

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI EXTRA HIGH STRENGTH
	CABLE BRACE	A475 - TYPE 1	EXTRA HÌGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH

MACHINE BOLTS & NUTS
HIGH STRENGTH BOLTS (1"\$ & LESS)
HIGH STRENGTH BOLTS (1"\$ TO 1 1/2"\$)
ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)
A36/A307/F1554 Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE $-5/8"\phi \times 0'-1$ 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS — 1/2" ø x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

Buildings in seismic design category c or lower and/or with crane systems 10 tons or less do not require turn of the nut pre tensioning

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE - BETWEEN ROOF PANELS & RIDGE CA - BETWEEN WALL PANELS & EAVE/GABLE TRIM

.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO
PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE MISTIS. THE CURRENTIAL OF MINOR MISTIS BY THE USE OF MINITERIAL OF THE MINISTER OF THE MINISTER OF THE MINISTER OF THE MINIST STREAM OF THE REPLACEMENT OF MINIST SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISTITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION.

THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION.
ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS
CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO
CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITE
CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
ATTECT EDITION)

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLAN COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF TI STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPOR, SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED ITHE RECECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECT NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCIBOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDAL BRACEDE LATES EDITION.) PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PET THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOT THE FOUNDATION OF FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COL REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE FOUNDATION OF THE BUILDING BESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING BY SYSTEMS MAINTAIL) SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATION AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVE IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST ENTITION)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHER ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNI SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper, both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wring, or tubing onto galvalume should be

2.13 <u>SAFETY COMMITMENT</u> METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFELY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTITOL OF M.B.: IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY A HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAY CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUB SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY N WHERE APPLICABLE, ARE RECOMMENDED.

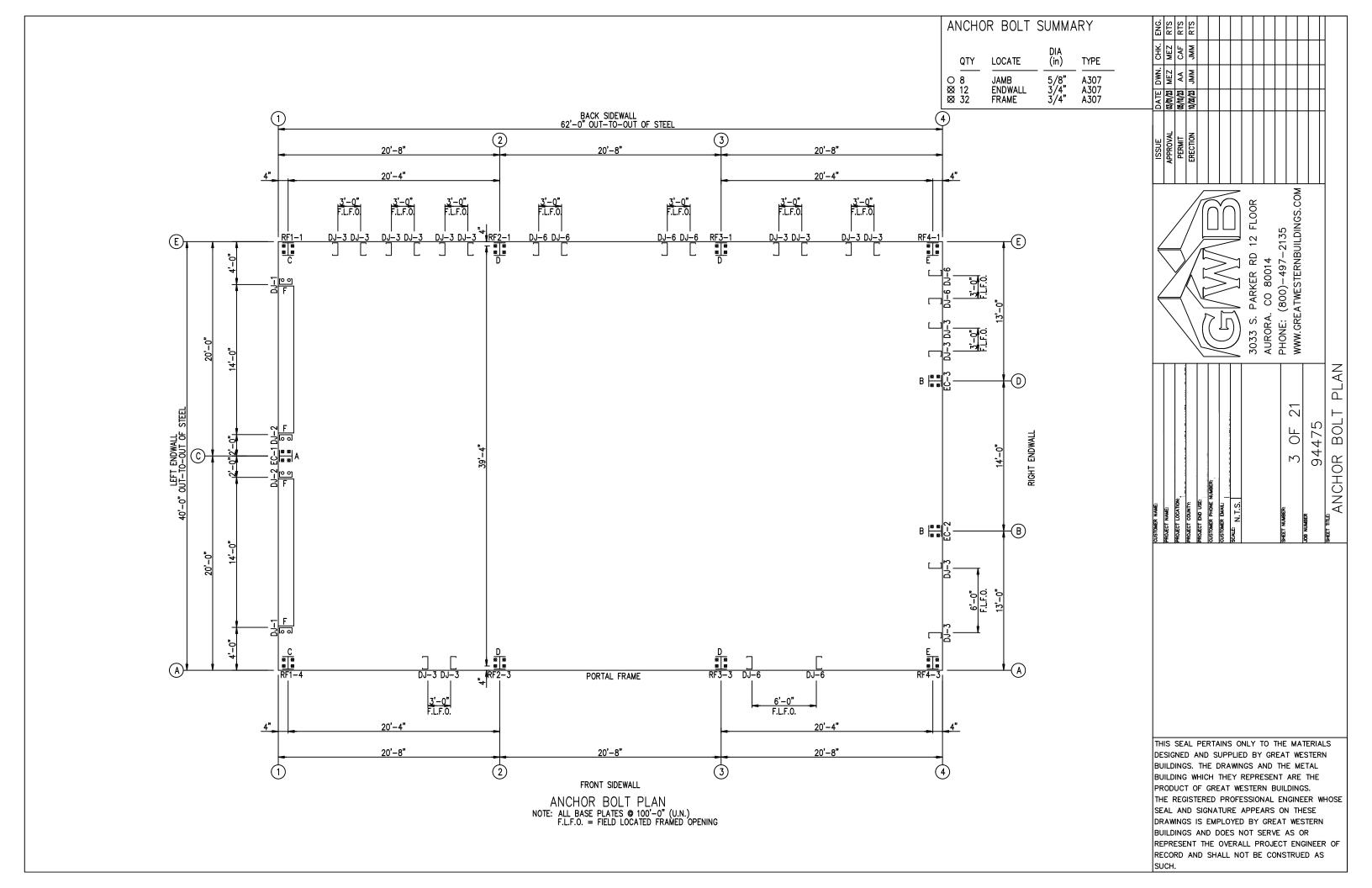
2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

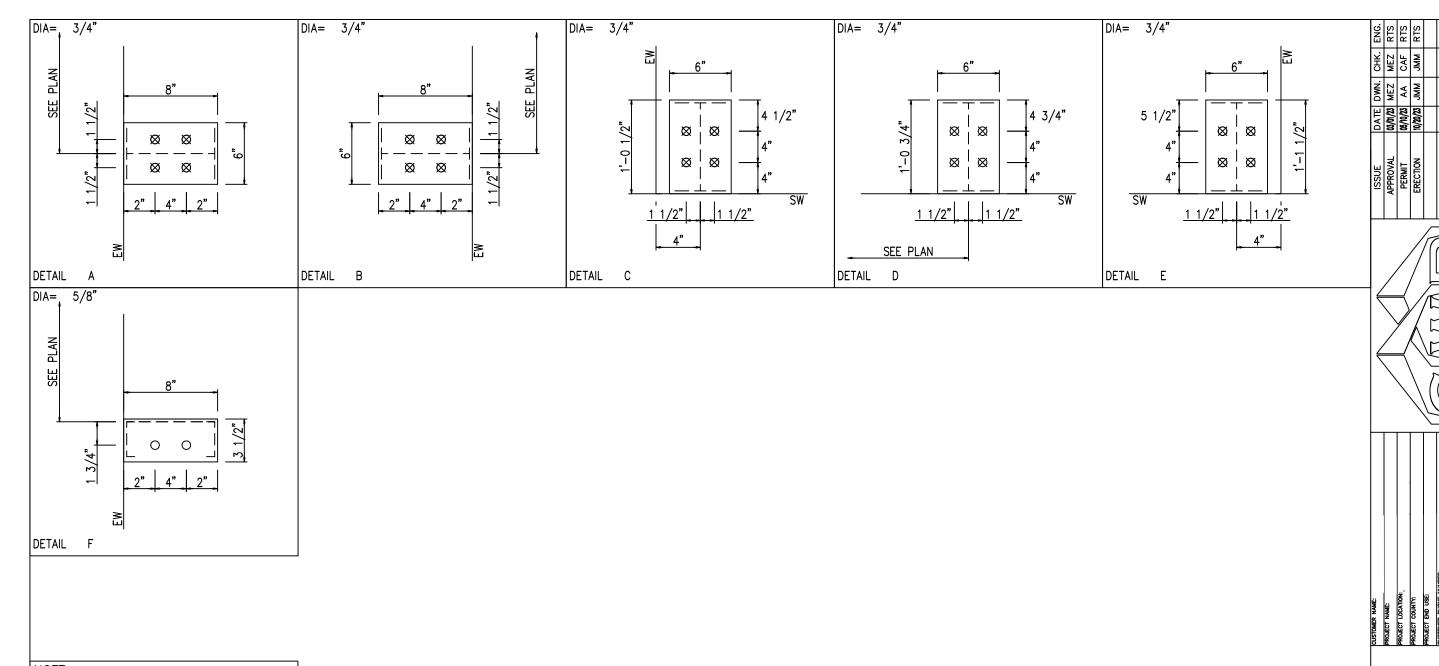
2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OFF PUR DUE SEE TABLE PELOW. ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION. SECTION A8.4. PAGE XI-A8-2

		BUILDING LOADS	ENG.
	THIS STRUCTURE HAS BEEN DESIGNED	IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:	CHK. OAF
TO BE AND THE COLUMN OF THE CO	DESIGN LOADS: DESIGN CODE / WIND CODE OCCUPANCY / RISK CATEGORY ENCLOSURE ROOF DEAD LOAD (D) (PSF) ROOF COLLATERAL LOAD (C) (PSF) WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF) WIND ENCLOSURE CLASSIFICATION LIVE LOAD PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF) SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is) THERMAL FACTOR, (Ct) SEISMIC LOAD SEISMIC LOAD SEISMIC LOAD SEISMIC LOAD SEISMIC FORCE FACTOR, (Ie) SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM TOTAL DESIGN BASE SHEAR, (V) (KIPS) RESPONSE MODIFICATION FACTORS, (R) SEISMIC RESPONSE COEFFICIENTS, (Cs) ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	: IBC-21 : II - Normal : Enclosed : 2.00 : 6.00 : 115.0 : C : 0.18/-0.18	COSTONER NAME: PROJECT NAME: SCALE DATE DWN. CI APPROVAL DÁJ0/12 MEZ M PERMIT 15/00/12 MAC M PROPORA. CO 80014 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM PROJECT NAME: PROJECT NAME: PROMIT 15/00/12 MAC M PROPORA. CO 80014 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM PROFILE T
THO POPONIE	BUILDING DESCRIPTION: WIDTH (FT) : 40.0 LENGTH (FT) : 62.0 EAVE HEIGHT AT BSW (FT) : 18.0 EAVE HEIGHT AT FSW (FT) : 18.0 ROOF SLOPE AT BSW : 5.0:12 ROOF SLOPE AT FSW : 5.0:12 BAY SPACING (FT) : 3 AT 20.67 COVERING AND TRIMS: ROOF PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : CHARCOAL GRAY TRIM COLORS GABLE/EAVE : CHARCOAL GRAY WALL PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : CHARCOAL GRAY WALL PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : COAL BLACK TRIM COLORS CORNER : CHARCOAL GRAY FRAMED OPENING : CHARCOAL GRAY BASE : COAL BLACK SOFFIT PANELS & TRIMS PANEL TYPE : 26 GA. PBR COOL BLACK TRIM COLORS : COAL BLACK TRIM COLORS : COAL BLACK COAL BLACK TRIM COLORS : COAL BLACK		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.





