

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1

1.2	MATERIALS HOT ROLLED STEEL SHAPES (W, & C) HOT ROLLED STEEL ANGLES (L) STEEL PIPES STRUCTURAL TUBING STRUCTURAL STEEL WEB PLATE STRUCTURAL STEEL FLANGE PLATES/BARS COLD FORMED LIGHT GAGE ROOF & WALL SHEETS CABLE BRACE	ASTM_DESIGNATION A572 A38 A500 A500 A500 A572/A1011 A529/A572 A653/A1011 A792/A653 A475 — TYPE 1 A36	MIN. YIELD STRENGTH Fy = 50 KSI Fy = 36 KSI Fy = 42 KSI Fy = 50 KSI Fy = 55 KSI Fy = 55 KSI Fy = 50, 80 KSI EXTRA HIGH STRENGTH Fy = 36 KSI
			MIN. TENSILE STRENGTH

Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI

MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & & LESS) MACHINE BOLTS & NUTS
HIGH STRENGTH BOLTS (1"0 & LESS)
HIGH STRENGTH BOLTS (1"0 TO 1 1/2"0)
ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)
A36/A307/F1554

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :
A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N
B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANGE BRACE CONNECTIONS - 1/2" x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AIG. "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

- .7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
- INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE— BETWEEN ROOF PANELS & RIDGE CAP
 - BETWEEN WALL PANELS & EAVE/GABLE TRIM

.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO

ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

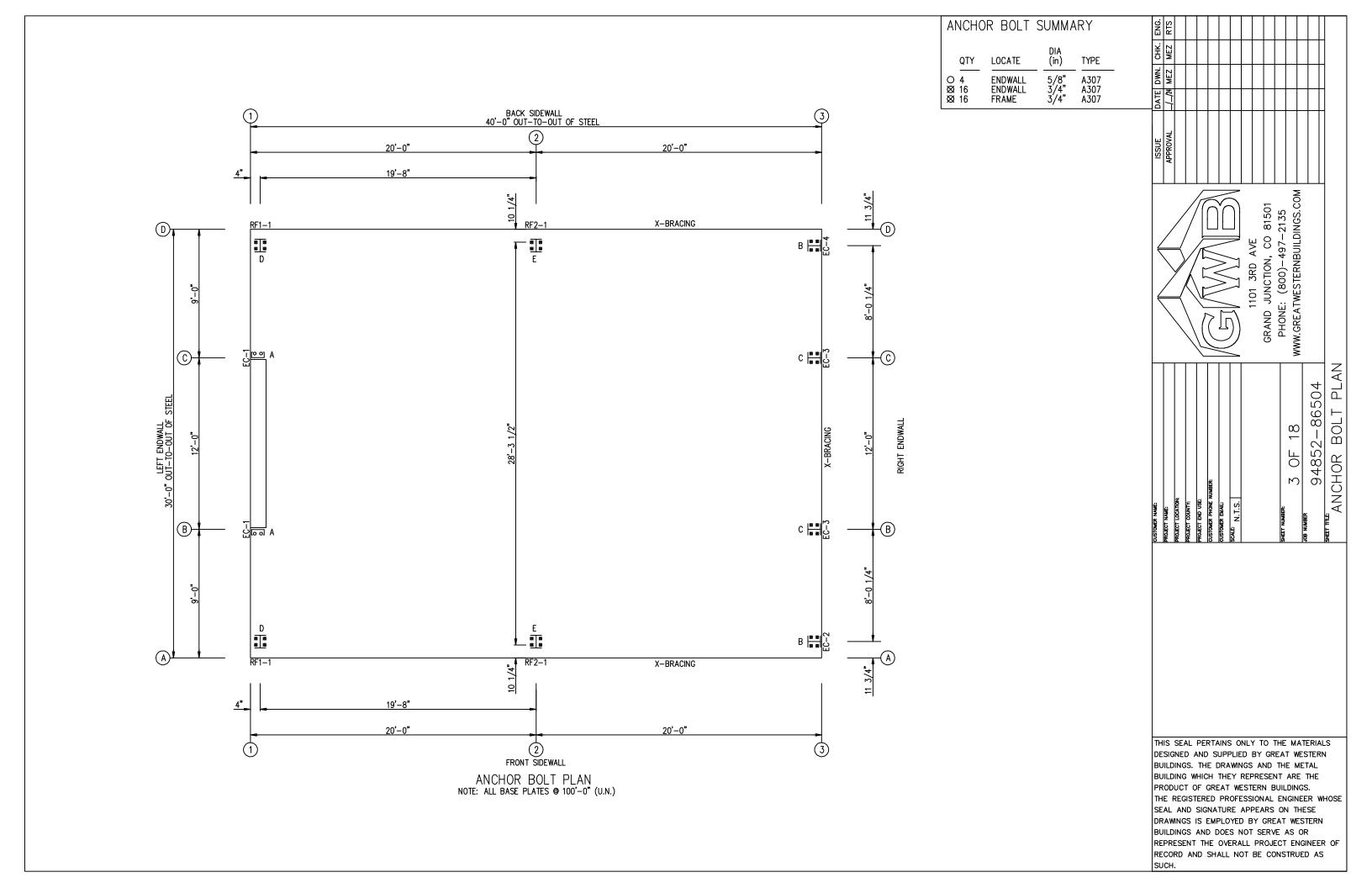
- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
 TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARRICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAFEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

