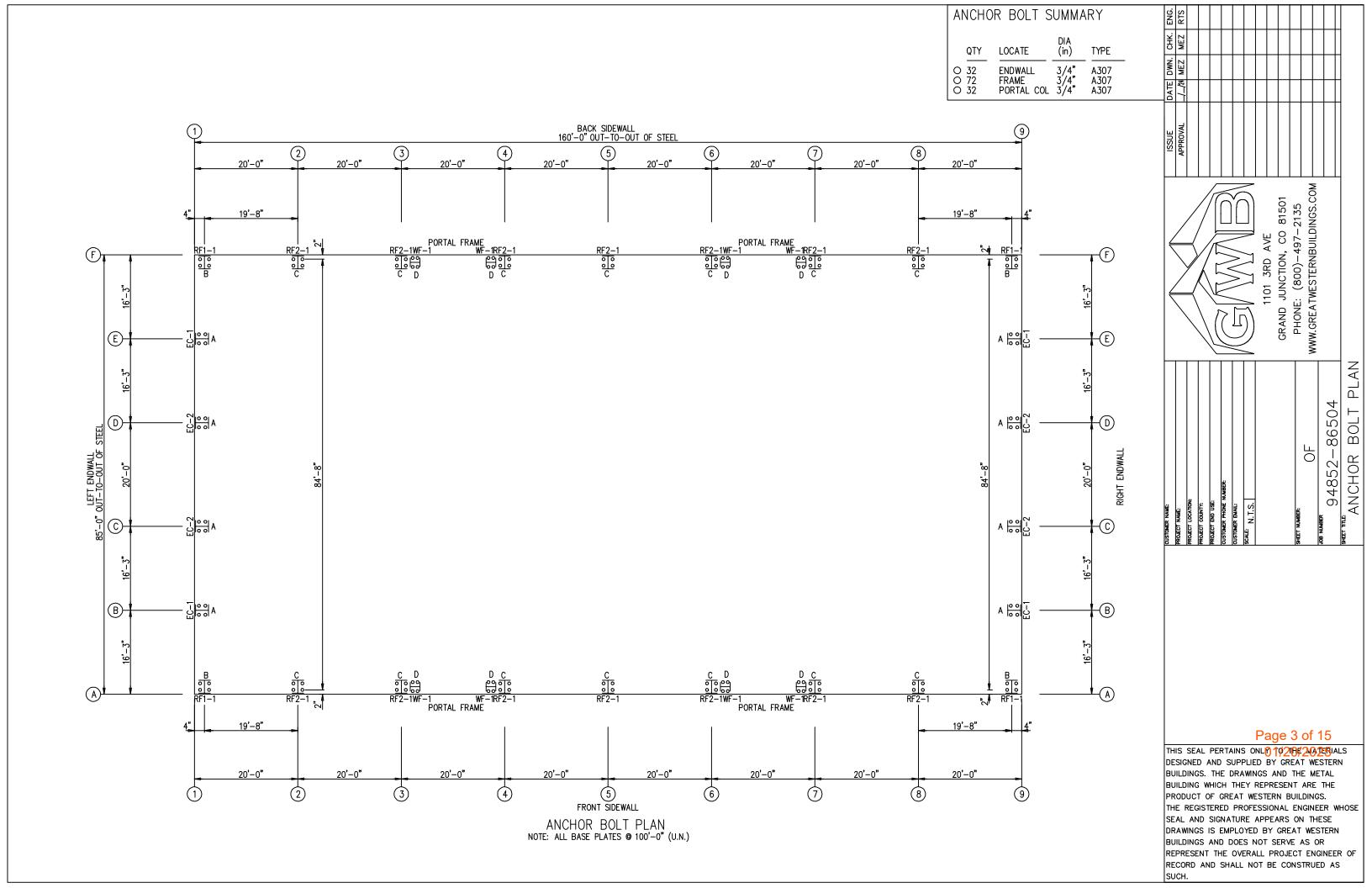
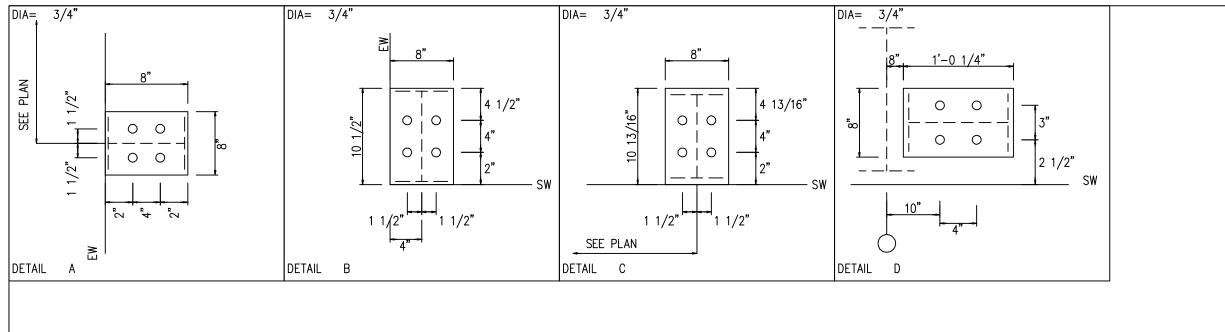