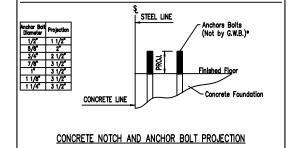
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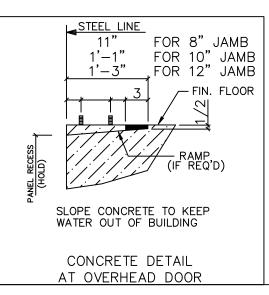
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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3033 S. PARKER RD 12 FLOOR
AURORA. CO 80014
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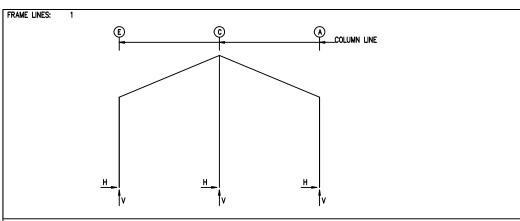
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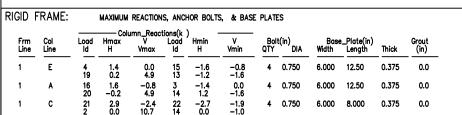
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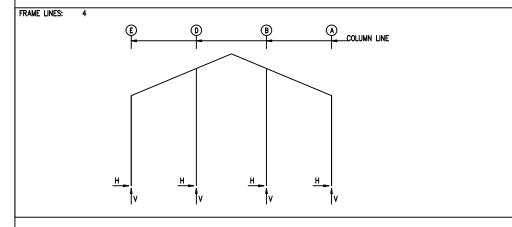
DETAIL

BOL

ANCHOR



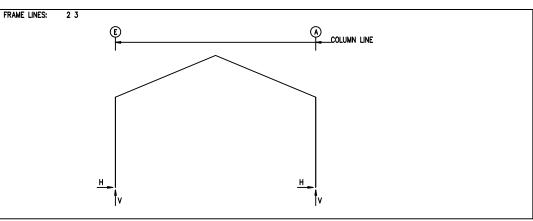




RIGID F	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BAS	E PLATE	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
4	E	14 8	2.4 1.1	1.9 7.3	5 13	-2.0 -1.6	-3.7 -4.8	4	0.750	6.000	13.50	0.375	0.0
4	A	6 7	2.0 -1.1	-3.7 7.3	13 14	-2.4 1.6	1.9 -4.8	4	0.750	6.000	13.50	0.375	0.0
4	D	23 25	1.9 0.0	-2.4 7.9	24 14	-1.4 0.0	-2.5 -4.6	4	0.750	6.000	8.000	0.375	0.0
4	В	21 27	1.9 0.0	-2.4 7.8	26 13	-1.4 0.0	-2.5 -4.6	4	0.750	6.000	8.000	0.375	0.0

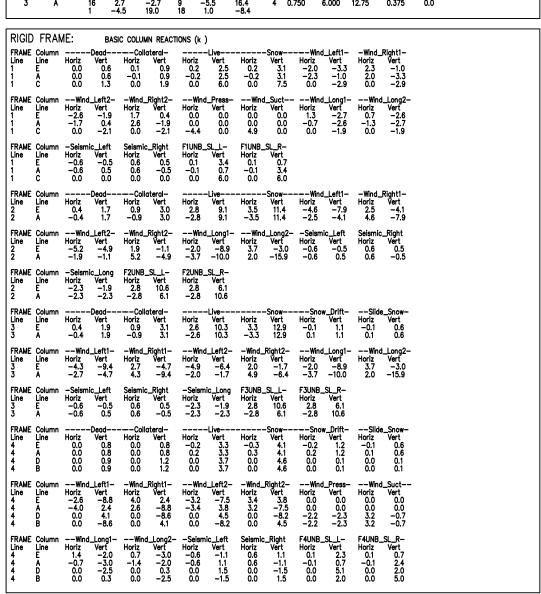
BUIL	DING	BRACI	NG RE	ACTIO	ONS			
Loc Wo	all — Line	- Col Line	Horz ±	Reacti ind — Vert	ions(k) - —Sei Horz	smic - Vert	Panel_Shear - (lb/ft) Wind Seis	Note
L_EW F_SW R_EW	1 A 4 F	2,3	4.4	7.0	2.7	4.3		(h) (b) (h)
B_SW	Ē	Torsion	al Bracin	g Used				(ii)
(b)Wind (h)Rigid	i bent d frame	in bay, b e at endv	ase abov vall	e finish	floor			
Reactio	ons for	seismic	represent	shear 1	force, El	1		

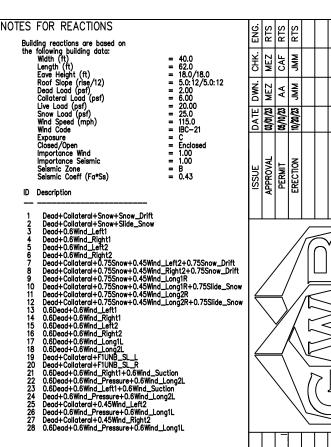
ANCHO	R BOLT	SUMMA	ARY
QTY	LOCATE	DIA (in)	TYPE
O 8 Ø 12 Ø 32	JAMB ENDWALL FRAME	5/8" 3/4" 3/4"	A307 A307 A307

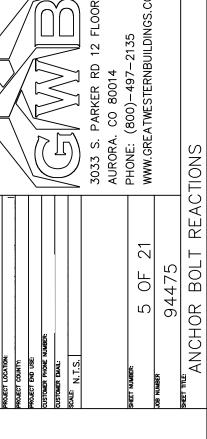


RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCH	HOR BOLT	rs, & Bas	E PLATE	S				
Frn Lin		Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Boli QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2	E	12	5.7 4.9	12.0 16.1	15 17	-2.9 -0.9	-1.9 -4.3	4	0.750	6.000	12.75	0.375	0.0
2	A	16 2	2.9 -4.9	-1.9 16.1	10 18	-5.7 0.9	15.0 -8.5	4	0.750	6.000	12.75	0.375	0.0

RIGID	FRAME:		MAXIMUM	REACTION	IS, ANC	HOR BOLT	S, & BAS	E PLATE	S				
Frm Line		Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Boli QTY	t(în) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
3	E	11	5.5 4.5	13.4 19.0	15 13	-2.7 -2.3	-2.7 -4.5	4	0.750	6.000	12.75	0.375	0.0
3	A	16 1	2.7 -4.5	-2.7 19.0	9 18	-5.5 1.0	16.4 -8.4	4	0.750	6.000	12.75	0.375	0.0

