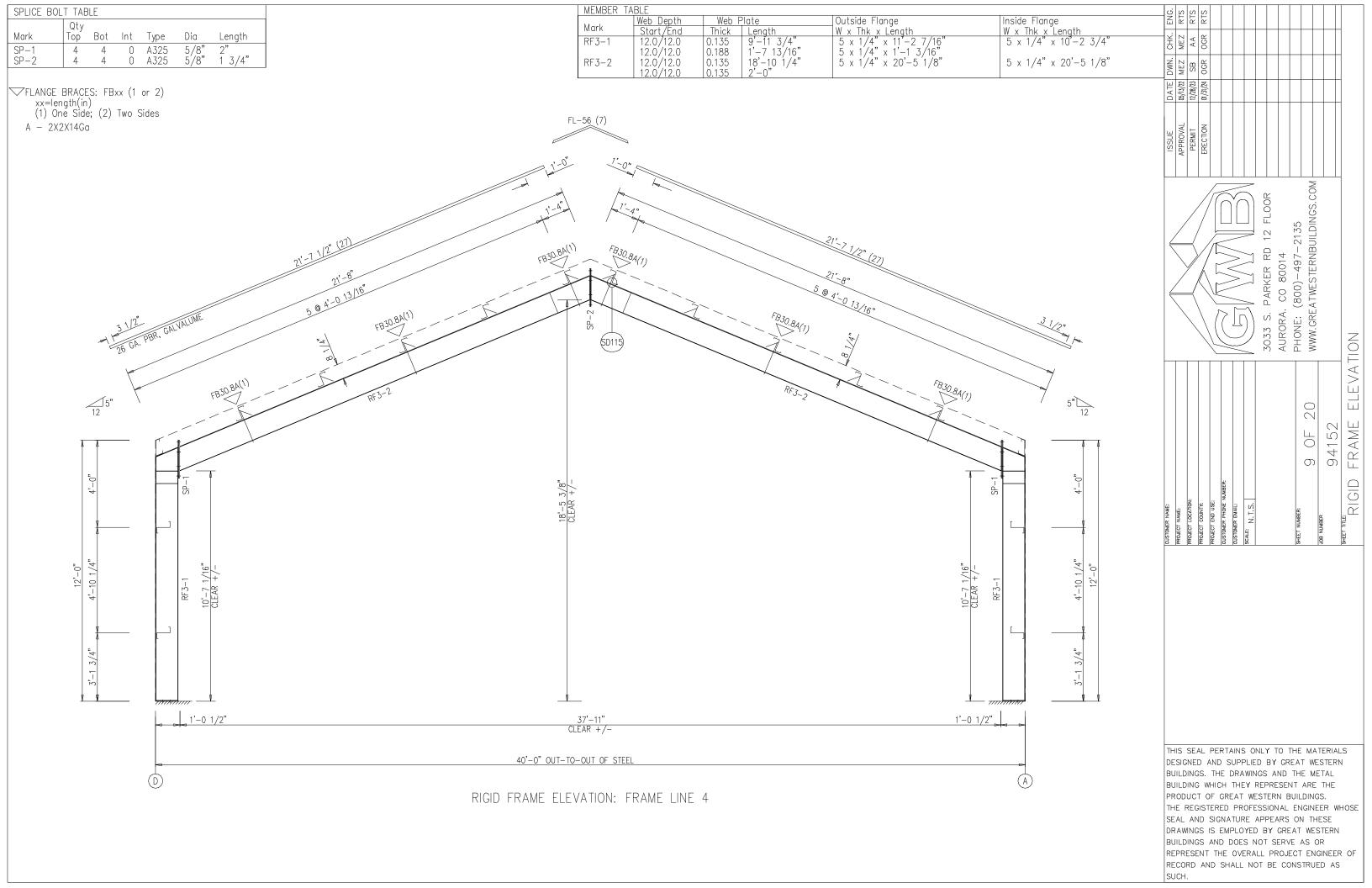
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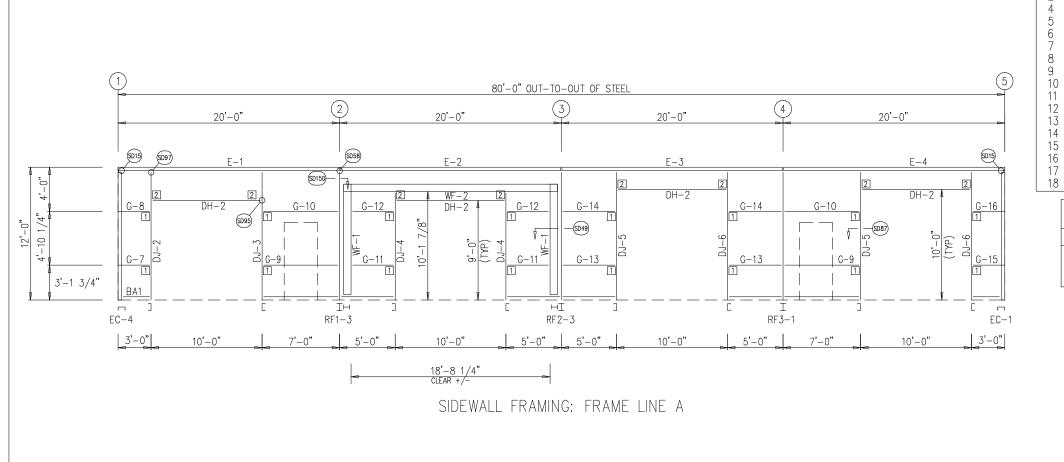
ENDV	VALL	COLU	JMN:		BASIC	COLUMN	REACTIC	ONS (k )									
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.2 0.5 0.4 0.2		Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.0 3.0 3.0	Sno Ver 1.0 3.0 3.0	t	Wind_Le Horz -1.7 0.0 0.0 0.0	eft1 Vert -3.0 -0.2 -1.6 -1.5	Wind_ Horz 0.0 1.7 0.0 0.0	Right1 Vert 1.5 -4.6 -2.5 -0.7	Wind_L Horz -1.7 0.0 0.0 0.0	eft2 Vert -2.3 0.5 -0.8 -0.8	Wind_ Horz 0.0 1.7 0.0 0.0	Right2 Vert 2.2 -3.8 -1.8 0.0	Wind Press Horz -0.7 -2.0 -2.0 -0.7	
Frm Line 1 1 1	Col Line D C B A	Wind Suct Horz 0.8 2.2 2.2 0.8		Wind_L Horz 0.0 0.8 0.0 0.0	Vert	Wind_Lo Horz -0.8 0.0 0.0	ng2 Vert -2.3 -0.4 -2.5 -1.9	Seis Horz -0.2 0.0 0.0	Left Vert -0.2 0.2 0.0 0.0	Sei Hor 0.0 0.2 0.0 0.0	0.3 -0.3 0.0	Seis Long Vert 0.0 0.0 0.0	E1 <sup>1</sup> Ho 0.6 0.6 0.0	0 1 0 3 0 1	_L- /ert .0 3.4 .3		
Frm Line 1 1 1	Col Line D C B A	E1UNI Horz 0.0 0.0 0.0 0.0	B_SL_F Ve 0.: 1.3 3.4	ert 2 3 4													
Frm Line 5 5 5	Col Line A B C D	Dead Vert 0.2 0.5 0.4 0.2		Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.0 3.0 3.0 1.0	Sno Ver 1.0 3.0 3.0	t	Wind_Le Horz -1.7 0.0 0.0 0.0	eft1 Vert -3.0 -0.2 -1.6 -1.5	Wind_ Horz 0.0 1.7 0.0 0.0	Right1 Vert 1.5 -4.6 -2.5 -0.7	Wind_L Horz -1.7 0.0 0.0 0.0	eft2 Vert -2.3 0.5 -0.8 -0.8	Wind_ Horz 0.0 1.7 0.0 0.0	Right2 Vert 2.2 -3.8 -1.8 0.0	Wind Press Horz -0.7 -2.0 -2.0	