## BUILDING LOADS **GENERAL NOTES** 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 DESIGN LOADS: FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, DESIGN CODE / WIND CODE : IBC-21 MIN. YIELD STRENGTH ASTM DESIGNATION HOT ROLLED STEEL SHAPES (W, & C) HOT ROLLED STEEL ANGLES (L) STEEL PIPES OCCUPANCY / RISK CATEGORY : II - Normal Fy = 36 KSI Fy = 42 KSI **ENCLOSURE** 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COURSENTS ELBUSISHED BY ARE SEALED. : Open STRUCTURAL TUBING A500 Fy = 42 KSI ROOF DEAD LOAD (D) (PSF) : 2.00 STRUCTURAL STEEL WEB PLATE ROOF COLLATERAL LOAD (C) (PSF) :1.00 STRUCTURAL STEEL FLANGE PLATES/BARS A529/A572 A653/A1011 Fv = 55 KSIWIND LOAD Fv = 55 KSLROOF & WALL SHEETS CABLE BRACE ROD BRACE A792/A653 A475 - TYPE 1 Fy = 50, 80 KSI EXTRA HIGH STRENGTH ULTIMATE WIND SPEED, (VULT) (MPH) :130.00 STRUCTURAL COMPONENTS FURNISHED BY M.B.S. Fy = 36 KSIWIND EXPOSURE CATEGORY : C 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS MICLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD MIN. TENSILE STRENGTH INTERNAL PRESSURE COEFFICIENT, (GCpi) : 0.00/0.00 MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1"Ø & LESS) HIGH STRENGTH BOLTS (1"Ø TO 1 1/2"Ø) ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554 Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI WALL PANEL DESIGN WIND PRESSURE (PSF) : 28.22/-31.04 WIND ENCLOSURE CLASSIFICATION : Open Fu = 60 KSILIVE LOAD PRIMARY FRAMING (PSF) : 20.00 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE WWW.GREATWESTERNBUILDINGS. PRIMER SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF TRIB. AREA REDUCTION :No $\infty$ 7 SECONDARY FRAMING (PSF) : 20.00 8 110, .... JUNCTION, CO CONCRETE SNOW LOAD AVE CONCRETE. UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL) IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION). GROUND SNOW LOAD, (Pg) (PSF) : 20.0 ROOF SNOW LOAD, (Pf) (PSF) :20.0 3RD NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT SNOW EXPOSURE FACTOR, (Ce) :1.00 SNOW IMPORTANCE FACTOR, (Is) :1.00 1101 .4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS THERMAL FACTOR, (Ct) :1.20 PHONE: SEISMIC LOAD 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHICEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION) SEISMIC IMPORTANCE FACTOR, (le) :1.00 1.5 ALL BOLTS ARE 1/2" x 0"-1 1/4" A307 EXCEPT: A) ENDWALL RAFTER SPLICE - 5/8" x 0"-1 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) RSHEE SITE CLASSIFICATION : d SPECTRAL RESPONSE ACCELERATION : Ss = 1.146:S1 = 0.443C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANGE BRACE CONNECTIONS - 1/2" ø x 0'-1 1/4" A325 SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.917:Sd1 = 0.548NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING SEISMIC DESIGN CATEGORY ٠Π 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION) BASIC SEISMIC FORCE RESISTING SYSTEM :STEEL SYSTEM NOT SPECIFICALLY 6 A325 BOLT TIGHTENING REQUIREMENTS OVE ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION. STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS. DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) $\circ$ TOTAL DESIGN BASE SHEAR, (V) (KIPS) :LONGITUDINAL = 20.91 2.12 <u>WARNING</u> IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EYEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE $\circ$ 50 : TRANSVERSE = 22.43ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS INF WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING 86. RESPONSE MODIFICATION FACTORS, (R) :RIGID FRAMES = 3.25 $\Omega = 3.00$ 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKES SAFETY, MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE. ARE RECOMMENDED. :SW WIND BENT = 3.25 $\Omega = 3.00$ DING. $\circ$ CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION: INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.2822BETWEEN WALL PANELS & EAVE/GABLE TRIM $\infty$ :SW WIND BENT = 0.2822948 ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE, IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO WHERE APPLICABLE, ARE RECOMMENDED. $\overline{\mathbf{m}}$ ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION PROCURE AND INSTALL AS NEEDED TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME. OTHER LOADS/REQUIREMENTS 2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ERECTION AND UNLOADING NOT BY G.W.B. ICE BUILDUP, SEE TABLE BELOW. **BUILDING DESCRIPTION:** ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE ROOF SNOW LOAD EQUIVALENT SNOW HEIGHT AT ROOF RECOMMENDED SNOW HEIGHT WIDTH (FT) : 85.0 WHEN SNOW REMOVAL SHOULD START (IN INCHES) (IN INCHES) LENGTH (FT) : 160 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10) CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL EAVE HEIGHT AT BSW (FT): 22.0 HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND TH REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND EAVE HEIGHT AT FSW (FT): 22.0 ROOF SLOPE AT BSW : 2.0:12 ROOF SLOPE AT FSW : 2.0:12 ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED BAY SPACING (FT) :8 AT 20 MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S. **COVERING AND TRIMS: ROOF PANELS & TRIMS** BUYER/END USE CUSTOMER RESPONSIBILITIES PANEL TYPE : 26 GA. PBR IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING 2.1 PANEL COLOR : GALVALUME FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 TRIM COLORS EDITION, SECTION A8.4, PAGE XI-A8-2 GABLE/EAVE : POLAR WHITE METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.) STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE, METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS EAVE GUTTER : NEED STD. COLOR : POLAR WHITE DOWNSPOUTS MEINOUS AND IDERNATES STALL GOVERN THE WORK WITH ANT OTHER INTERFETAINT TO THE CONTRARY NOTWITHSTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS. Page 2 of 15 WALL PANELS & TRIMS (TO REMAIN OPEN) THIS SEAL PERTAINS ONLY 10 THE MATERIALS IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR 2.3 OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD DESIGNED AND SUPPLIED BY GREAT WESTERN INSULATION BUILDINGS. THE DRAWINGS AND THE METAL ROOF INSULATION :N/A APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3) BUILDING WHICH THEY REPRESENT ARE THE WALL INSULATION :N/A PRODUCT OF GREAT WESTERN BUILDINGS.

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

DESIGNED AND SUPPLIED BY GREAT WESTERN
BUILDINGS. THE DRAWINGS AND THE METAL
BUILDING WHICH THEY REPRESENT ARE THE
PRODUCT OF GREAT WESTERN BUILDINGS.
THE REGISTERED PROFESSIONAL ENGINEER WHOSE
SEAL AND SIGNATURE APPEARS ON THESE
DRAWINGS IS EMPLOYED BY GREAT WESTERN
BUILDINGS AND DOES NOT SERVE AS OR
REPRESENT THE OVERALL PROJECT ENGINEER OF
RECORD AND SHALL NOT BE CONSTRUED AS
SUCH.