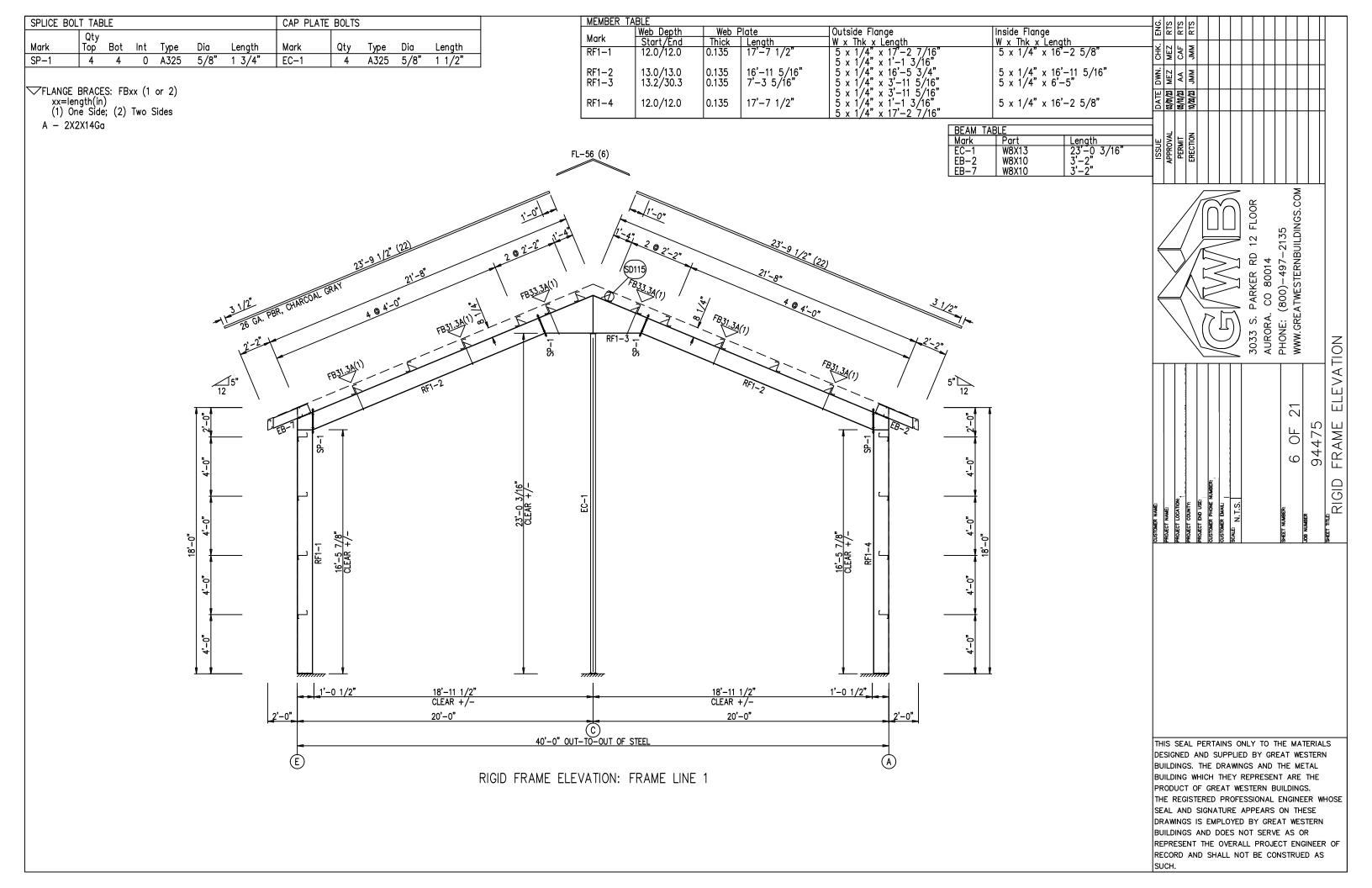


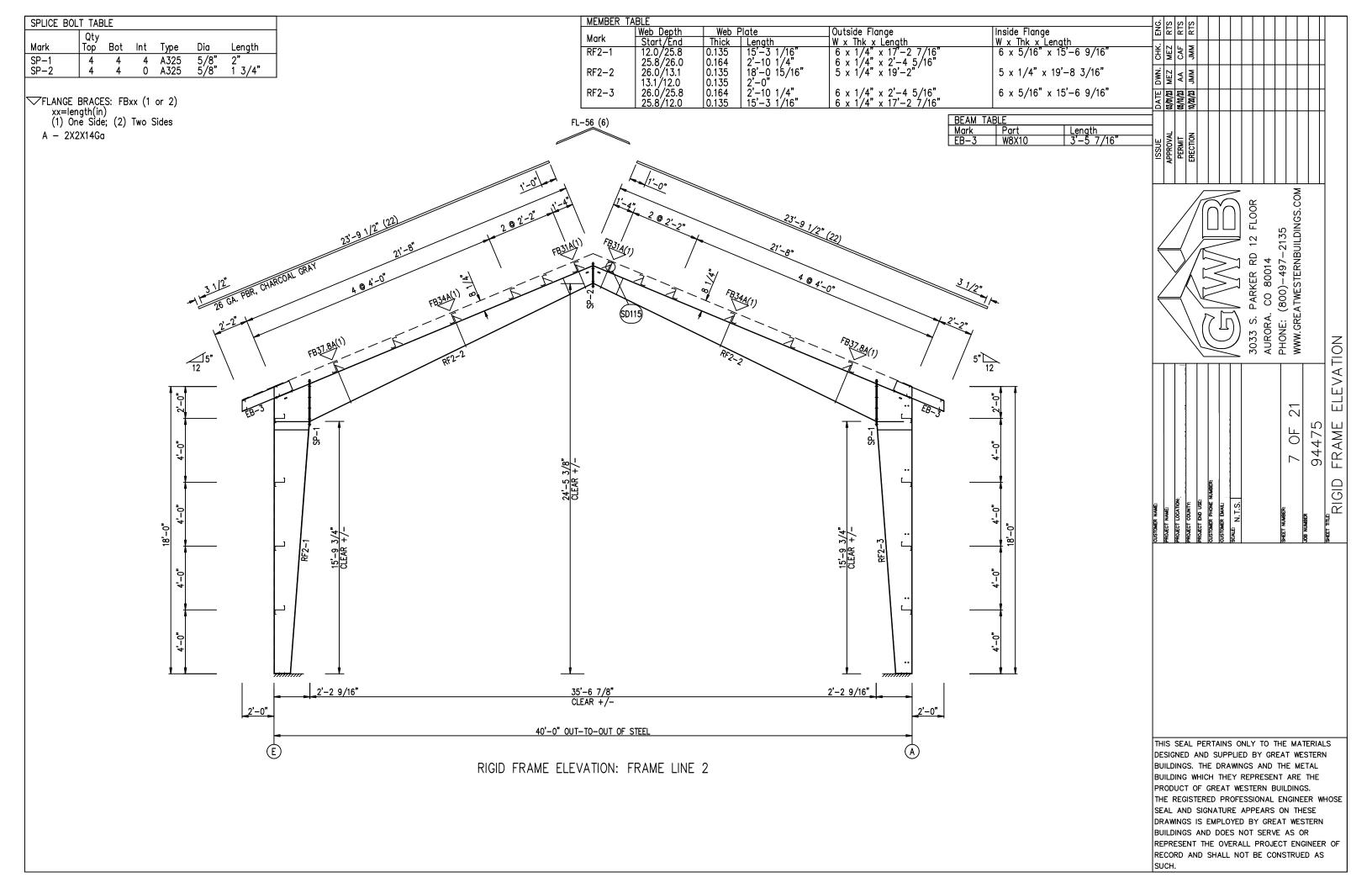


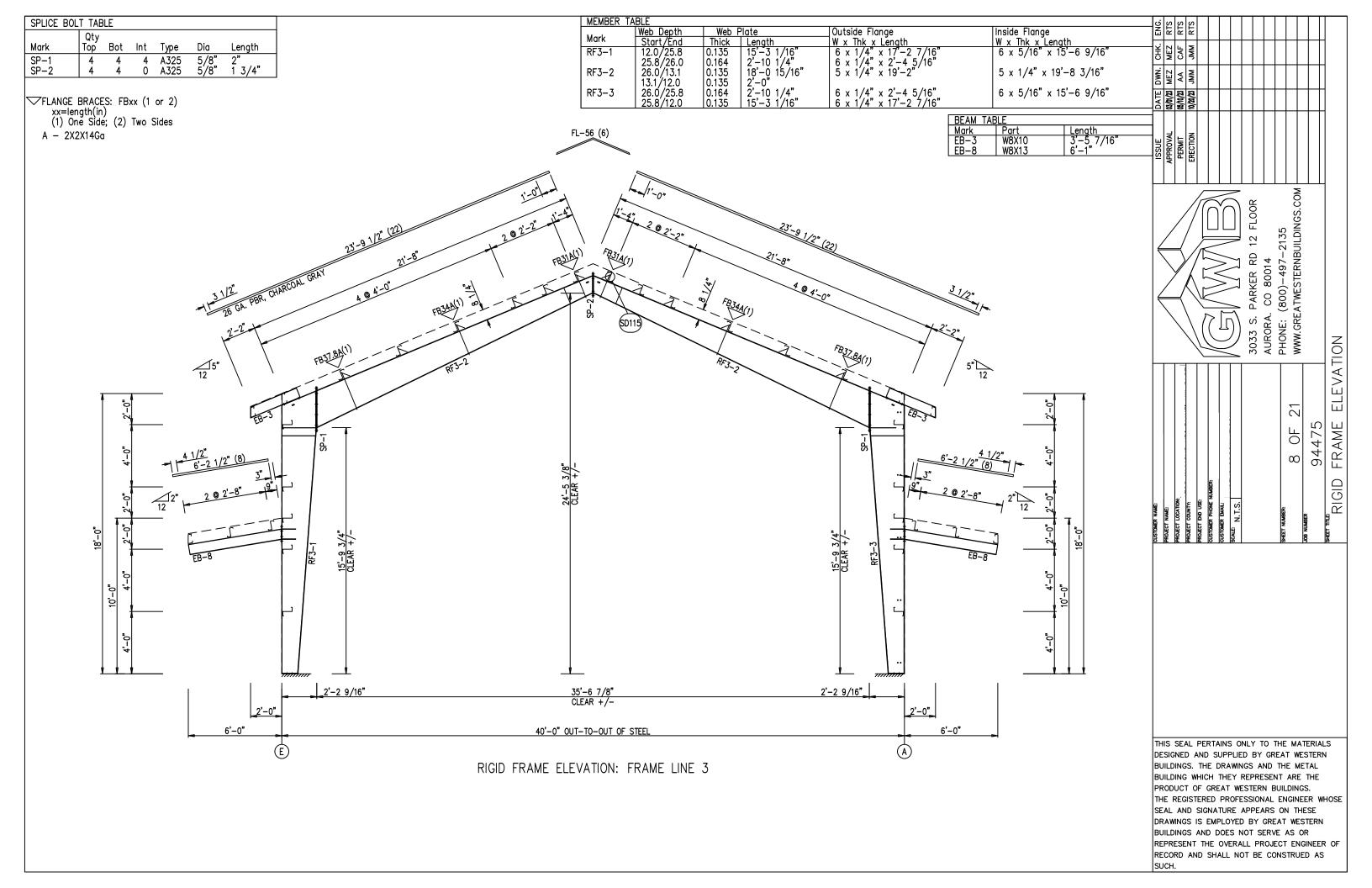


FLOOR