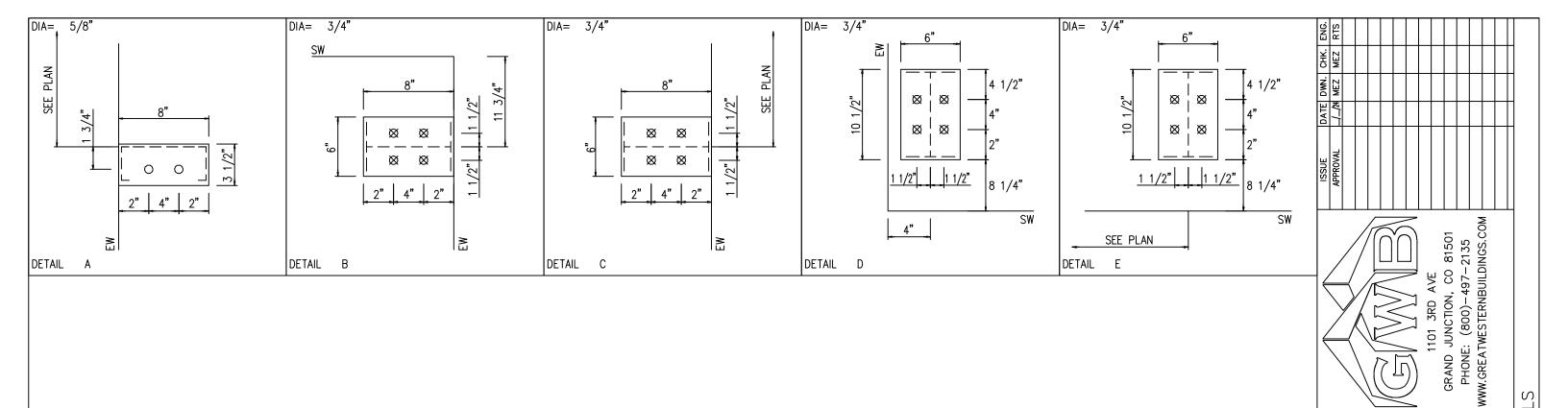
- THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, ATTEST ENTITION)
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT, THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD EPACTOR LATEST ENTION).
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER USCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY, MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY METINGS HIGHLIGHTING SAFETY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY METINGS SIGNED SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP, SEE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20
NOTE:		•