## NOTE:

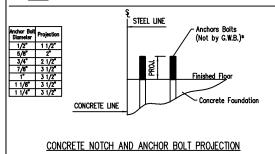
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.

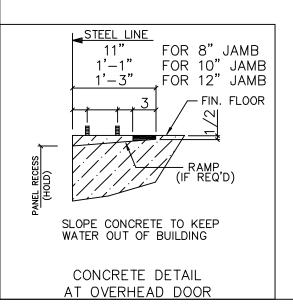
G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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DETAILS

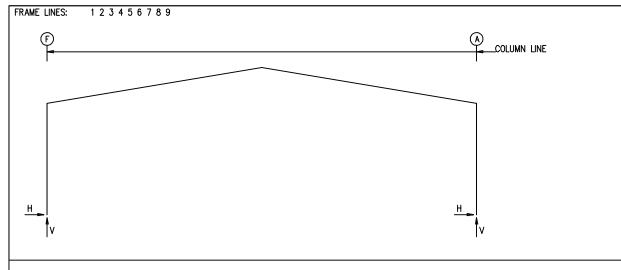
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THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.



RI	RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES													
	Frm Line	Col Line	Load Id	Hmax H	umn_React V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
	1*	F	1	7.5	11.6	2 3	-5.6 -4.6	-6.2 -8.5	4	0.750	8.000	10.50	0.375	0.0
	1*	Α	3 1	5.6 -7.5	-6.2 11.6	1 2	-7.5 4.6	11.6 -8.5	4	0.750	8.000	10.50	0.375	0.0
	1*	FRAME lir	nes:	1 9										

RIGID	FRAME:		MAXIMUM	REACTION	S, ANC	HOR BOLT	S, & BAS	E PLATE	ES					
Frm Line	Col Line	Load Id	Hmax H	umn_React V Vmax	ions(k Load Id	Hmin H	V Vmin	Bol <sup>1</sup> QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)	
2*	F	1	14.4	22.1	2 3	-11.2 -9.2	-12.6 -17.3	4	0.750	8.000	10.81	0.625	0.0	
2*	Α	3 1	11.2 -14.4	-12.6 22.1	1 2	-14.4 9.2	22.1 -17.3	4	0.750	8.000	10.81	0.625	0.0	
2*	FRAME li	nes:	2 3 4	5 6 7 8										

END	WALL	COLUN	ΛN:	BASIC CO	DLUMN REAC	TIONS (k	)
Frm Line 1 1 1 1 9 9	Col Line E D C B B C D	Dead Vert 0.5 0.6 0.5 0.5 0.5 0.5	Wind Press Horz -4.0 -5.1 -5.1 -4.0 -4.0 -5.1	Wind Suct Horz 4.5 5.8 5.8 4.5 5.8 5.8	Seis Long Horz 0.1 0.2 0.2 0.1 0.1 0.2 0.2		•
9	E	0.5	-4.0	4.5	0.1		
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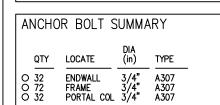
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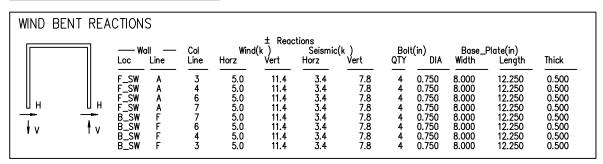
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Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt QTY	t(in) DIA	Base Width	e_Plate(in) Length -	Thick	Grout (in)
1	Е	4 6	2.7 0.0	0.3 0.6	5	-2.4	0.3	4	0.750	8.000	8.000	0.375	0.0
1	D	4 6	3.5 0.0	0.4 0.7	5	-3.1	0.4	4	0.750	8.000	8.000	0.375	0.0
1	С	4 6	3.5 0.0	0.4 0.7	5	-3.1	0.4	4	0.750	8.000	8.000	0.375	0.0
1	В	4 6	2.7 0.0	0.3 0.6	5	-2.4	0.3	4	0.750	8.000	8.000	0.375	0.0
9	В	4 6	2.7 0.0	0.3 0.6	5	-2.4	0.3	4	0.750	8.000	8.000	0.375	0.0
9	С	4 6	3.5 0.0	0.4 0.7	5	-3.1	0.4	4	0.750	8.000	8.000	0.375	0.0
9	D	4 6	3.5 0.0	0.4 0.7	5	-3.1	0.4	4	0.750	8.000	8.000	0.375	0.0
9	E	4 6	2.7 0.0	0.3 0.6	5	-2.4	0.3	4	0.750	8.000	8.000	0.375	0.0