Frm Line 5 5 5	Col Line A B C D	Wind Suct Horz 0.8 2.2 2.2 0.8		Wind_L Horz 0.0 0.8 0.0 0.0	Vert	Wind_Lo Horz -0.8 0.0 0.0 0.0	ng2 Vert -2.3 -0.4 -2.5 -1.9	Seis Horz -0.2 0.0 0.0 0.0	Left Vert -0.2 0.2 0.0 0.0	Sei Hor 0.0 0.2 0.0 0.0	0.3 -0.3 0.0	Seis Long Vert 0.0 0.0 0.0	E2 Ho 0.0 0.0 0.0	0 1 0 3 0 1	_L- /ert .0 3.4 .3		
Frm Line 5 5 5	Col Line A B C D	E2UN Horz 0.0 0.0 0.0 0.0	B_SL_I Ve 0.: 1.3 3.4	ert 2 3 4													
ENDV	VALL	COLU	JMN:		MAXIMUN	/ REACTI	ONS, AN	NCHOR B	OLTS, & E	BASE PL	ATES						
Frn Lin			Load Id	Hmax H	Column_Rec V Vmax	ctions(k Load Id	) Hmin H	V Vmi		Bolt(in) Y [	Bo DIA Width	ase_Platel Lengt	(in) h Thicl	Gro k (ir			
1	[	D	8 10	0.5 0.4		9 8	-0.4 0.5	-1 -1		2 0.62	5 3.500	8.000	0.25	0 0	0.0		
1		С	11 13	1.3 0.0	4.0	12 11	-1.2 1.3	-2	5						0.0		
1		В	14 15 14	1.3 0.0 0.5	4.0	9 11	-1.2 1.3 -0.4	-1	.3						0.0		
1 5		A A	1 8	0.5 0.5	1.3	9 14 9	-0.4 0.5 -0.4	-1	.1						).0		
5		В	10 11	0.4 1.3	2.0 -2.5	8 12	0.5 -1.2	-1 -2	:.1 2						0.0		
5	(	С	16 14 17	0.0 1.3 0.0	-1.3	11 9 11	1.3 -1.2 1.3	-2 -1 -1	.3 2	2 0.62	5 3.500	8.000	0.25	0 0	0.0		
5	[	D	14 1	0.5 0.0	-1.1	9 14	-0.4 0.5	-1	.1 2	2 0.62	5 3.500	8.000	0.25	0 C	0.0		

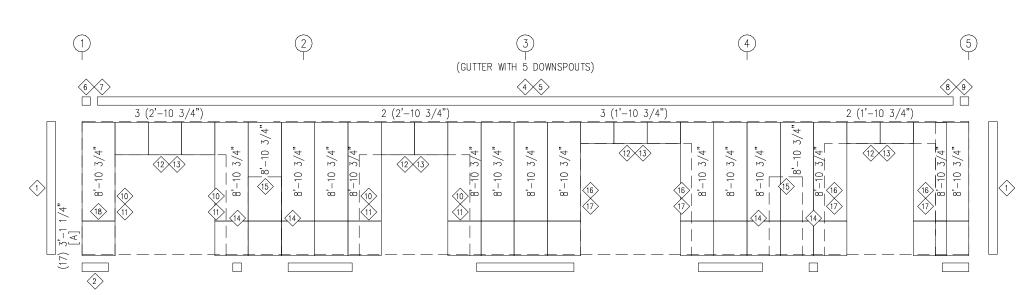
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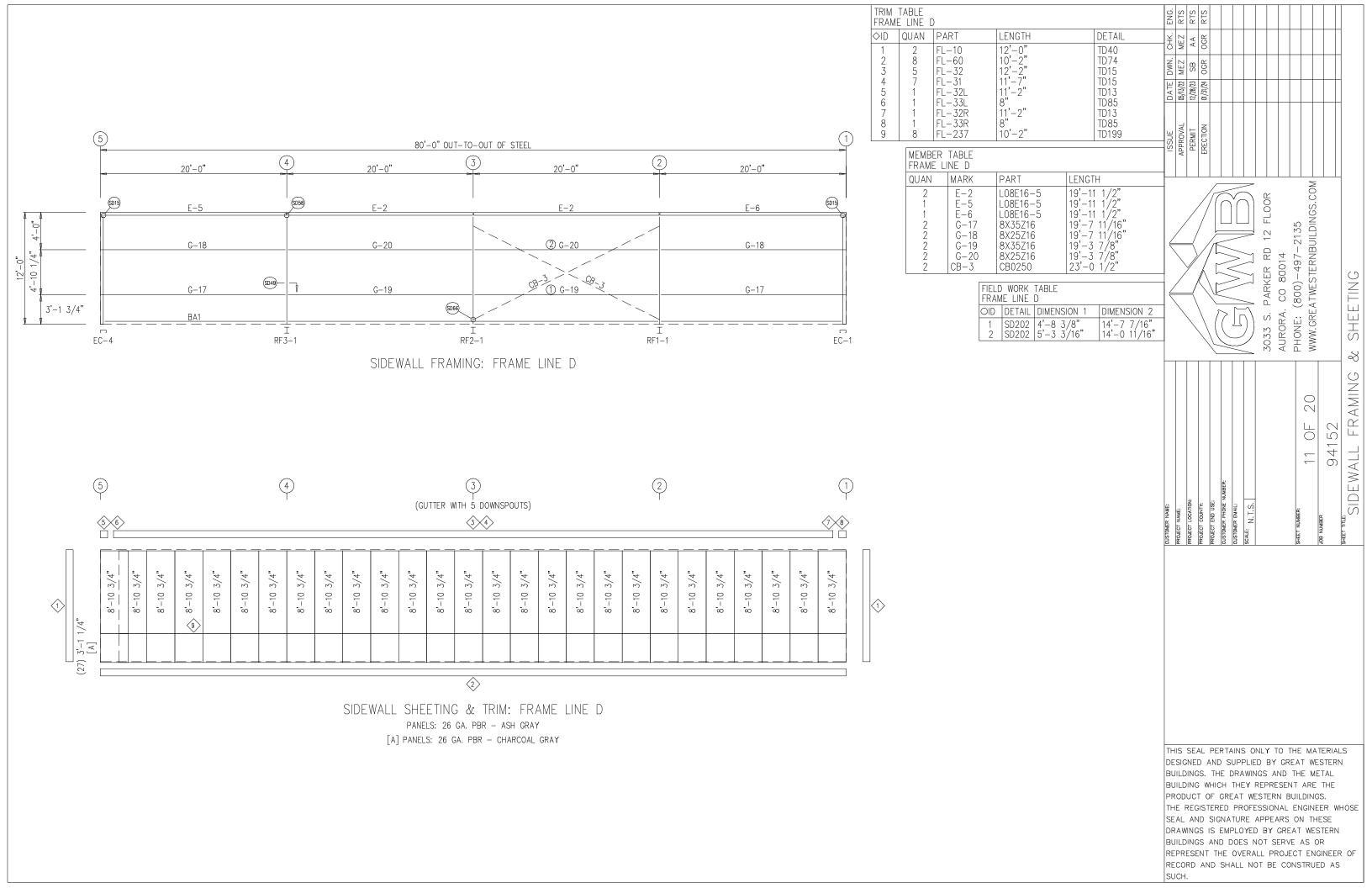
SIDEWALL SHEETING & TRIM: FRAME LINE A

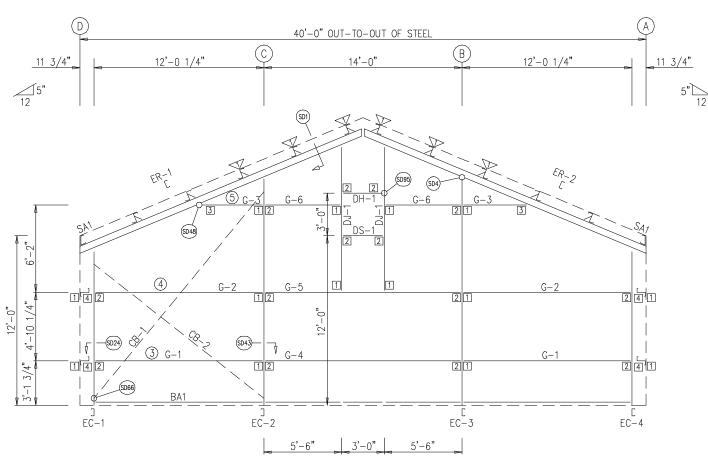
PANELS: 26 GA. PBR - ASH GRAY