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: **DESIGN LOADS:** DESIGN CODE / WIND CODE : IBC-21 OCCUPANCY / RISK CATEGORY : II-Normal ENCLOSURE : Enclosed ROOF DEAD LOAD (D) (PSF) : 2.00 ROOF COLLATERAL LOAD (C) (PSF) : 1.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) :125.00 WIND EXPOSURE CATEGORY : C INTERNAL PRESSURE COEFFICIENT, (GCpi) : 0.18/-0.18 WALL PANEL DESIGN WIND PRESSURE (PSF) : 28.07/-30.41 WIND ENCLOSURE CLASSIFICATION : Enclosed LIVE LOAD PRIMARY FRAMING (PSF) : 20.00 GRAND JUNCTION, CO 81501 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.C : No TRIB. AREA REDUCTION SECONDARY FRAMING (PSF) : 20.00 AVE , CO SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) : 5.00 ROOF SNOW LOAD, (Pf) (PSF) : 40.00 3RD SNOW EXPOSURE FACTOR, (Ce) : 1.00 SNOW IMPORTANCE FACTOR, (Is) : 1.00 5 THERMAL FACTOR, (Ct) : 1.20 SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) : 1.00 SITE CLASSIFICATION : d SHE SPECTRAL RESPONSE ACCELERATION : Ss = 1.144 : S1 = 0.443SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.914 : Sd1 = 0.548SEISMIC DESIGN CATEGORY : D \propto :STEEL SYSTEM NOT SPECIFICALLY BASIC SEISMIC FORCE RESISTING SYSTEM DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) \circ :BRACED FRAMES (OCBF/OMF) 50, Ō TOTAL DESIGN BASE SHEAR, (V) (KIPS) :LONGITUDINAL = 2.30 0 :TRANSVERSE = 2.56 9 \propto ∞ :RIGID FRAMES = 3.25 $\Omega = 3.00$ RESPONSE MODIFICATION FACTORS, (R) :SW X-BRACING = 3.25 $\Omega = 2.00$ 2 () $\overline{\circ}$ 48 SEISMIC RESPONSE COEFFICIENTS, (Cs) \leq :RIGID FRAMES = 0.2815:SW X-BRACING = 0.2815 $\overline{\Box}$ Ò \sim BUIL ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS **BUILDING DESCRIPTION:** WIDTH (FT) : 30.00 LENGTH (FT) : 40.00 EAVE HEIGHT AT BSW (FT): 15.00 EAVE HEIGHT AT FSW (FT): 15.00 ROOF SLOPE AT BSW : 2.0:12 ROOF SLOPE AT FSW : 2.0:12 BAY SPACING (FT) :2 AT 20.00 **COVERING AND TRIMS: ROOF PANELS & TRIMS** PANEL TYPE :26 GA. PBR PANEL COLOR : GALVALUME TRIM COLORS :CHARCOAL GRAY GABLE/EAVE :NEED STD. COLOR EAVE GUTTER WALL PANELS & TRIMS PANEL TYPE :26 GA. PBR PANEL COLOR : ASH GRAY THIS SEAL PERTAINS ONLY TO THE MATERIALS TRIM COLORS DESIGNED AND SUPPLIED BY GREAT WESTERN CORNER :CHARCOAL GRAY BUILDINGS. THE DRAWINGS AND THE METAL : CHARCOAL GRAY FRAMED OPENING BUILDING WHICH THEY REPRESENT ARE THE DOWNSPOUTS : NEED STD. COLOR PRODUCT OF GREAT WESTERN BUILDINGS. : CHARCOAL GRAY BASE THE REGISTERED PROFESSIONAL ENGINEER WHOSE INSULATION SEAL AND SIGNATURE APPEARS ON THESE ROOF INSULATION DRAWINGS IS EMPLOYED BY GREAT WESTERN WALL INSULATION :N/A BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS



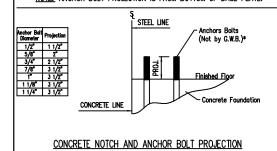


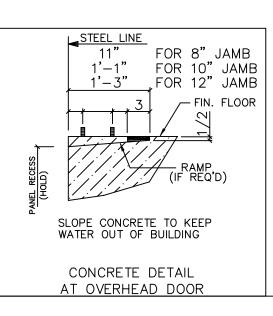
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS

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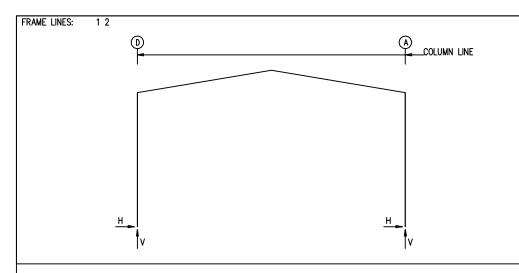
BOL.