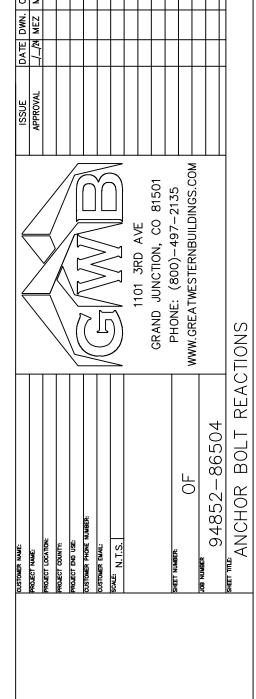
RIGID	RIGID FRAME: BASIC COLUMN REA						)								
FRAME Line 1* 1*	Column Line F A	Horz 1.1 -1.1	-Dead Vert 2.1 2.1	-—-Collo Horz 0.3 -0.3	teral— Vert 0.4 0.4	Horz 5.9 -5.9	-Live Vert 8.6 8.6	Horz 5.9 -5.9	-Snow Vert 8.6 8.6	Wind Horz -9.1 10.1	Left1- Vert -15.5 -13.2	−Wind_ Horz −10.1 9.1	Right1- Vert -13.2 -15.5		
FRAME Line 1* 1*	Column Line F A	Wind Horz -10.4 8.8	_Left2- Vert -12.4 -16.3	-Wind_ Horz -8.8 10.4	Right2- Vert -16.3 -12.4	Wind Horz -9.2 9.2	l_Long1- Vert -13.8 -13.8	Wind Horz 3.9 -3.9	I_Long2- Vert 5.8 5.8	-Seism Horz -1.3 -1.3	ic_Left Vert -0.6 0.6	Seismic Horz 1.3 1.3	:_Right Vert 0.6 -0.6		
FRAME Line 1* 1*	Column Line F A	-MIN_S Horz 1.5 -1.5	NOW Vert 2.1 2.1	F1UNB_ Horz 4.3 -4.3	SL_L- Vert 7.6 4.5	F1UNB_ Horz 4.3 -4.3	SL_R- Vert 4.5 7.6								
FRAME Line 2* 2*	Column Line F A	Horz 1.8 -1.8	Dead Vert 3.4 3.4	-—-Collo Horz 0.6 -0.6	teral— Vert 0.9 0.9	 Horz 11.5 -11.5	-Live Vert 17.0 17.0	Horz 11.5 -11.5	-Snow Vert 17.0 17.0	Wind Horz -17.8 19.7	_Left1- Vert -30.5 -26.0	-Wind_ Horz -19.7 17.8	Right1- Vert -26.0 -30.5		
FRAME Line 2* 2*	Column Line F A	Wind Horz -20.4 17.2	_Left2- Vert -24.4 -32.1	-Wind_ Horz -17.2 20.4	Right2- Vert -32.1 -24.4	Wind Horz -17.0 17.0	l_Long1- Vert -25.6 -25.6	Wind Horz 7.6 -7.6	l_Long2- Vert 11.5 11.5	-Seism Horz -1.7 -1.7	ic_Left Vert -0.8 0.8	Seismic Horz 1.7 1.7	:_Right Vert 0.8 -0.8		
FRAME Line 2* 2*	Column Line F A	-MIN_S Horz 2.9 -2.9	NOW Vert 4.2 4.2	F2UNB_ Horz 8.3 -8.3	SL_L- Vert 14.9 8.8	F2UNB <sub>-</sub> Horz 8.3 –8.3	_SL_R- Vert 8.8 14.9								
	FRAME lir FRAME lir		1 9 2 3 4	5 6 7	8										

OTES	FOR REACTIONS		
	ding reactions are based of following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Coeff (Fa*Ss)	= = = = = = = = = = = = = = = = = = =	Open 1.00 1.00
ID	Description		
1 2 3 4 5 6	Dead+Collateral+0.75Sno 0.6Dead+0.6Wind_Left2 0.6Dead+0.6Wind_Right2 0.6Dead+0.6Wind_Right2 0.6Dead+0.6Wind_Pressu 1.13Dead+1.13Collateral+	+0.6Wind_Suction	n p2L

BUILDING	BRACING	REACTION	IS		
	— Col — Line Ho	± Reactions —Wind — - rz Vert F	s(k ) —Seismic — Horz Vert	Panel_Shear (lb/ft) Wind Seis	Note
L_EW 1 F_SW A	3,4 6,7				(h) (a) (a)
R_EW 9 B_SW F	6,7 3,4				(h) (a) (a)
(a)Wind ben (h)Rigid fra	t in bay ne at endwall				
Reactions fo	or seismic repres	ent shear for	ce, Eh		