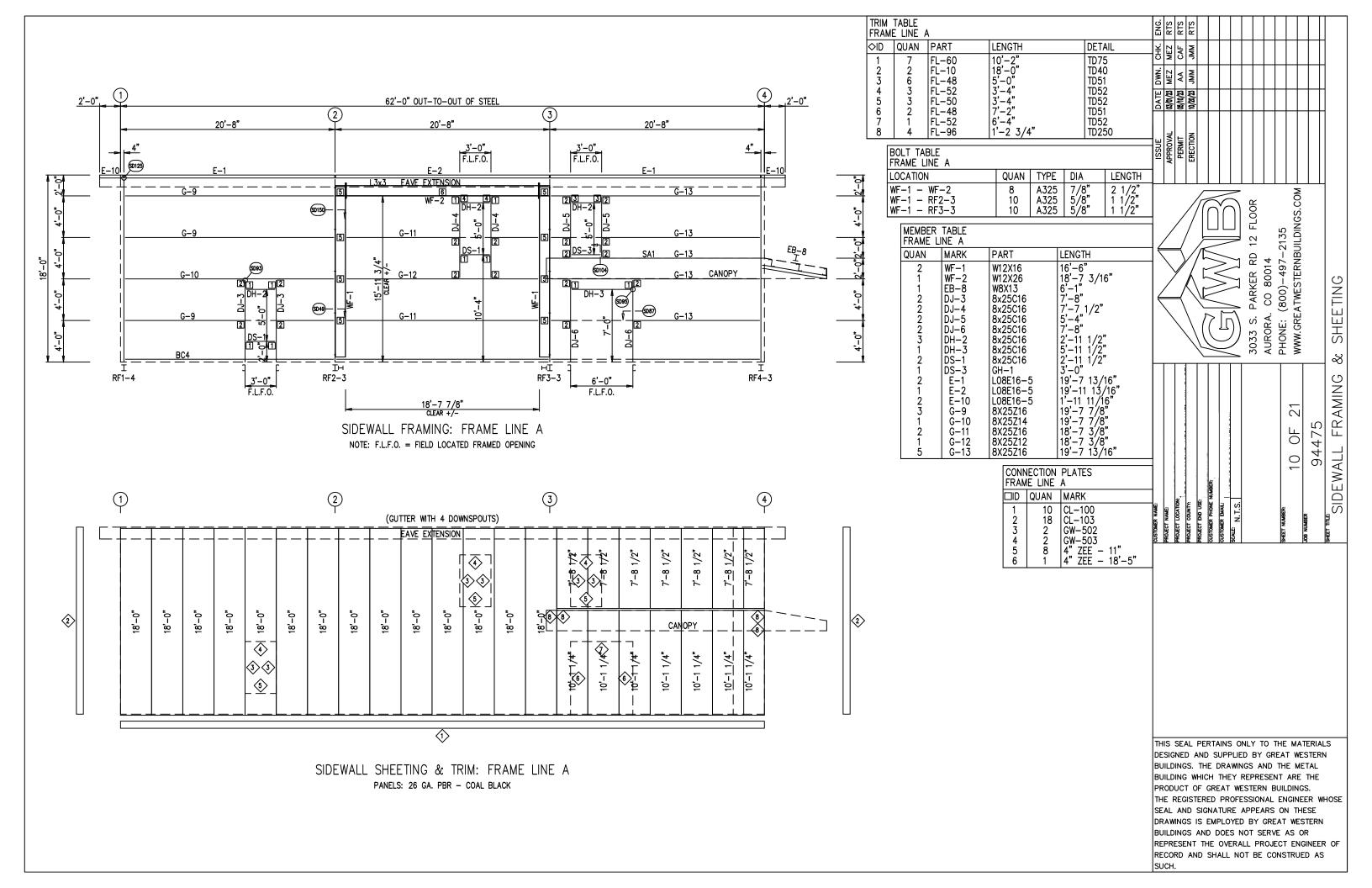
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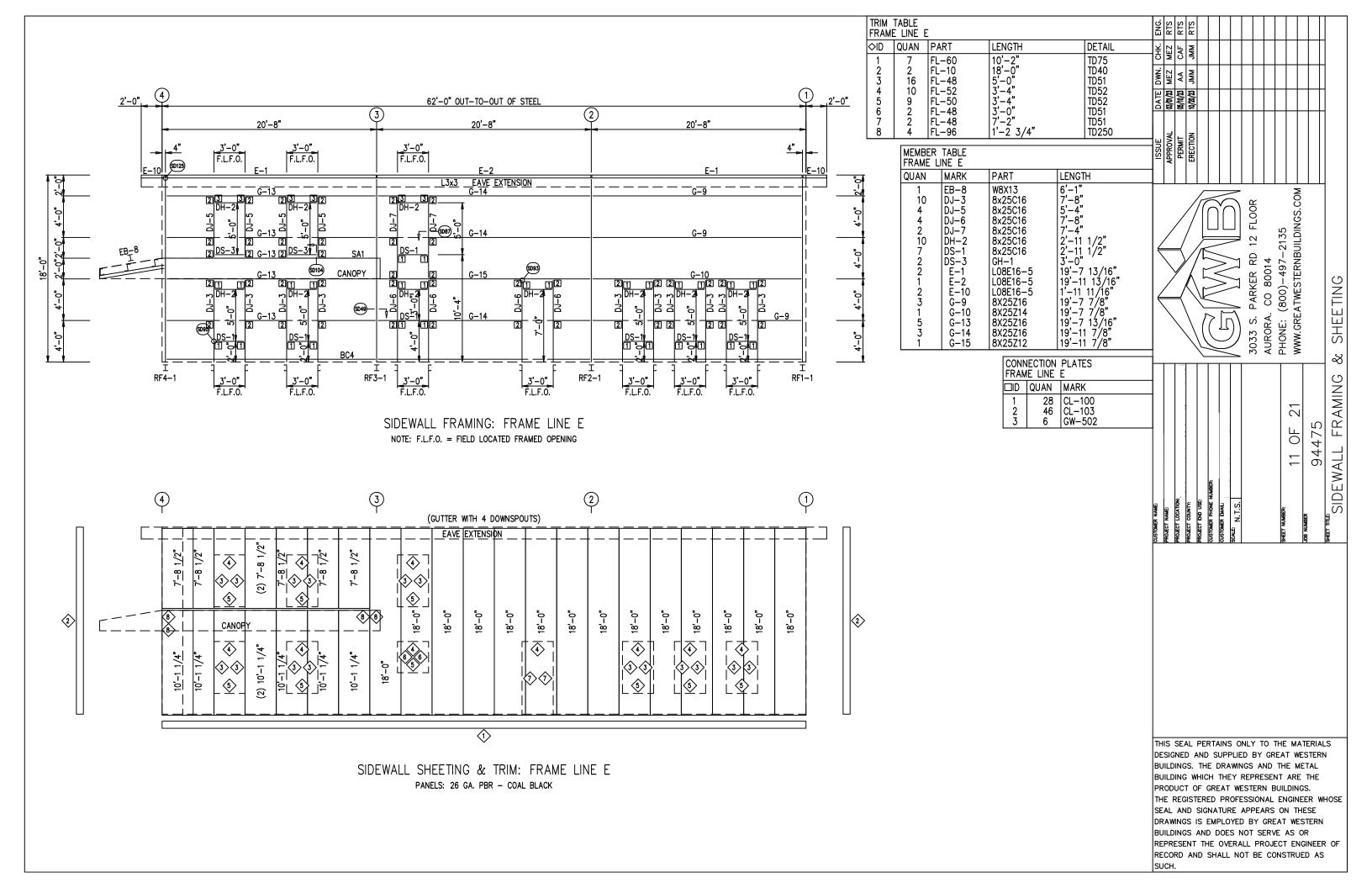