ANCHOR

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RIGID F	FRAME:		MAXIMUM	REACTION	NS, ANC	HOR BOLT	S, & BASI	E PLATE	ES				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Boli QTY	t(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)
1	D	3 1	1.7 1.6	4.6 6.9	6 4	-1.3 -1.0	-1.1 -1.9	4	0.750	6.000	10.50	0.375	0.0
1	Α	7 1	1.3 -1.6	-1.1 6.9	2 5	-1.7 1.0	4.6 -1.9	4	0.750	6.000	10.50	0.375	0.0

RIGID	FRAME:		MAXIMUM	REACTION	IS, ANC	HOR BOLT	rs, & Bas	E PLATE	S				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Boli QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2	D	3 1	4.4 4.4	10.7 16.4	6 4	-3.2 -2.7	-2.9 -5.0	4	0.750	6.000	10.50	0.375	0.0
2	Α	7 1	3.2 -4.4	-2.9 16.4	2 5	-4.4 2.7	10.7 -5.0	4	0.750	6.000	10.50	0.375	0.0

END	WALL	COLUI	MN:	BASIC C	OLUMN REA	CTIONS (k)							
Frm Line 1	Col Line C B	Dead Vert 0.0 0.0	Wind Press Horz -1.8 -1.8	Wind Suct Horz 2.0 2.0	Seis Long Horz 0.1 0.1									
Frm Line 3 3 3	Col Line A B C D	Dead Vert 0.2 0.5 0.5 0.2	Collat Vert 0.0 0.1 0.1 0.0	Live Vert 0.7 2.4 2.4 0.7	Snow Vert 1.4 4.7 4.7 1.4	Wind_ Horz 0.0 -1.6 0.0 0.0	Left1 Vert -0.9 -5.3 0.1 -0.8	Wind_ Horz 0.0 0.0 1.6 0.0	Right1 Vert -0.8 0.1 -5.3 -0.9	Wind_l Horz 0.0 -1.6 0.0 0.0	Left2 Vert -0.4 -4.3 1.1 -0.3	Wind_ Horz 0.0 0.0 1.6 0.0	Right2 Vert -0.3 1.1 -4.3 -0.4	
Frm Line 3 3 3 3	Col Line A B C D	Wind_P Horz -3.1 -1.7 -1.7 -3.1	ress Vert -1.6 0.0 0.0 -1.6	Wind_Suct Horz Ve 0.9 1.6 1.9 0.0 1.9 0.0 0.9 1.6	rt Horz 5 0.0 0.0 0 0.3	_Long1 Vert -1.3 -2.4 -2.1 -0.8	Wind_ Horz 0.0 -0.3 0.0 0.0	Long2 Vert -0.8 -2.1 -2.4 -1.3	Seis_ Horz 0.0 -0.8 0.0 0.0	Left Vert 0.0 -1.1 1.1 0.0	Seis_F Horz 0.0 0.0 0.8 0.0	Right Vert 0.0 1.1 -1.1 0.0	Seis_ Horz -1.5 0.1 0.1 -1.5	Long Vert -1.0 0.0 0.0 -1.0
Frm Line 3 3 3 3	Col Line A B C D	-MIN_S Horz 0.0 0.0 0.0 0.0	SNOW Vert 0.2 0.6 0.6 0.2	E2UNB_SL_I Horz Ve 0.0 1.4 0.0 4.6 0.0 1.9 0.0 0.3	rt Horz 0.0 0.0 0.0	IB_SL_R- Vert 0.3 1.9 4.6 1.4								

ENDWALL	COLUMN:
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MAXIMIIM	REACTIONS	ANCHOR	ROLTS	۶٠	BASE PLATES	
MAVIMON	NEACHONS,	ANCHOR	DULIS,	α	DASE FLAILS	

Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
1	С	8 10	1.2 0.9	0.0 0.0	9	-1.1	0.0	2	0.625	3.500	8.000	0.250	0.0
1	В	8 10	1.2 0.9	0.0 0.0	9	-1.1	0.0	2	0.625	3.500	8.000	0.250	0.0
3	Α	11 13	0.5 0.4	0.3 1.9	12	-1.9	-1.6	4	0.750	6.000	8.000	0.375	0.0
3	В	14 1	1.2 0.0	-2.9 5.3	12 14	-1.0 1.2	-1.2 -2.9	4	0.750	6.000	8.000	0.375	0.0
3	С	15 1	1.2 0.0	-2.9 5.3	9 15	-1.0 1.2	-1.2 -2.9	4	0.750	6.000	8.000	0.375	0.0
3	D	16 17	0.5 0.4	0.3 1.9	9	-1.9	-1.6	4	0.750	6.000	8.000	0.375	0.0

RIGID F	FRAME	Ξ:	BASIC	COLUM	N REACTION	ONS (k)									
FRAME Co Line Lin 1 D 1 A	ne H		ead Vert 0.7 0.7	——Colla Horz 0.0 0.0	teral— Vert 0.2 0.2	Horz 0.7 -0.7	Live Vert 3.1 3.1	Horz 1.4 –1.4	Snow Vert 6.1 6.1	-—-Wind_ Horz -1.8 -1.0	Left1- Vert -3.9 -1.8	-Wind_F Horz 1.0 1.8	Right1- Vert -1.8 -3.9		
FRAME Co Line Lin 1 D 1 A	ne H	Wind_ lorz -2.2 -0.7	Left2- Vert -2.5 -0.4	-Wind_ Horz 0.7 2.2	Right2- Vert -0.4 -2.5	Wind_ Horz 0.6 -0.4	Long1- Vert -3.0 -2.6	Wind Horz 0.4 -0.6	_Long2- Vert -2.6 -3.0	-Seismid Horz -0.4 -0.4	c_Left Vert -0.4 0.4	Seismic Horz 0.4 0.4	_Right Vert 0.4 -0.4		
FRAME Co Line Lin 1 D 1 A	ne H	-MIN_SN Horz 0.2 -0.2	0W Vert 0.8 0.8	F1UNB_ Horz 1.0 -1.0	SL_L- Vert 5.2 2.9	F1UNB_S Horz 1.0 -1.0	SL_R- Vert 2.9 5.2								
FRAME Co Line Lin 2 D 2 A	ne H	D Horz 0.3 -0.3	ead Vert 1.1 1.1	——Collo Horz 0.1 —0.1	teral- Vert 0.4 0.4	Horz 2.0 -2.0	Live Vert 7.4 7.4	Horz 4.0 -4.0	Snow Vert 14.9 14.9	-—-Wind_ Horz -4.8 -2.2	Left1- Vert -9.5 -4.5	-Wind_F Horz 2.2 4.8	Right1- Vert -4.5 -9.5		
FRAME Co Line Lin 2 D 2 A	ne H	Wind_ lorz -5.5 -1.5	Left2- Vert -6.0 -1.0	−Wind_ Horz 1.5 5.5	Right2- Vert -1.0 -6.0	Wind_ Horz 1.4 -0.9	Long1- Vert -9.0 -7.9	Wind Horz 0.9 -1.4	_Long2- Vert -7.9 -9.0	-Seismid Horz -0.8 -0.8	c_Left Vert -0.8 0.8	Seismic Horz 0.8 0.8	_Right Vert 0.8 -0.8		
FRAME Co Line Lin 2 D 2 A	ne H	-Seismic Iorz 0.0 0.0	Long Vert -1.0 -1.0	-MIN_S Horz 0.5 -0.5	NOW Vert 1.9 1.9	F2UNB_5 Horz 2.7 -2.7	SL_L- Vert 12.8 7.2	F2UNB_ Horz 2.7 -2.7	SL_R- Vert 7.2 12.8						