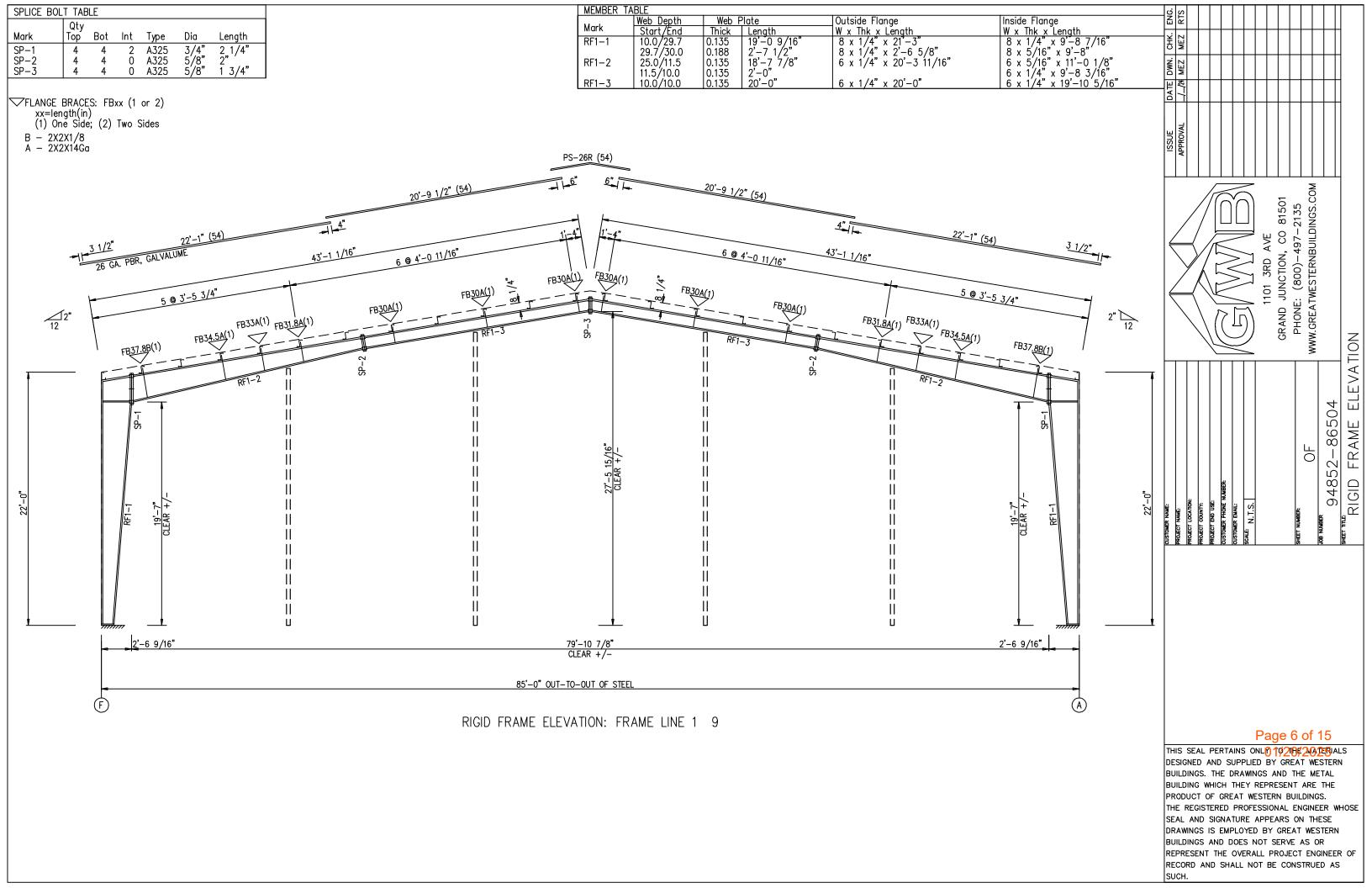


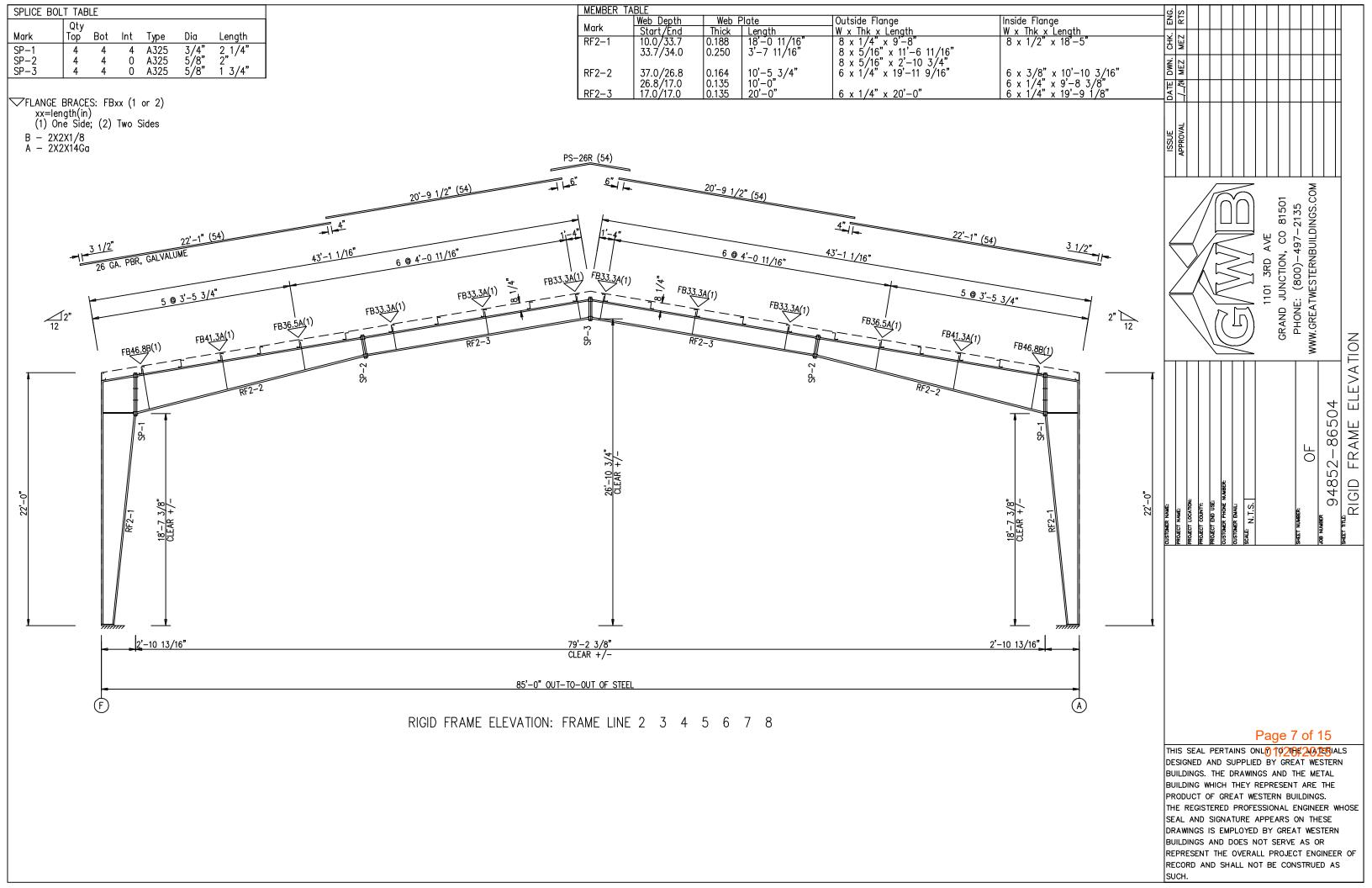


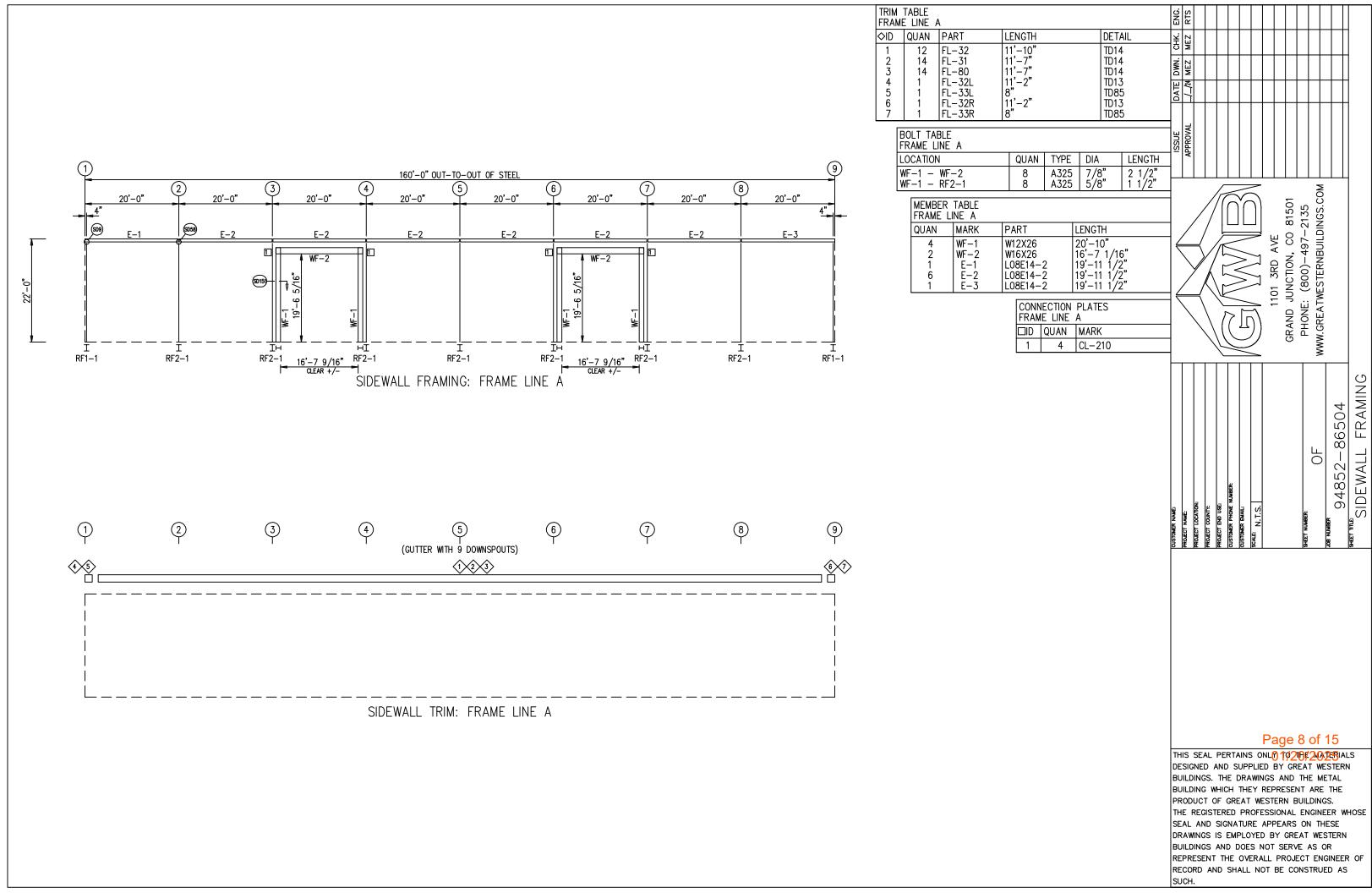