[A] PANELS: 26 GA. PBR - CHARCOAL GRAY

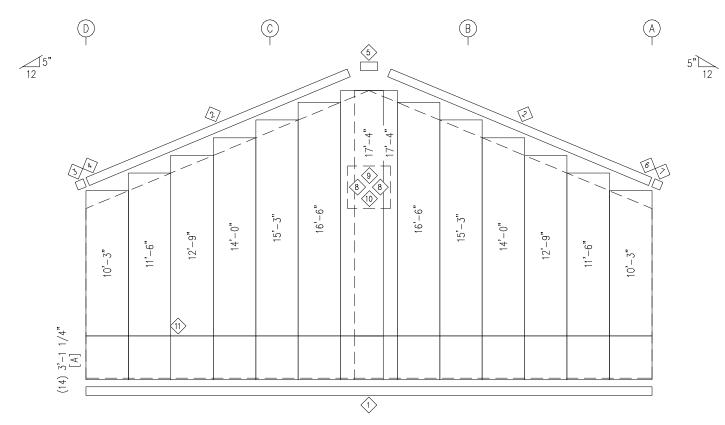
		FR/ QU	BOLT FRAME LOCAT WF-1 WF-1	4 2 4 4 4 4	1 1 4 4 4 4 4	2 4 5 7 1	ME LINE	/ TABLE
	2 1 1 1 1 2 2 2 4 1 1 1 1 1 1 1 2 2 2 2	AME L AN	ION O WF	FL FL FL	FL FL FL FL	FL FL FL	N PA	- - -
	WF-1 WF-2 DJ-2 DJ-3 DJ-4 DJ-5 DJ-6 DH-2 E-1 E-2 E-3 E-4 G-7 G-8 G-9 G-10 G-11 G-12 G-13 G-14 G-15 G-16	TABLE INE A MARK	A -2 1-3	48 52 55 48 237	-32R -33R -55 -48 -55	10 60 32 31 32L 33L	\RT	
CONNECTION FRAME LINES QUAN 1 16 2 8	W8X10 W8X18 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 L08E16-5 L08E16-5 L08E16-5 8X35Z16 8X25Z16 8X35Z16 8X25Z16 8X35Z16	PART	QUAN TYP 8 A32 6 A32 6 A32	10'-4" 7'-2" 3'-4" 10'-3" 10'-2" 10'-2"	11'-2" 8" 9'-3" 9'-2" 10'-7"	12'-0" 10'-2" 12'-2" 11'-7" 11'-2" 8"	LENGTH	
E A MARK	10'-4" 18'-7 7/16" 11'-6 3/8" 11'-6 3/8" 11'-6 3/8" 11'-6 3/8" 11'-6 3/8" 11'-6 3/8" 9'-11 1/2" 19'-11 1/2" 19'-11 1/2" 19'-11 1/2" 2'-7 11/16" 6'-3 7/8" 6'-3 7/8" 3'-11 11/16" 3'-11 11/16" 4'-3 7/8" 4'-3 7/8" 4'-3 7/8" 2'-7 11/16"	LENGTH	5 5/8" 1 3/4" 5 5/8" 1 1/2"	TD51 TD52 TD51 TD51 TD51 TD199	TD13 TD85 TD51 TD51 TD52 TD52	TD40 TD74 TD15 TD15 TD13 TD85	DETAIL	
	CUSTOMER NAME:		$\bigg] \in$		ISSUE	DATE DWN.	CHK.	ENG.
	PROJECT NAME:				APPROVAL	MEZ	MEZ R	RTS
	PROJECT LOCATION:	\			PERMIT	SB	AA R	RTS
	PROJECT COUNTY:			<u> </u>	ERECTION	OGR		RTS
	PROJECT END LISE: CUSTOMER PHONE NUMBER:							
	CUSTOMER EMAIL:	, [[ 						
	SCALE: N.T.S.							