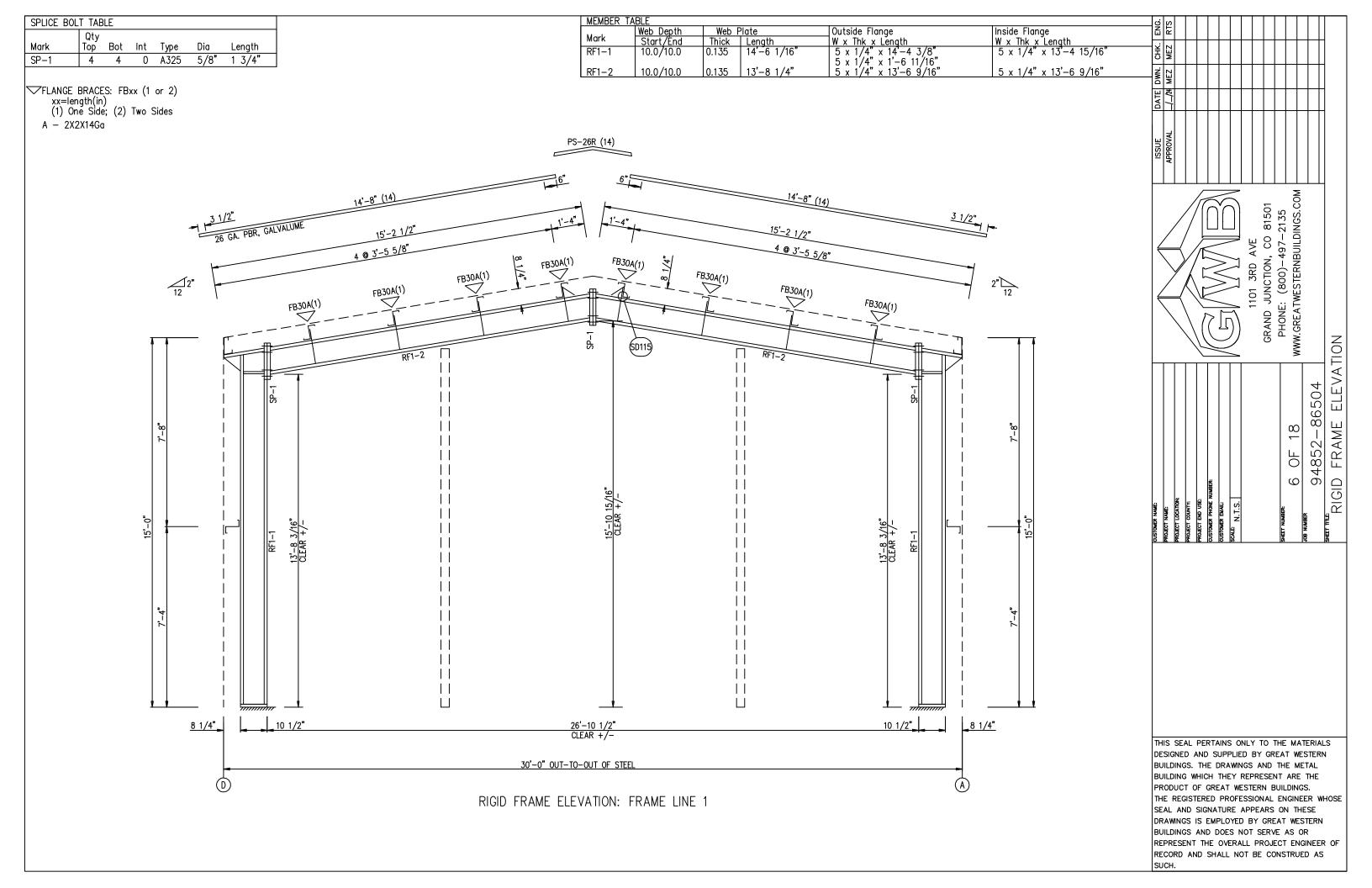
NOTES	FOR REACTIONS	
the 1	ing reactions are based on following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Callateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= 30.00 = 40.00 = 15.00/15.00 = 2.0:12/2.0:12 = 2.00 = 1.00 = 20.00 = 40.00 = 125.00 = IBC-21 C C = Enclosed = 1.00 = 1.37
ID	Description	
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17		L_Right1+0.75Slide_Snow uction Long2L .45Wind_Suction+0.75E1UNB_SL_R .ong1L .tong1L .45Wind_Suction+0.75E2UNB_SL_L ction uction .ong2L