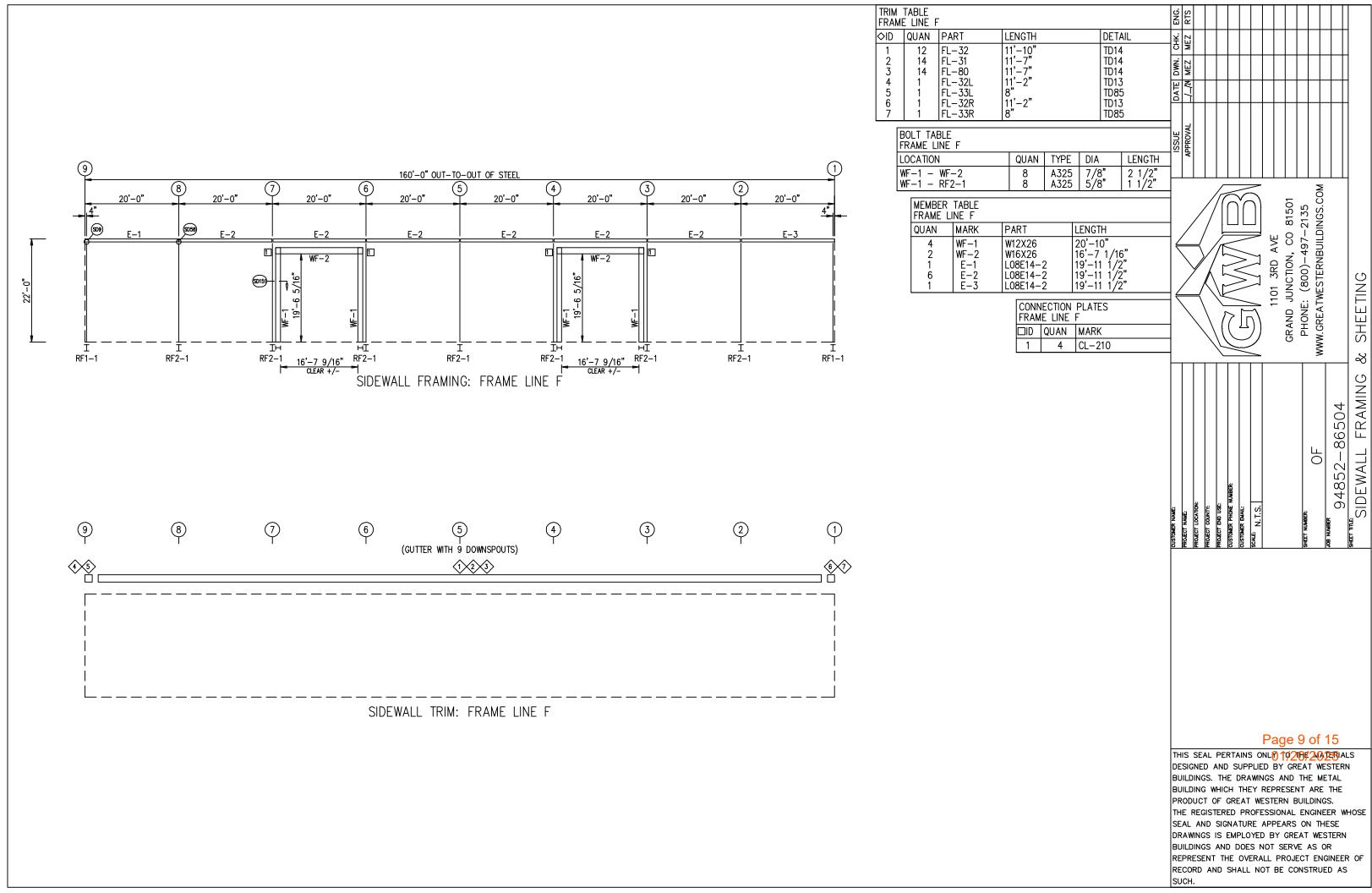
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