	(*)	3033 S.	PARKER RD 12	FLOOR				
	7	AURORA.	CO 80014					
	SHEET NUMBER: 10 OF 20 V	PHONE:	PHONE: (800)-497-2135 www.greatwesternbuildings.com	35 DINGS, COM				
	SHEET TILE. SIDEWALL FRAMING &	TS.	EETING			-	_	

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ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 GA. PBR - ASH GRAY

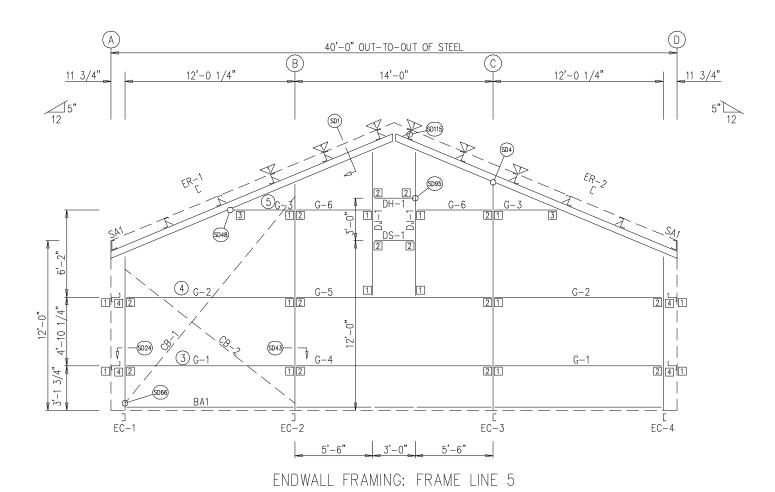
[A] PANELS: 26 GA. PBR - CHARCOAL GRAY

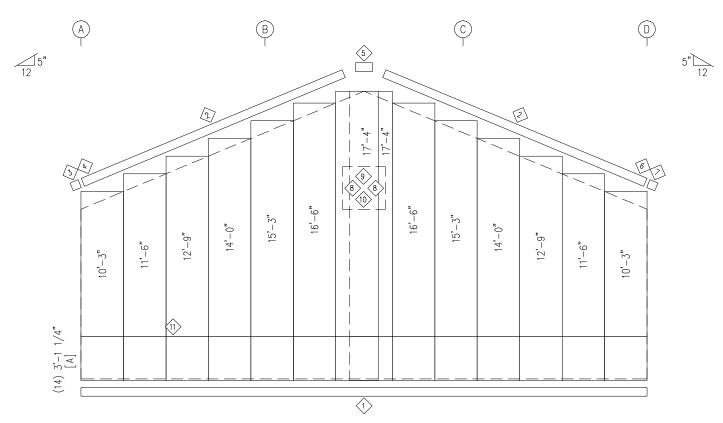
TABLE ME LINE 1 QUAN PART	LENGTH	DETAIL	CHK. ENG. MEZ RTS AA RTS OGR RTS
4 FL-60 2 FL-21 1 FL-21L 1 FL-328L 1 FL-23 1 FL-21R	10'-2" 11'-10" 11'-2" 9 1/2" 1'-4" 11'-2"	TD74 TD35 TD85 TD13	DATE DWN. CH 05/13/22 MEZ ME 12/28/23 SB A 01/31/24 OGR OC
1 FL-328R 2 FL-48 1 FL-52 1 FL-50 4 FL-237	11'-2" 9 1/2" 3'-4" 3'-4" 3'-4" 10'-2"	TD13 TD51 TD52 TD52 TD199	ISSUE APPROVAL PERMIT ERECTION
BOLT TABLE FRAME LINE 1 LOCATION ER-1/ER-2 COLUMNS/RAFTER	QUAN TYPE  4 A325 2 A325 2 A325	DIA LENGTH 5/8" 1 1/2" 5/8" 1 1/2" 5/8" 1 1/2"	FLOOR DINGS.COM
MEMBER TABLE FRAME LINE 1 QUAN MARK 1 EC-1	PART L	LENGTH	3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM
1 EC-2 1 EC-3 1 EC-4 1 ER-1 1 ER-2 2 DJ-1	8x25C14   2 8x25C14   2	10'-10 9/16" 15'-10 5/8" 15'-10 5/8" 10'-10 9/16" 21'-7 3/4" 21'-7 3/4" 3'-10 1/8"	3033 S. P.A. AURORA. C. PHONE: (80 WWW.GREAT
1 DH-1 1 DS-1 2 G-1 2 G-2 2 G-3 1 G-4 1 G-5 2 G-6 1 CB-1 1 CB-2	8x25C16 8x25C16 8X35Z16 8X25Z16 8x25Z16 8X35Z16 8X35Z16 8X25Z16 8X25Z16 6X25Z16 8X25Z16	9,-10 1/8" 2'-11 1/2" 2'-11 1/2" 11'-7 15/16" 11'-7 15/16" 3'-8" 13'-11 1/2" 13'-11 1/2" 5'-1 11/16" 19'-7"	12 OF 20
	1 14 2 14 3 2		PROJECT NAME: PROJECT NAME: PROJECT LOCATION: PROJECT COUNTY: PROJECT FOUN USE: CUSTOMER ENAUL: SCALE: N. T.S.