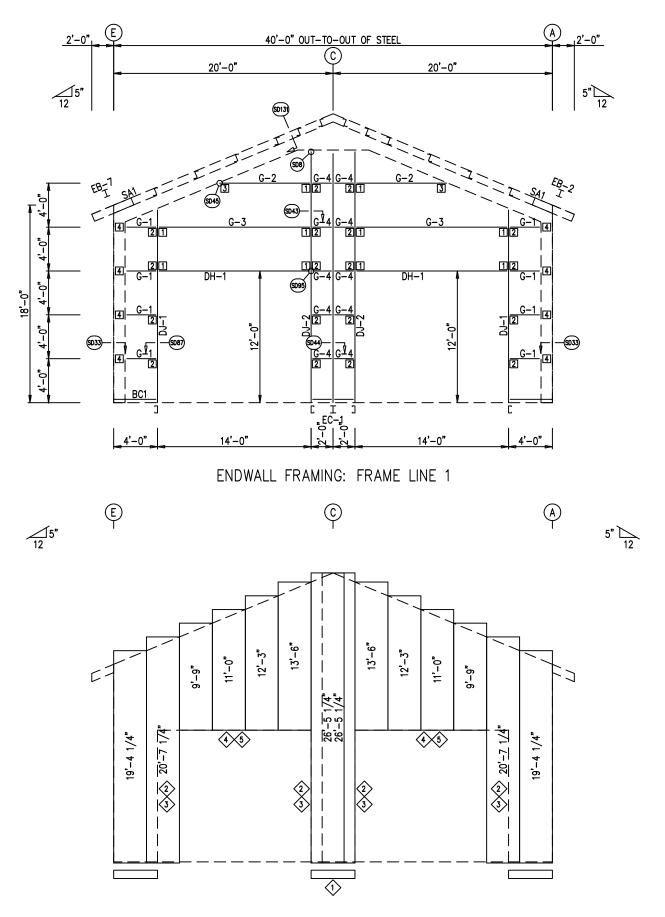




SPLICE BOLT TABLE CAP PLATE BOLTS	MEMBER T	ABLE					ပါတုတြကု
Qty Mark Top Bot Int Type Dia Length Mark Qty Type Dia Length	Mark	Web Depth Start/End	Web Plo Thick 0.188	Lenath	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length 6 x 1/4" x 16"-3 7/8"	
SP-1 4 2 2 A325 5/8" 2" EC-2 4 A325 5/8" 1 1/2"	RF4-1	13.0/13.0	0.188	17'–7 15/16"	6 x 1/4" x 8'-4 1/2" 6 x 1/4" x 7'-8 7/16"	6 x 1/4" x 16'-3 7/8"	NAE:
SP-2 4 4 2 A325 5/8" 1 3/4" EC-3 4 A325 5/8" 1 1/2"	RF4-2	15.0/15.0	0.135	18'-10 3/8"	Outside Flange W x Thk x Length 6 x 1/4" x 8'-4 1/2" 6 x 1/4" x 7'-8 7/16" 6 x 1/4" x 1'-2 1/4" 6 x 1/4" x 20'-4 1/16"	6 x 1/4" x 20'-4 1/16"	MEZ AA AA AAA AAAAAAAAAAAAAAAAAAAAAAAAAA
FLANGE BRACES: FBxx (1 or 2)	RF4-3	15.0/15.0 15.0/15.0 13.0/13.0	0.135 0.135 0.188	18'-10 3/8" 2'-0" 17'-7 15/16"		6 x 1/4" x 16'-3 7/8"	DATE (0,0/0/2) (0,0/0/2)
FLANGE BRACES: FBxx (1 or 2) xx=length(in) (1) One Side; (2) Two Sides					6 x 1/4" x 7'-8 7/16" 6 x 1/4" x 8'-4 1/2"		
A - 2X2X14Ga	EC-2 EC-3	7.5/ 7.5 7.5/ 7.5	0.135 0.135	21'-2 7/16" 21'-2 7/16"	6 x 1/4" x 1'-2 1/4" 6 x 1/4" x 7'-8 7/16" 6 x 1/4" x 8'-4 1/2" 5 x 1/4" x 21'-3 9/16" 5 x 1/4" x 21'-3 9/16"	5 x 1/4" x 21'-3 9/16" 5 x 1/4" x 21'-3 9/16"	ISSUE ISSUE APPROVAL PERMIT ERECTION
					BFAM	TABLE Length	
	FL-56 (6)				Mark EB-4 EB-6 EB-9	I W8X10	
					EB-9	W8X10 3'-3 1/16" W8X13 6'-0 7/8"	Si COM
	1-0"	1'-0"					3033 S. PARKER RD 12 FLOOR AURORA. CO 80014 PHONE: (800)–497–2135 www.GREATWESTERNBUILDINGS.C
(02)				≈ 2 <i>₹</i> ′			RNB
33-9-12-12-12	202-2	202'-2"		23'-9 1/2" (22)			KER SOO SOO STEELS STEELS STEELS SOO SOO STEELS STE
3 1/2" 26 GA. PBR. CHARCOAL GRAY 21'-8"	70 3A(1) 1832 34	*		21'-8"			P P A R S S S S S S S S S S S S S S S S S S
3 1/2" CHARCOAL GO.	(1) FB32 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)) FB32.34(1)	. ·	4.0	31/2"		
31/2" CHARCO" 4 4 4-0" 16(1)	FB3		FB30	4 @ 4'-0"			N WW. N
7 - 2 - 4 - FB 32-35 -	2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2		(1)		12'-2"		303. 303. AUR PHO PHO PHO PHO PHO
	W (0)1		7	FD			
12 5"\ FB32.3A(1)		†		7 - F837	2.34(1) / 5" <u>12</u>		
12			7	PF4-2	, / 12		
9				1		- 0 - I	9 OF 94475 FRAME
				7	-	- 24	
\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\					6'-2 1/2" (8)	*O-	
3" 4	1,8					<u>,4</u>	WE: RIT: USE: NTY: USE: NAMEER: S.S. RI:
12" 2 @ 2-0		16" +/-			2 @ 2'-8" 2"	. "0	ER NAME COUNTY N. T.I.S N. T.I.S N. T.I.S
	21'-3 3/16" CLEAR +/-	21'-3 3/16" CLEAR +/- EC-2			1 12	25	USTOME PROJECT USTOME U
18'-0" 18'-0" 14'+ 14'+ 14'+	[] 21, C[21, CL		1/4	OLEAR +1	3, -0,	
16 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				16,-4	RF4 B-9		
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1'-1 1/2" 11'-10 1/2" CLEAR +/-				11'-10 1/2" 1'-	1 1/2"		
CLEAR +/- 2'-0" 13'-0"	14'-0"			11'-10 1/2" 1'- CLEAR +/- 13'-0"	2'-0"		
	D 40'-0" OUT-TO-OUT OF	STEF! B)	· - •	6'-0"		
	+0 -0 001-10-001 OF	JILLL					THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN
(E)	IGID FRAME ELEVATION: 1	RAMF LINF	4		(A)		BUILDINGS. THE DRAWINGS AND THE METAL
	TOID TRANSLE ELEVATION. I	TATALL LINE	1				BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS.
							THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE
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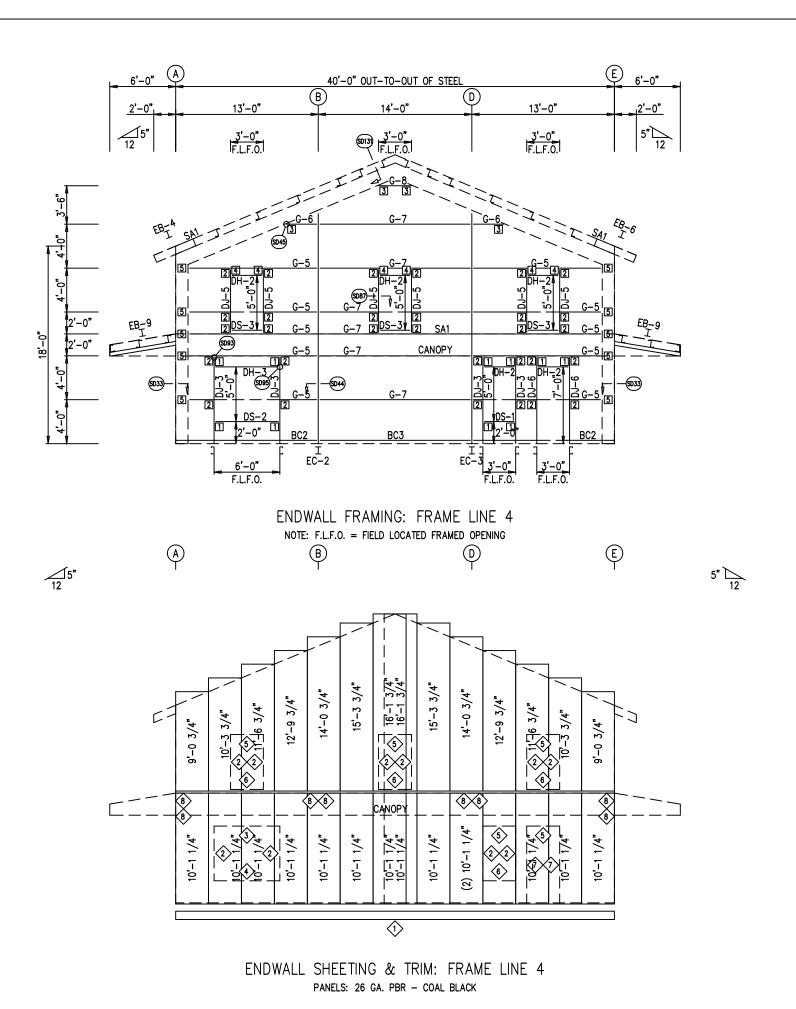






ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - COAL BLACK

RIM	TABLE				ENG.	S	ς	ပ္					Τ		П	П	
RAM >ID	E LINE 1 QUAN	PART	LENGTH	DETAIL		₩	⊢			-	+		+	Н	+	+	
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1 2 3 4 5	4	FL-55 FL-48	12'-2" 12'-2" 14'-7"	TD51 TD51	DWN.		¥										
4 5	2 2	FL-55 FL-48 FL-55 FL-52	14'-7"	TD52 TD52	DATE	33/01/23	05/10/23	10/20/23									
	BOLT TAI																
-	FRAME L LOCATION		QUAN TYPE	DIA LENGTH	ISSUE	APPROVAL	PERMIT	ERECTION									
(COLUMNS JAMBS/R	RAFTER AFTER	4 A325 2 A325	5/8" 1 1/2" 5/8" 1 1/2"]ÿ	APP	<u> </u>	ERE									
	МЕМВ	ER TABLE			┞				\bigsqcup						<u>_</u> 		
	FRAMI QUAN	E LINE 1 MARK	PART L	ENGTH	$\frac{1}{1}$		/	/	///	$\tilde{\gamma}$	\bigcap		FLOOR		PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM		
	1	EB-2 EB-7	W8X10 3 W8X10 3	'-2" '-2" 3'-0 3/16" 7'-7 11/16" 2'-11 1/2"		/	/	/			L				35 DING		
	1	EC-1	W8X13 2	3'-0 3/16" 7'-7 11/16"	{	_	\dashv	/		\leq	-		RD 12		-21. BUIL		
	2	DJ-2 DH-1	8x25C12	2'-11 1/2" 3'-11 1/2"			$/\!\!/$		۲	\geq	_			CO 80014	(800)-497-2135 ATWESTERNBUILDIN		
	8 2	G-1 G-2	8X25Z16 2 8X25Z16 8	'-7 1/8" '-2 3/8"	∠	_	_/	$\langle \ \rangle$	Œ	2		٦	PARKER	ω Ο	00)- WES:		
	2 2 8 2 2 10	G-3 G-4	8X25Z16 1 8X25Z16 1	3'-11 1/2" '-7 1/8" '-2 3/8" 3'-11 1/2" '-3 3/4"	`	/	\ \	\	\ [)(۸.	_	∀.	: (8 ?EAT		
			CONNECTION F		1		\	/)	(1			3033	AURORA.	PHONE: WWW.GRE		SHEETING
			FRAME LINE 1 □ID QUAN I	MARK	L		_	_		<u>\</u>	\leq	_	کر ص	₹ —	╁┋		8
			1 10	CL-100 CL-103	1												
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					CUSTOMER NAME	PROJECT NAME:	PROJECT LOCAT	PROJECT COUNT	ROJECT END USE:	CUSTOMER PHO	D NEW CO	z			sheet number	JOB NUMBER	SHEET TILE:
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TD75 TD51 TD52 TD52 TD52 TD52 TD51 TD250 FL-48 10 2 3 4 5 6 7 FL-52 FL-50 FL-52 FL-50 FL-48 DATE 03/01/23 05/10/23 10/20/23 5 2 8 BOLT TABLE FRAME LINE 4 LOCATION QUAN TYPE DIA LENGTH COLUMNS/RAFTER A325 5/8" 1 1/2" PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM MEMBER TABLE FRAME LINE 4 QUAN MARK LENGTH PART 3'-3 1/16" 3'-3 1/16" 6'-0 7/8" 21'-4 5/16" 7'-8" 5'-4" 7'-8" EB-4 W8X10 W8X10 EB-6 EB-9 EC-2 EC-3 DJ-5 DJ-6 DH-2 DH-3 DS-1 DS-1 G-6 G-7 G-8 W8X13 W08541 W08541 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 8x25C16 8x25Z16 8X25Z16 8X25Z16 8X25Z16 SHEETING AURORA. | /-8" 2'-11 1/2" 5'-11 1/2" 2'-11 1/2" 5'-11 1/2" 11'-6 3/16" 2'-5 5/16" 13'-3 7/8" 2'-9 3/16" Ø 2 FRAMING 2 CONNECTION PLATES FRAME LINE 4 9F 944 □ID QUAN MARK 10 CL-100 30 CL-103 4 CL-109F 6 GW-502 10 4" ZEE - 1'-0" 13 ENDWALL 2 3 4 5 THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