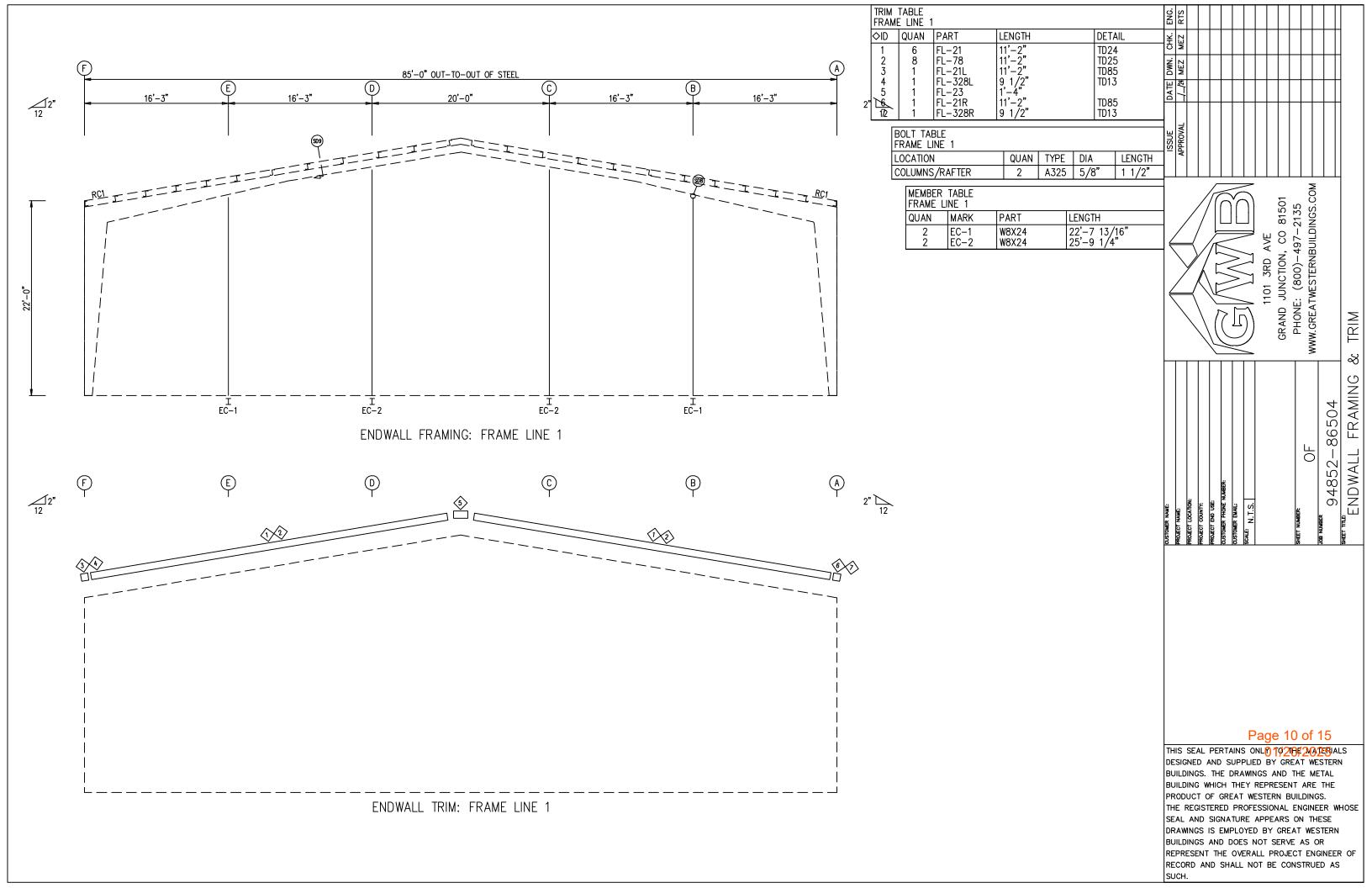
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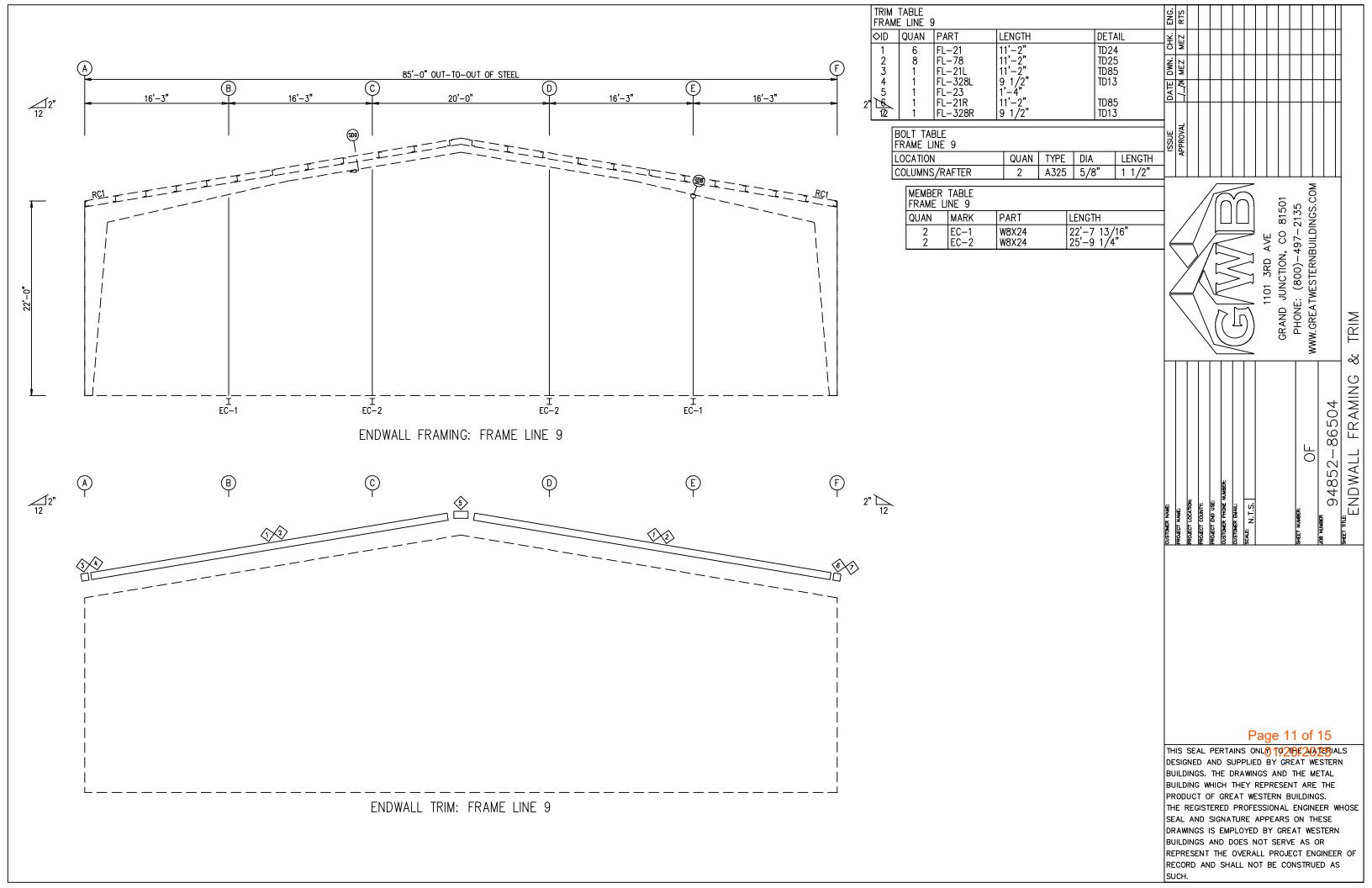