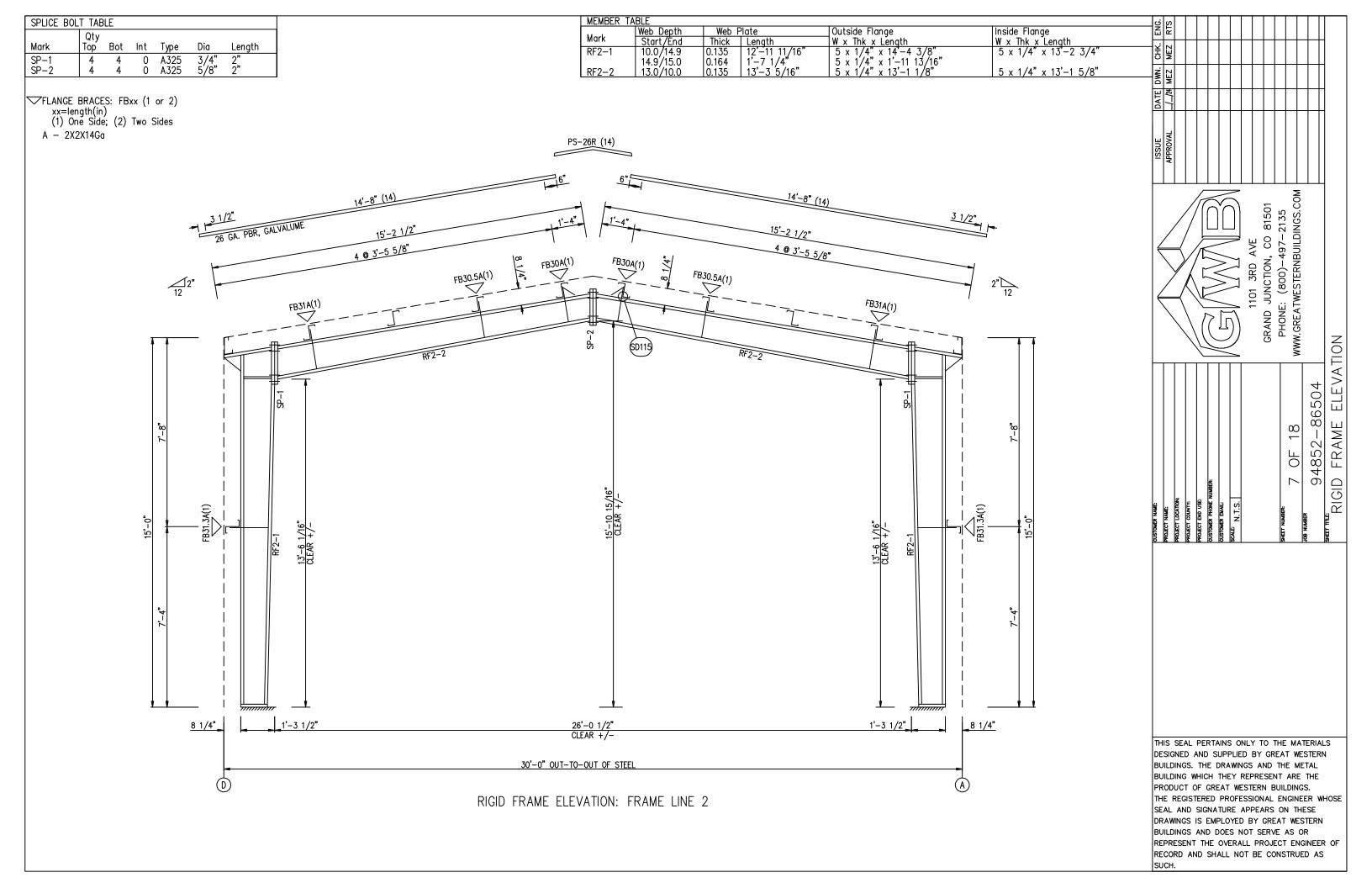
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F_SW R_EW B SW	A 3 D	2,3 B,C 3,2	2.4 1.6 2.4	1.6 2.1 1.6	1.5 0.8 1.5	1.0 1.1 1.0		