TRIM TABLE FRAME LINE 4

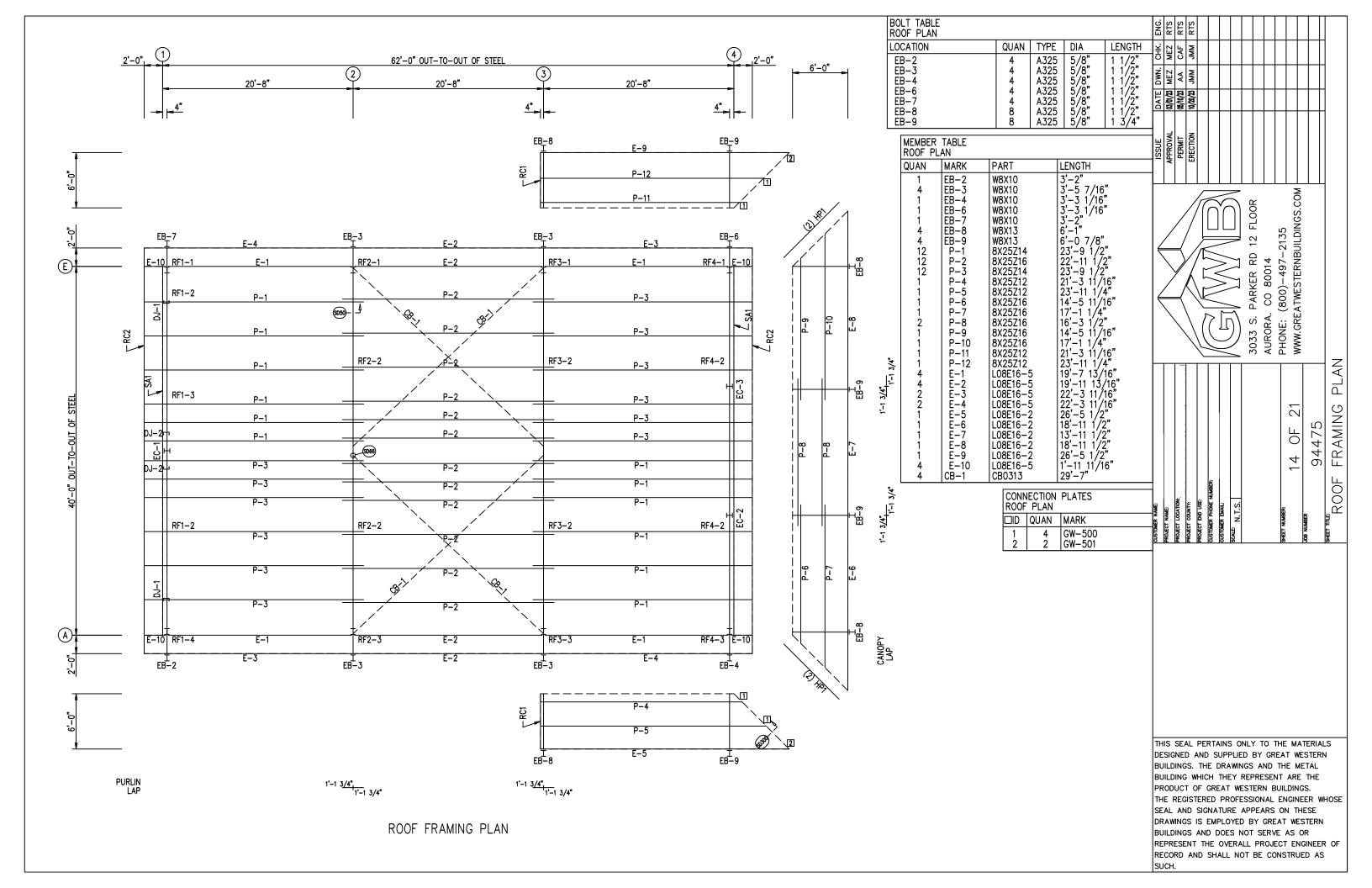
QUAN PART

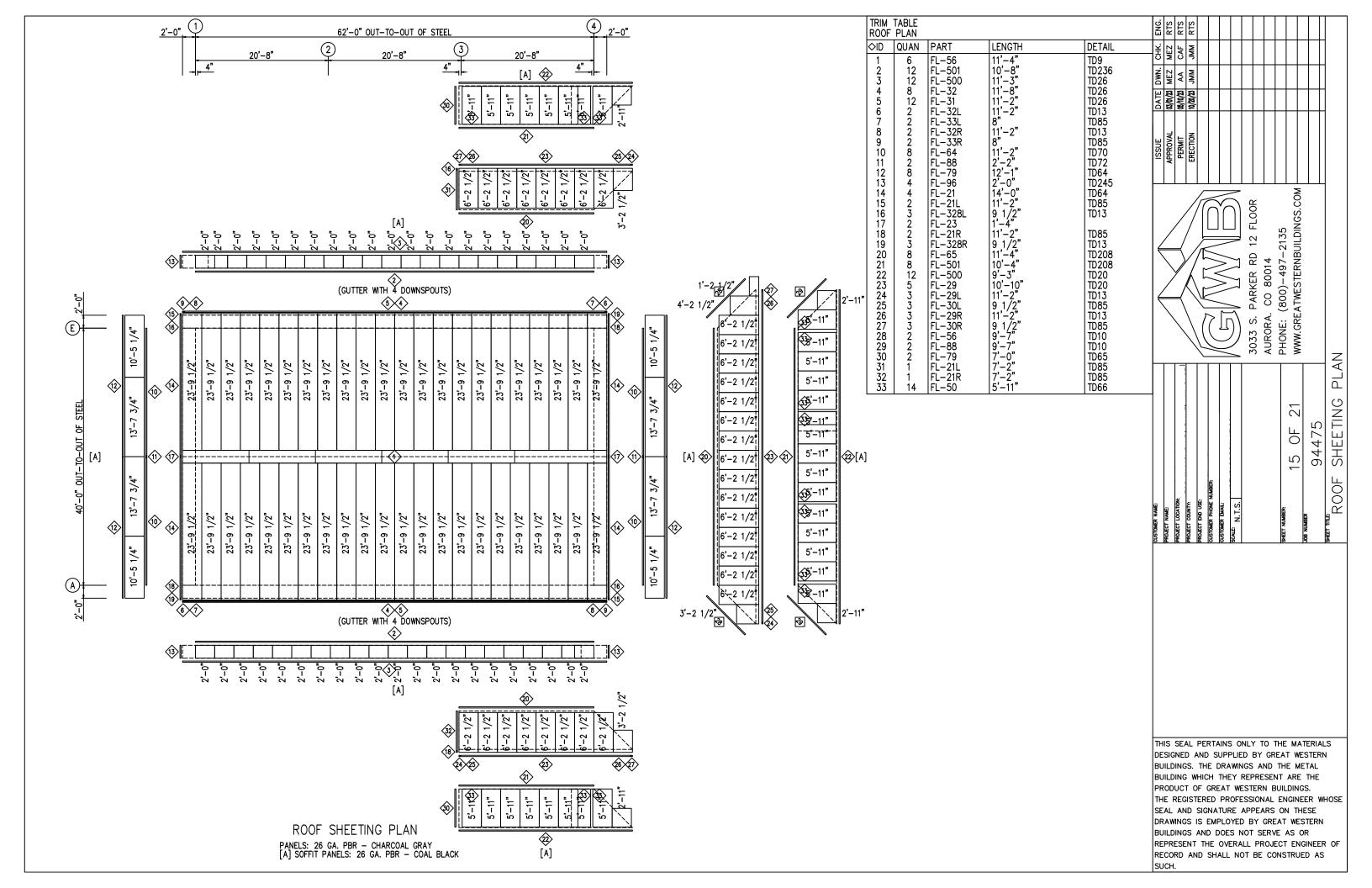
FL-60

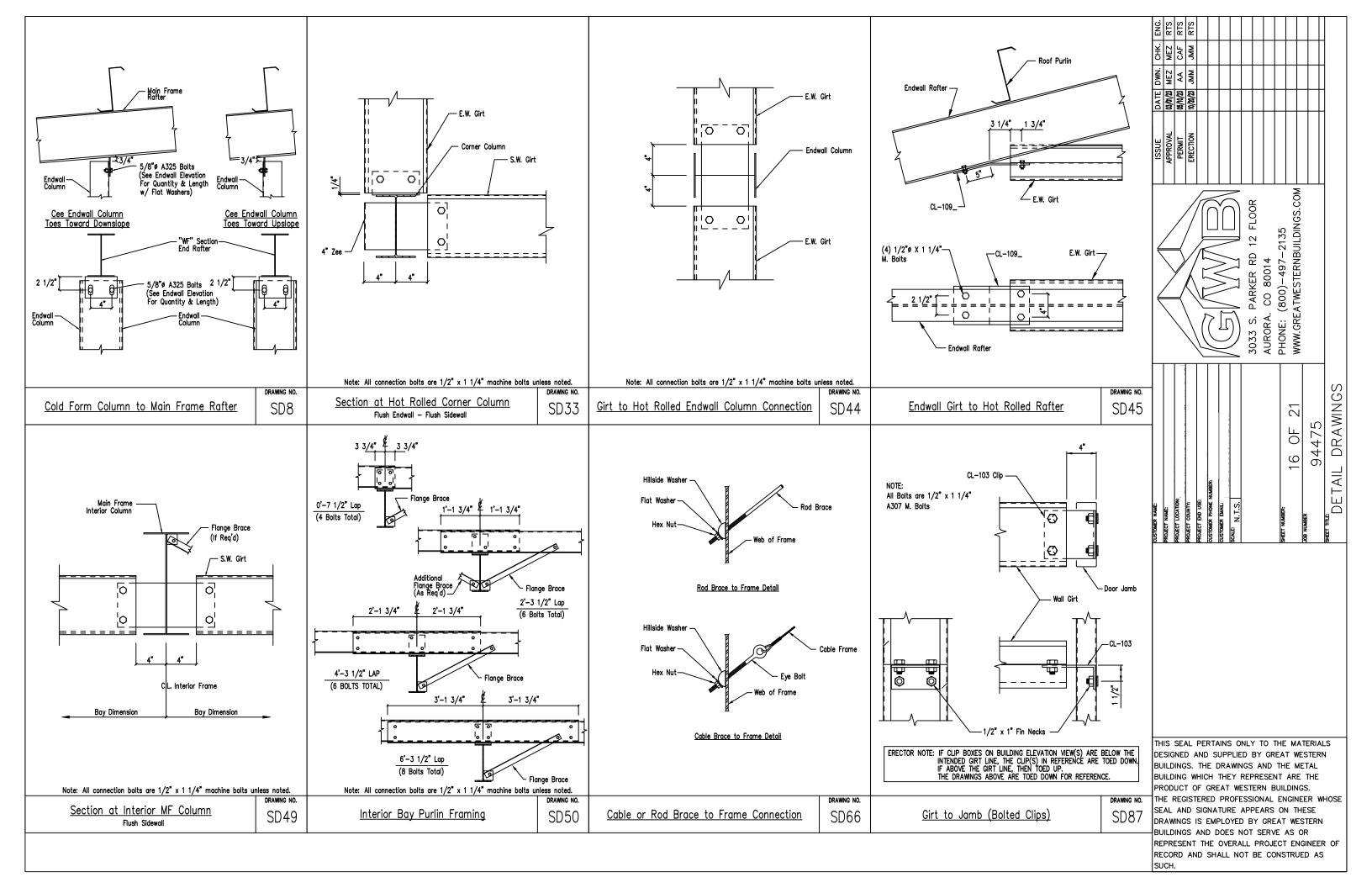
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