	FLANGE FRAME ▽ID QU		
FRAME OID DI 3 SI 4 SI	ORK TABLE LINE 1  ETAIL DIMENSION  0202 2'-1 5/16" 0202 2'-10 1/8" 0202 3'-0 5/8"	1 DIMENSION 2 8'-9 7/16" 6'-0 1/2"	
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ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 GA. PBR - ASH GRAY

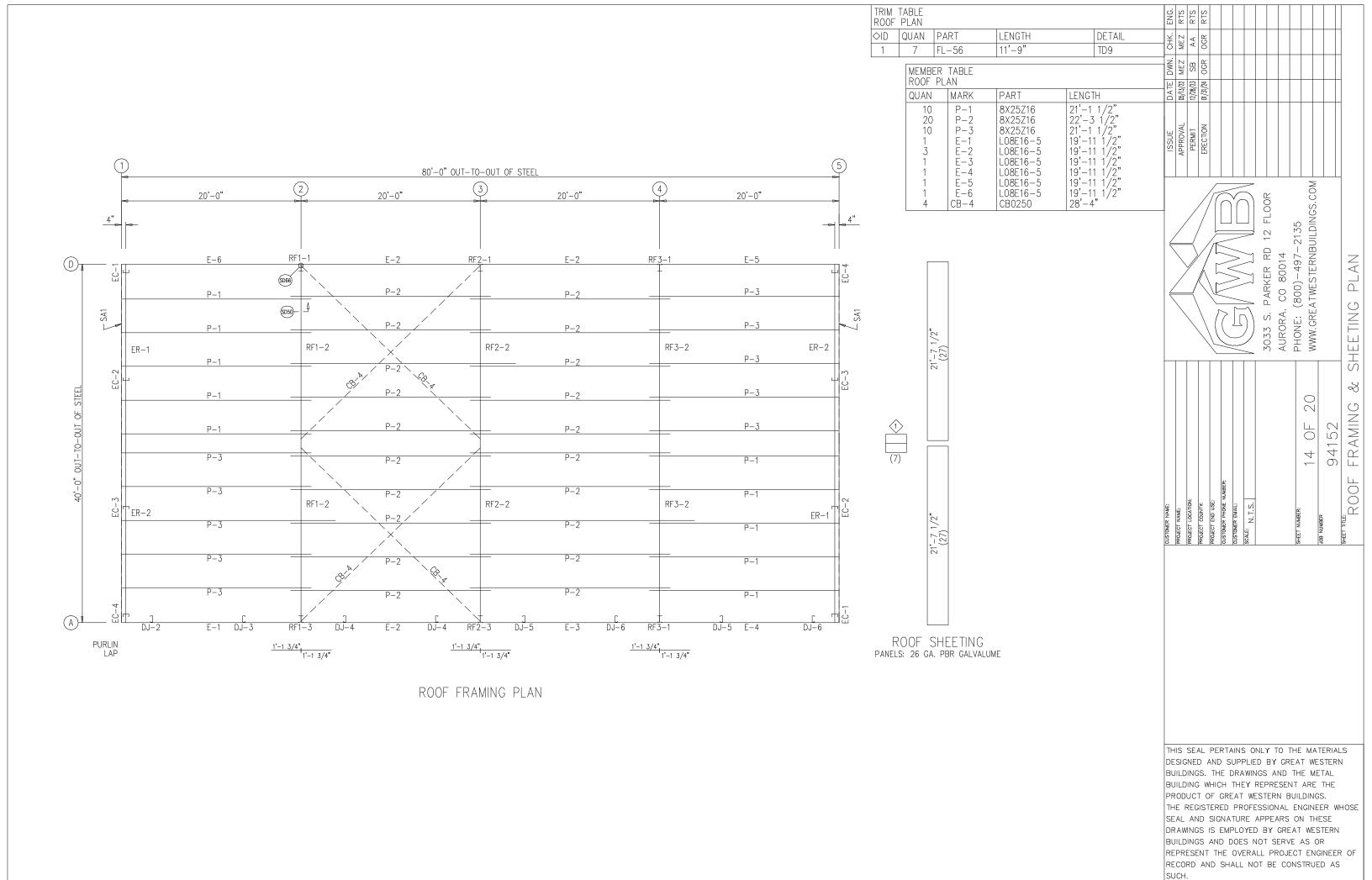
[A] PANELS: 26 GA. PBR - CHARCOAL GRAY

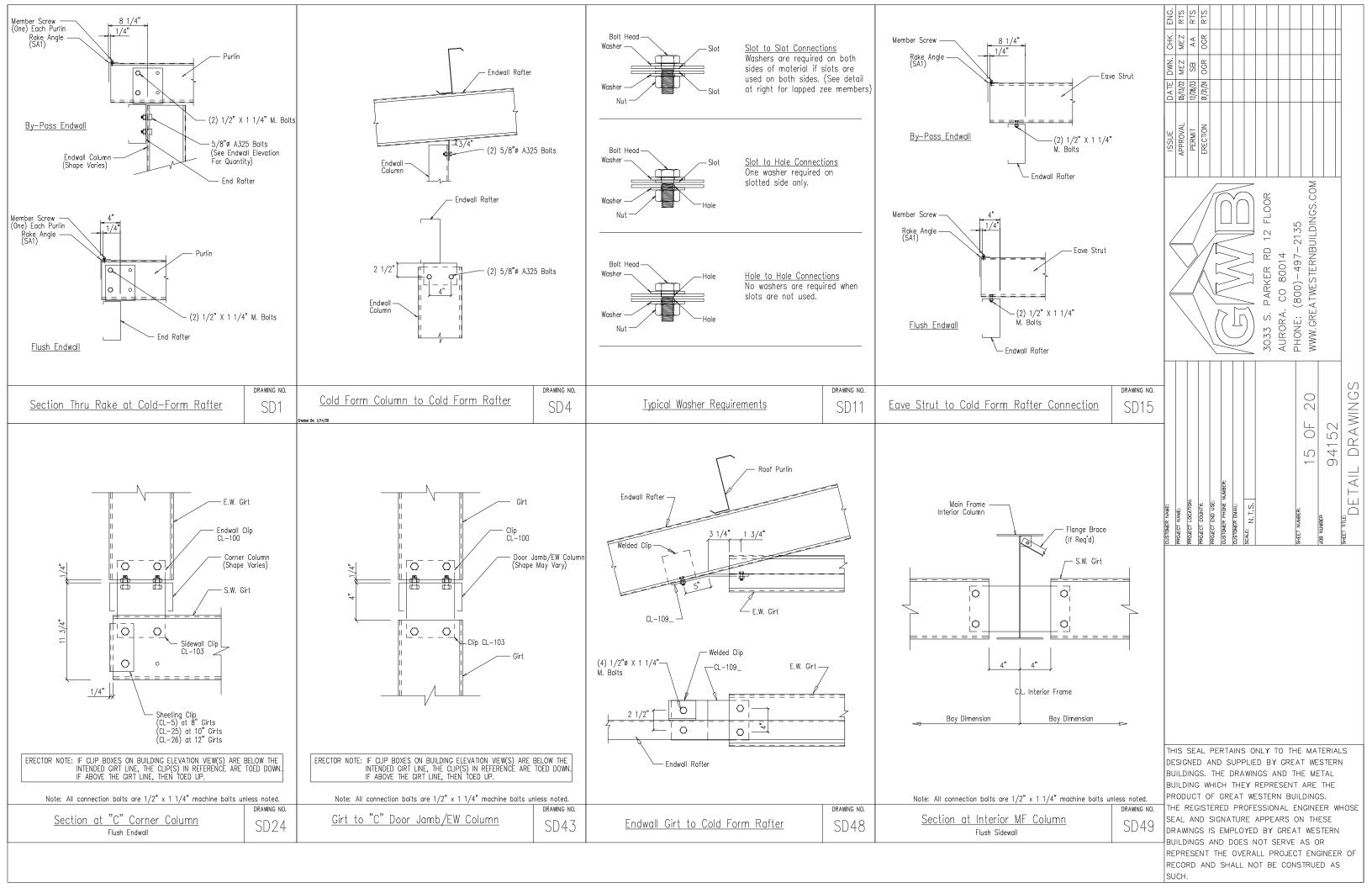
TRIM FRAM	TABLE ME LINE S	5							ENG.	RTS	RTS	RTS								
◇ID 1	QUAN 4	PART FL-60		LENGTH 10'-2"		Т	ETAIL D74		O.H.	MEZ	AA	OGR								
2 3 4	2 1 1	FL-21 FL-21L	- RI	111' 1∩"		T	D35 D85 D13		DWN.	MEZ	SB	0GR								
5 6		FL-32 FL-23 FL-21F FL-32	3	11'-2" 9 1/2" 1'-4" 11'-2" 9 1/2" 3'-4"		Т	D85		DATE	05/13/22	12/28/23	01/31/24								
7 8 9	1 2 1	FL-32 FL-48 FL-52	3R	9   1/2"   3'-4"   3'-4"   3'-4"   10'-2"		T	D13 D51 D52		E	JVAL	AIT.	NOIT								