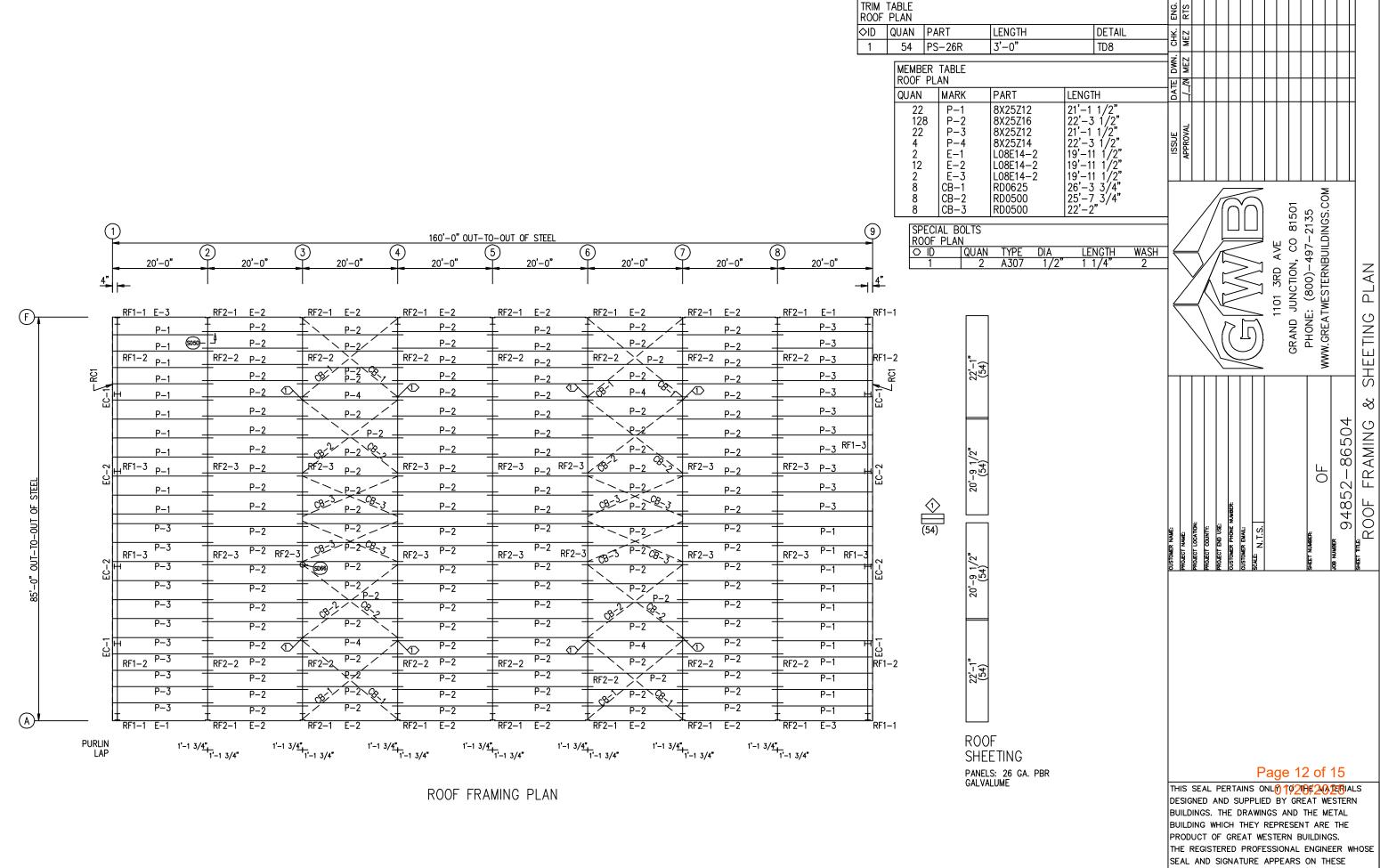












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