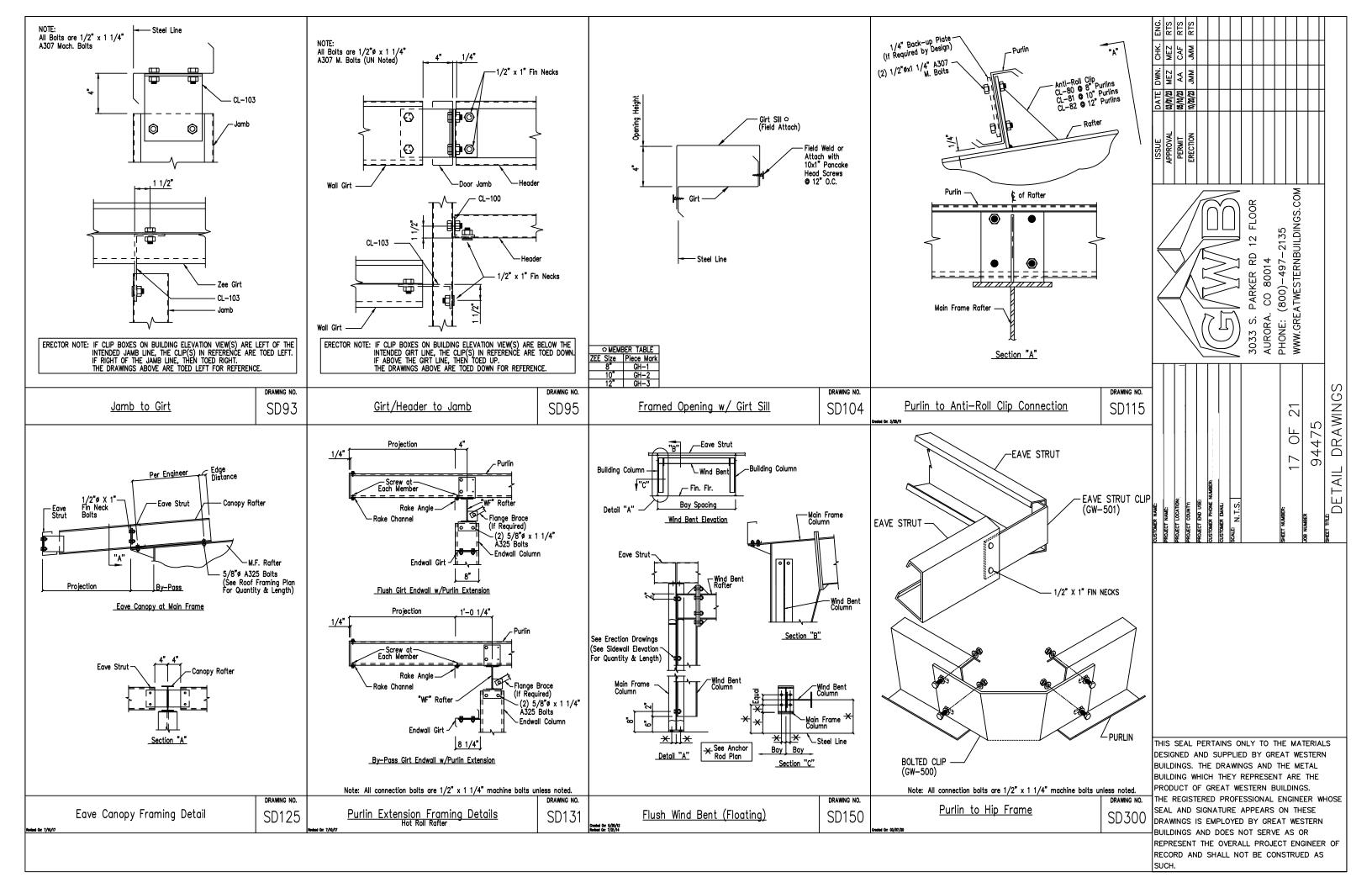
DETAIL

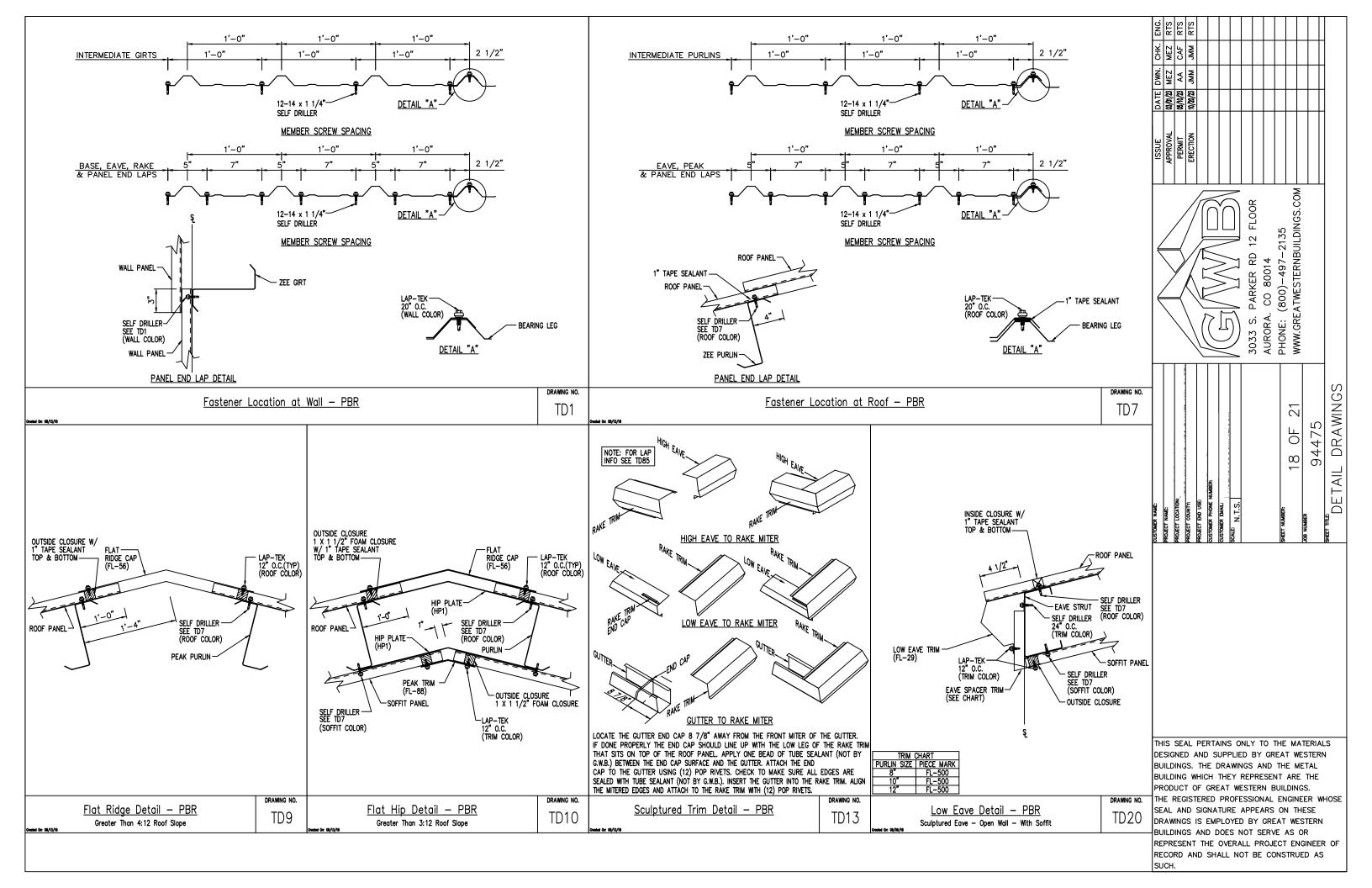
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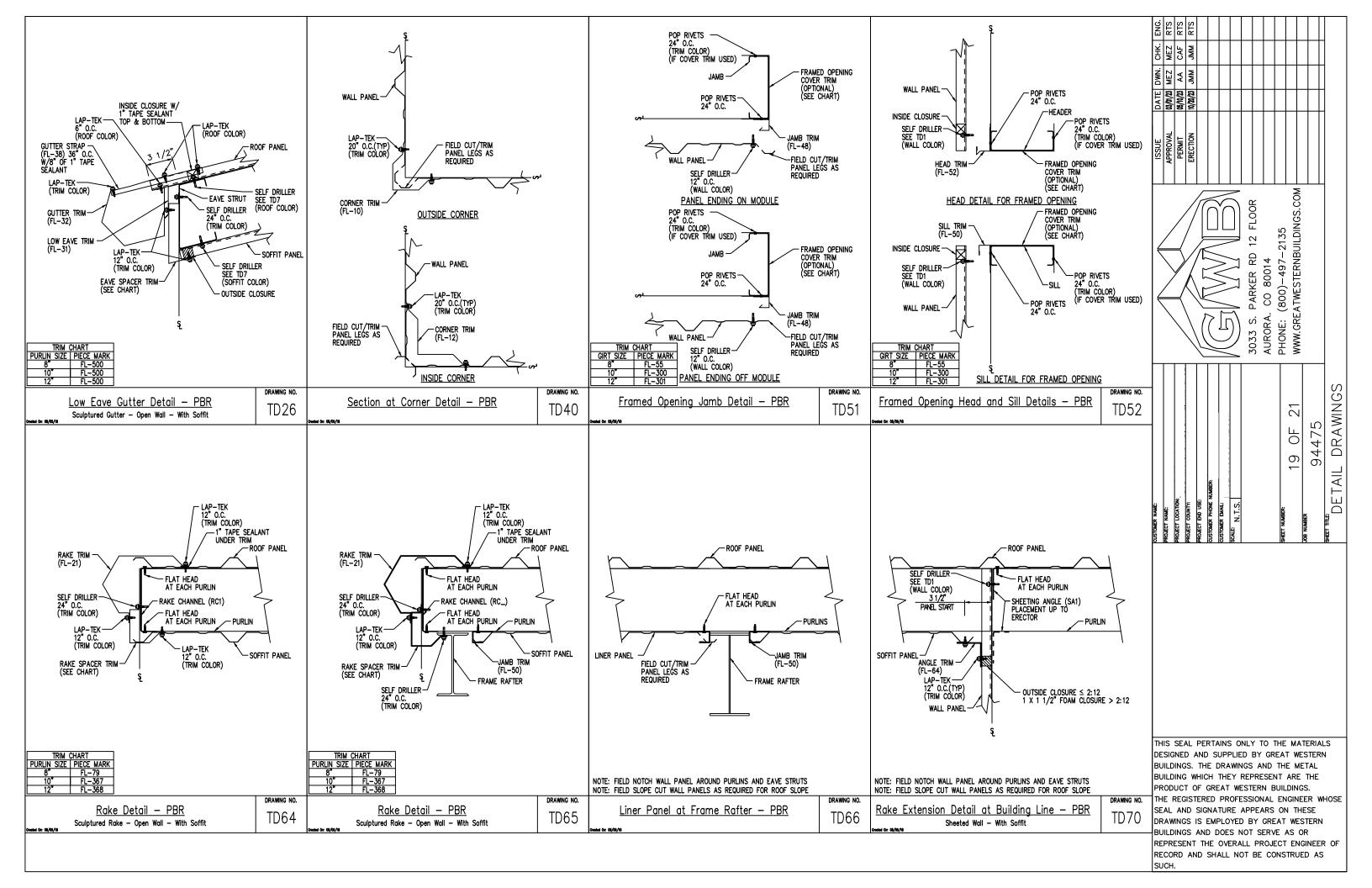