10	1 4	FL-50 FL-23	7	3'-4" 10'-2"		T	D52 D199		ISSNE	APPROVAL	PERMIT	ERECTION								
	BOLT TAI FRAME L	BLE INE 5																	<u> </u>	
	LOCATION ER-1/ER COLUMNS			QUAN 4	TYPE A325	DIA 5/8"	1	ENGTH 1/2"			/	//		$\int$		FLOOR				
	COLUMNS JAMBS/R	S/RAFTE RAFTER	:R	2 2	A325 A325	5/8" 5/8" 5/8"	1	1/2" 1/2" 1/2"	 	/	, /	/ /,				12		PHONE: (800)-497-2135	5	
	FRAM	ER TAB E LINE	5						] `					<u></u>	\	ER RD	80014	(800)-497-2135		
	QUAN 1	EC-	-1	PART 8x25C16		LENGTH 10',-10	9/16"		$\mid$ $\in$	_	$\prec$			<u> </u>	_	PARKER	CO 8	300)- TWFC	)    -	IN C
	1 1 1	EC- EC- EC-	- 3	8x25C12 8X35C14 8x25C16		10'-10 15'-10 15'-10 10'-10 21'-7 3 21'-7 3 9'-10 1 2'-11 1	5/8" 5/8" 9/16"			\	/	\	7			Ś		VE: (S	7	
	1	ER-	-1 -2	8x25C14 8x25C14		21'-7 3 21'-7 3	5/4" 5/4"				`	//		$\leq$		3033	AURORA.	PHONE:		T.S.
	1 1	DJ- DH- DS-	-1	8x25C16 8x25C16 8x25C16		9 – 10 1 2'–11 1 2'–11 1	/8 /2" /2"													()   \&