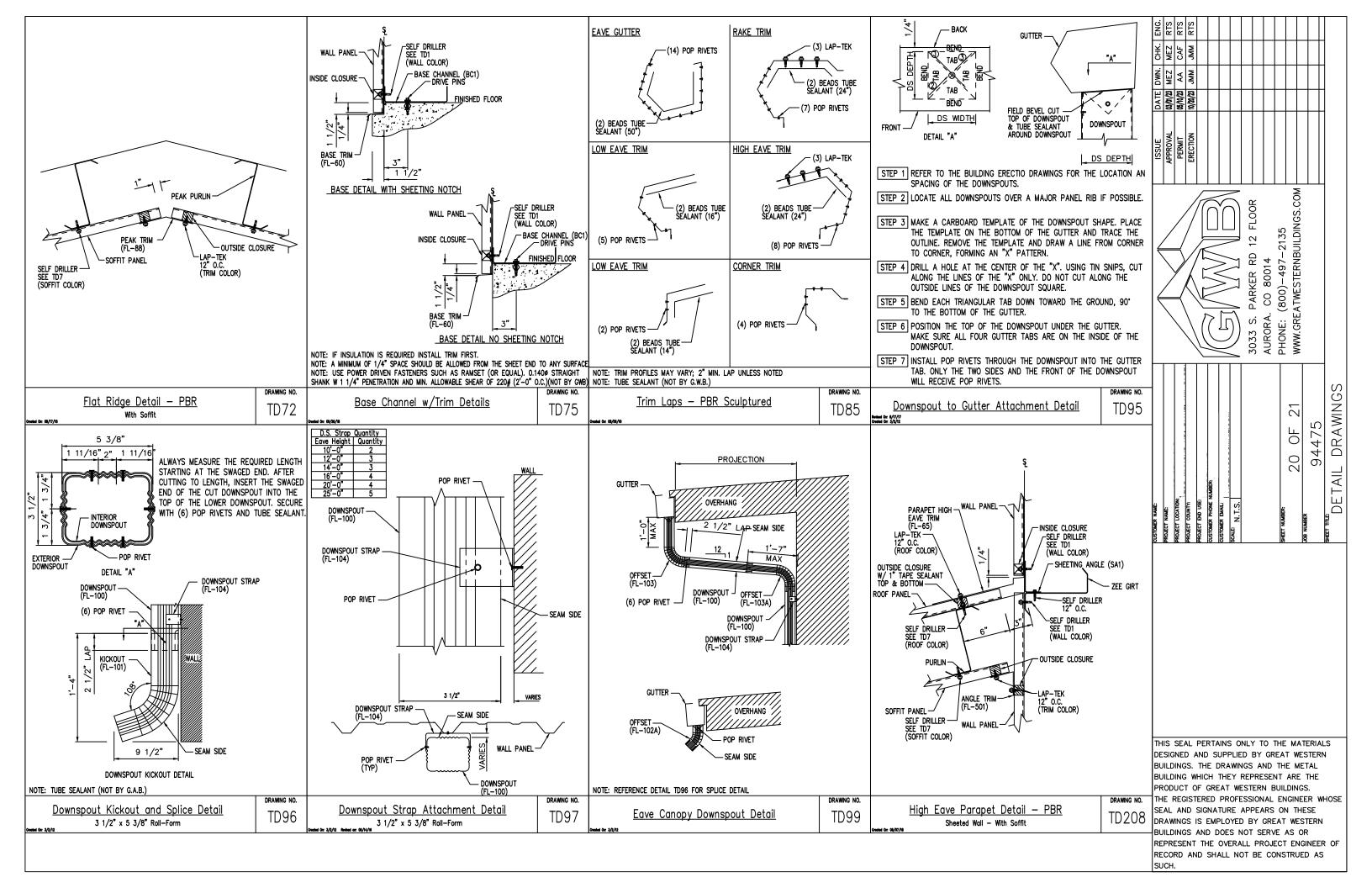


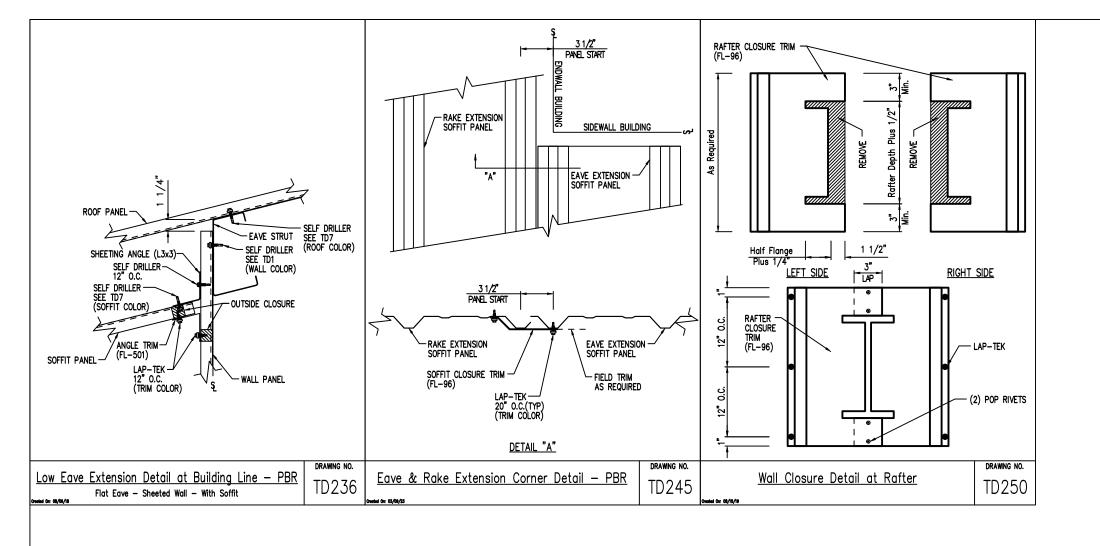












<u> </u>	CUSTOMER NAME:	←	ISSUE	DATE DWN. CHK. ENG.	N		ENG.
ΙŒ	PROJECT NAME:	/ /	APPROVAL	03/01/23 MEZ MEZ RTS	MEZ	MEZ	RTS
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i <u>c</u>	PROJECT COUNTY:		ERECTION	10/20/23 JMM		MM	RTS
ΙŒ	PROJECT END USE:						
IQ	CUSTOMER PHONE NUMBER:						
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		JUSS S. PARNER ND 12 FLUUR					
		AURORA. CO 80014					
ᄨ	SHEET NUMBER:	PHONE: (800)-497-2135					
	21 OF 21	WWW CREATWESTERNBILL DINGS COM					
13	JOB NUMBER	WWW.GITEN WESTEIN BOILDINGS.COM					
	94475						
i							
<u> </u>	DETAIL DRAWINGS						

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