	2 2 2	G- G- G-	-1 -2	8X35Z16 8X25Z16 8X25Z16		11'-7 1 11'-7 1	5/16" 5/16"											000		Ž
	1	G- G-	-4 -5	8X35Z16 8X25Z16		13'-11 13'-11	1/2" 1/2"											\ 		FRAMING
	1 1	G- CB- CB-	-1	8X25Z16 CB0250 CB0250		2 - 11 1 2' - 11 1 11' - 7 1 11' - 7 1 3' - 8" 13' - 11 13' - 11 5' - 1 11 19' - 7" 16' - 0 1	/16" /4"											1,3	) 4	-
				CONNI		PLATES	/ -						UMBER:							DWALL
				□ID	QUAN	MARK	,		NAME:	IAME:	OCATION:	COUNTY:	Z	EMAIL:	1.T.S.			ABER:	æ	
				1 2 3 4	14 14 2 4	CL-103 CL-100 CL-109 CL-5	)		CUSTOMER NAME:	PROJECT NAME:	PROJECT LOCATION	PROJECT COUNTY:	CUSTOMER PHONE	CUSTOMER EMAIL:	SCALE: N.			SHEET NUMBER	JOB NUMBER	SHEET TIL
						E BRACE		LE												
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		FIE	LD WC	RK TABLE	'	6	FB29.	<u> </u>												
		FR OII	AME L		NSION	1 DIN	MENSI	ON 2												
		4	- SD2	202   2′-1	5/16' 0 1/8' 0 5/8"	" 8'- 6'-	-9 7, -0 1, 	/16" /2"												
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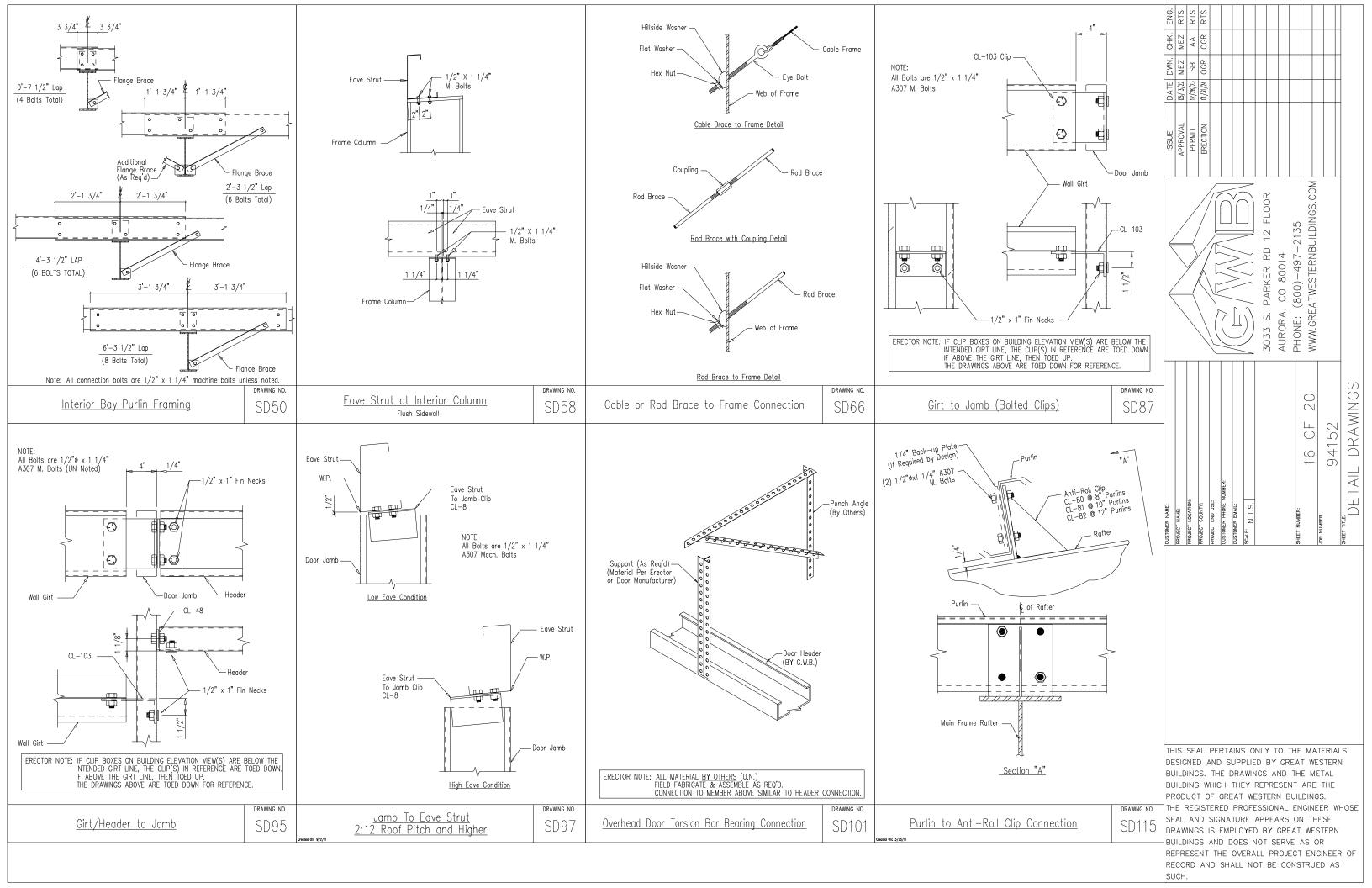
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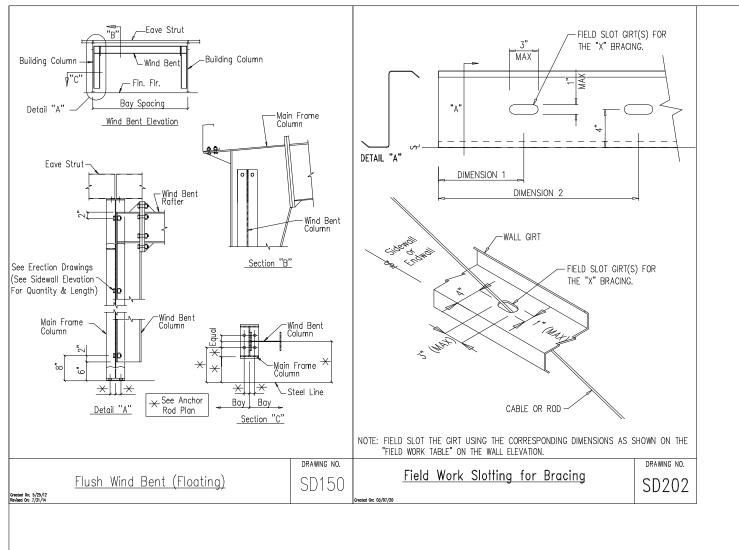
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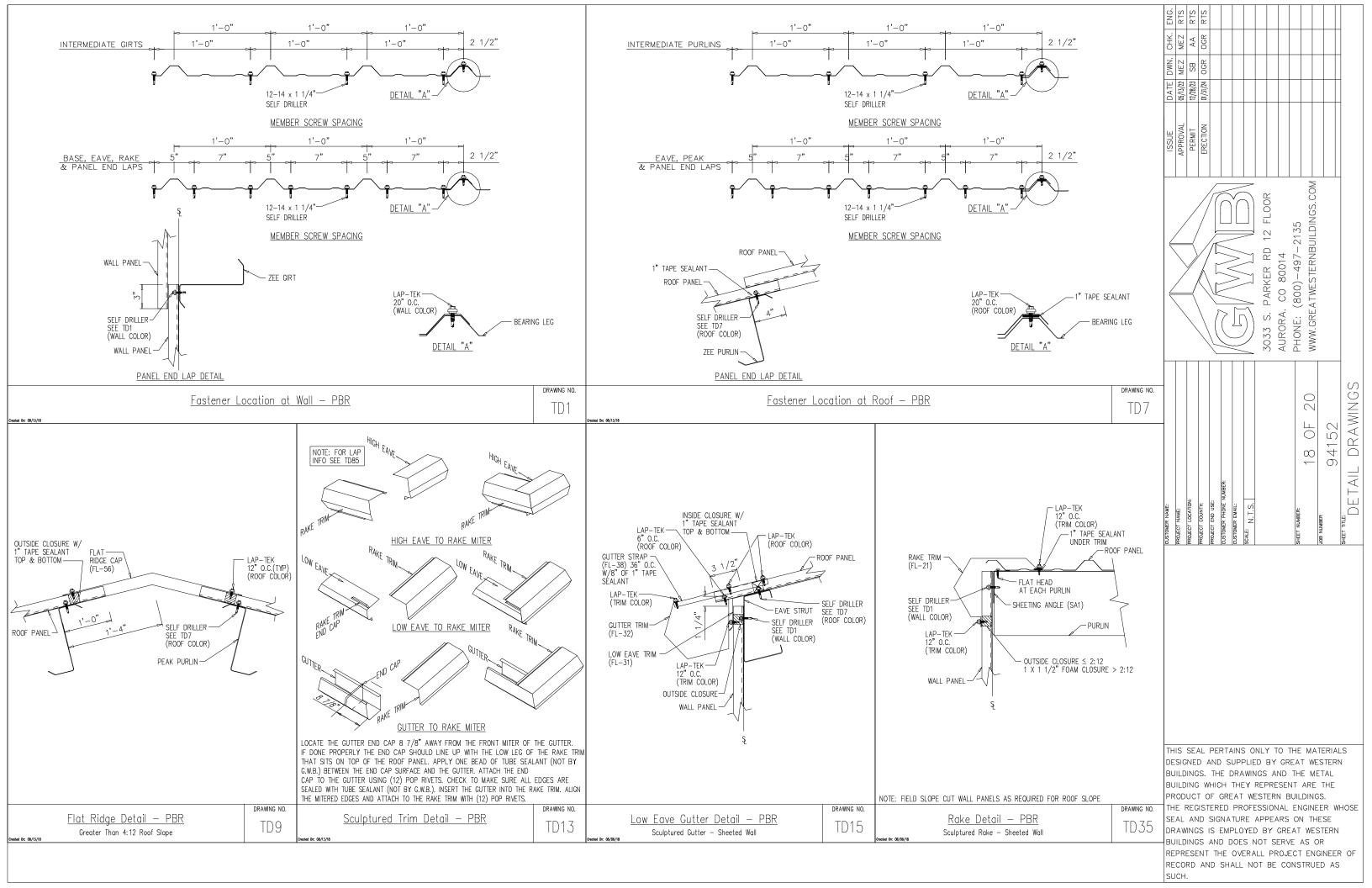


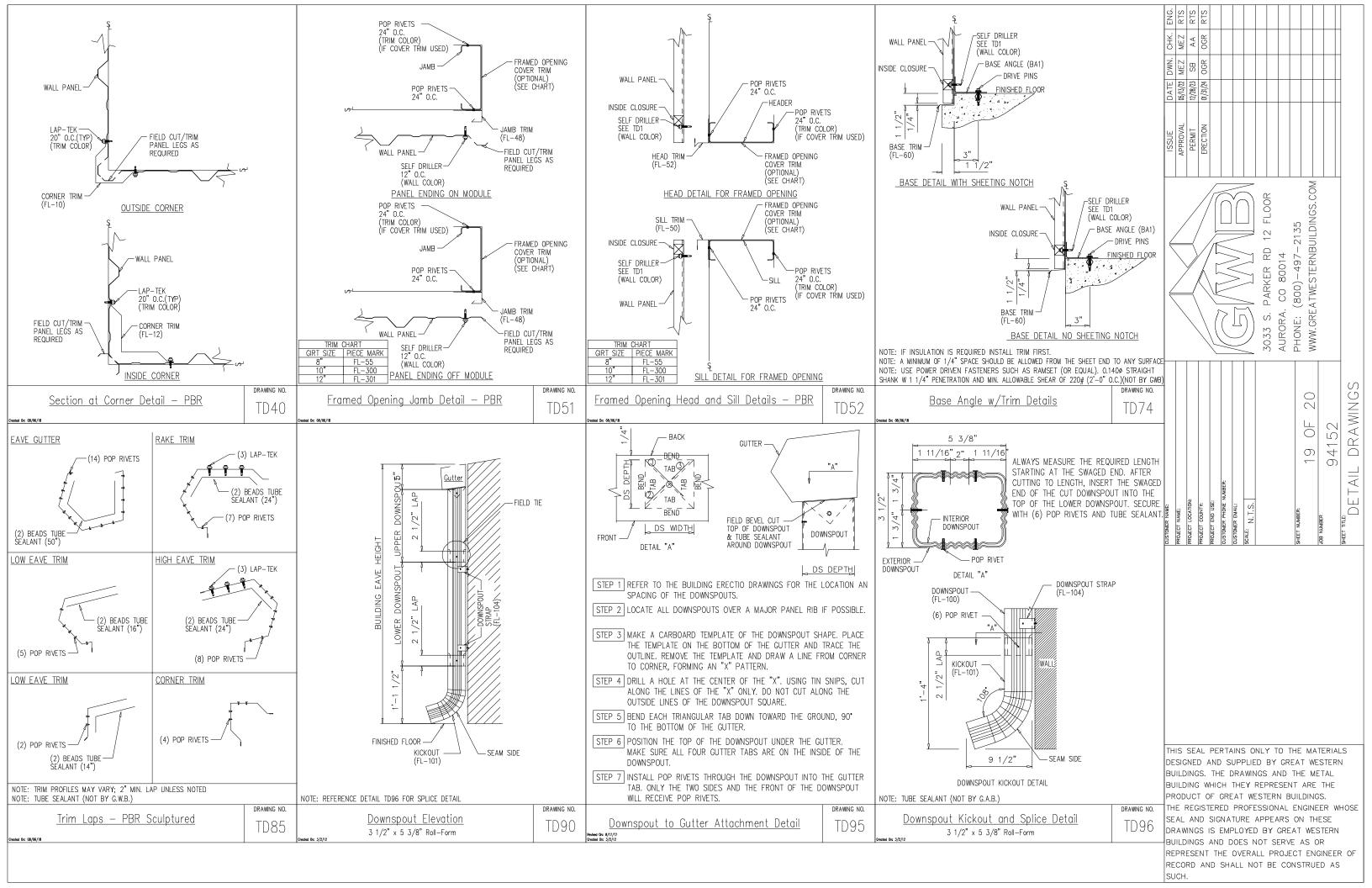


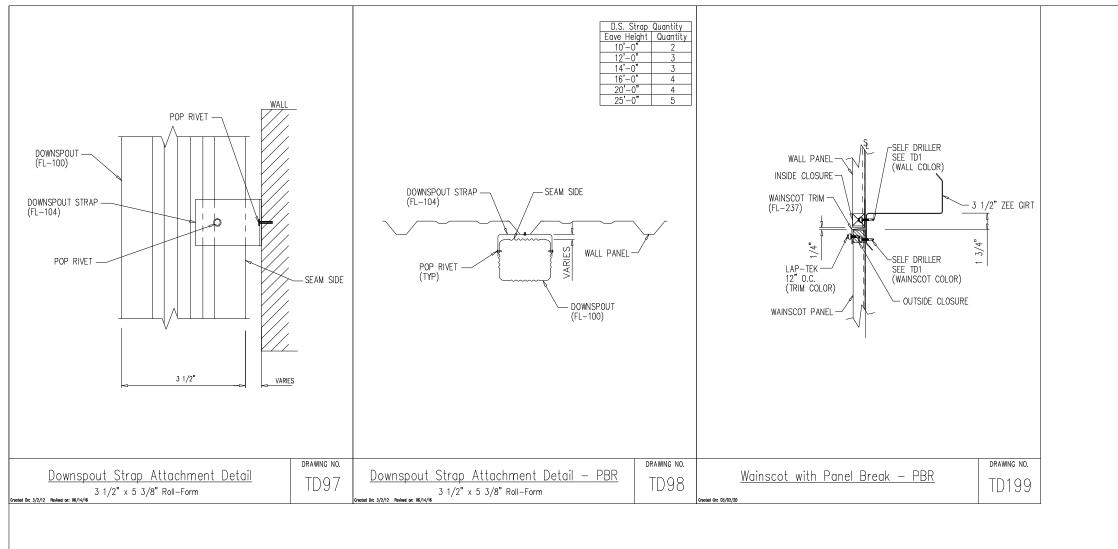


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DATE DWN. CHK.	AL 05/13/22 MEZ MEZ	. 12/28/23 SB AA	IN 01/31/24 OGR OGR											
ISSUE ISSUE	APPROVAL	PERMIT	ERECTION					000 13 01 00 03 70 0 0 5 5 0 5	JUSS S. MARKER RD IZ FLOOR	AURORA. CO 80014	PHONE: (800)-497-2135	WWW.GREAIWESIERINGS.COM		
CUSTOMER NAME:	PROJECT NAME:	PROJECT LOCATION:	PROJECT COUNTY:	PROJECT END USE:	CUSTOMER PHONE NUMBER:	CUSTOWER EMAIL:	SCALE: N.T.S.	N	')	4	SHEET NUMBER:		94152	SHEET IIILE:

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