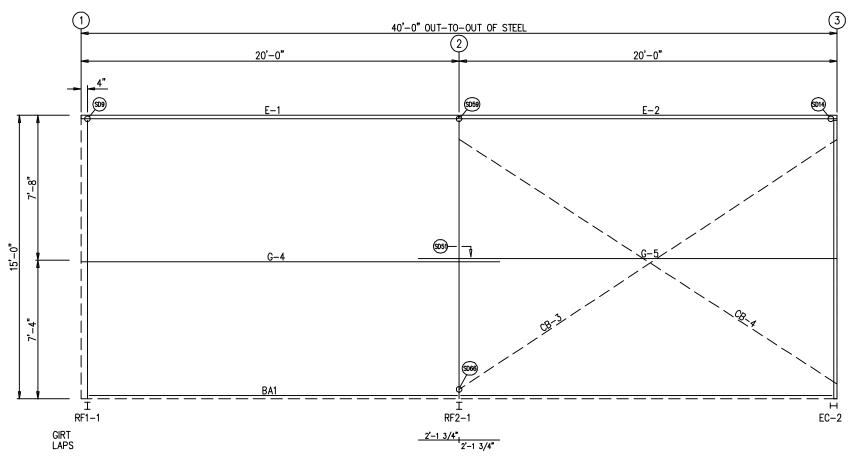
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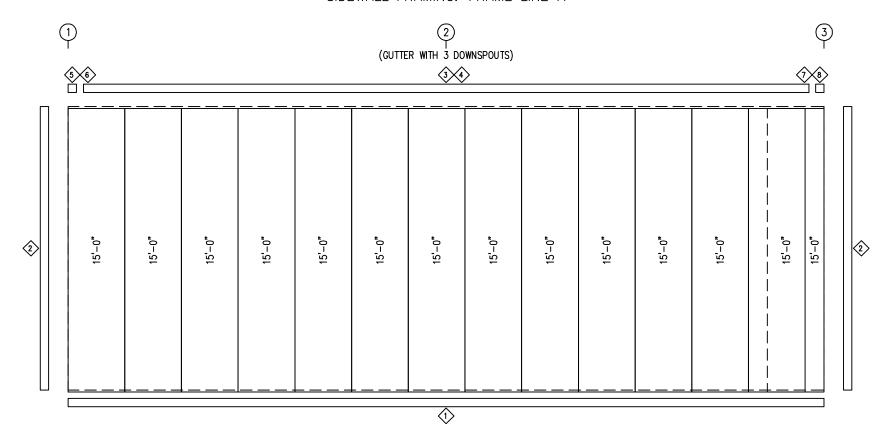
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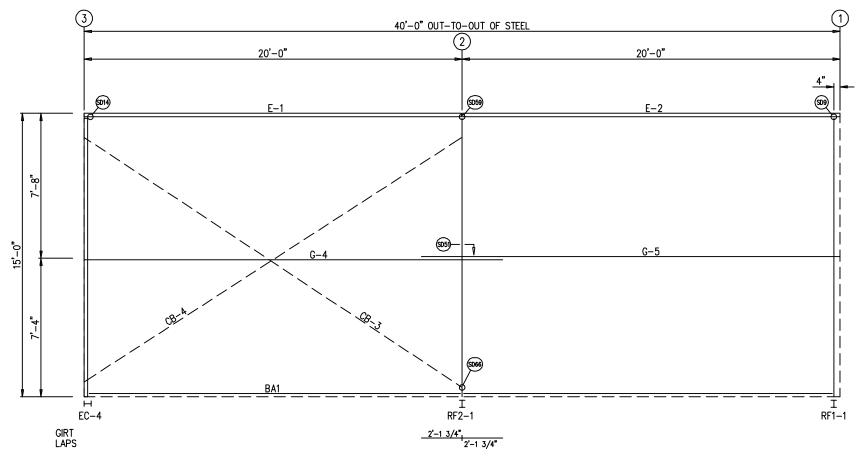


SIDEWALL FRAMING: FRAME LINE A

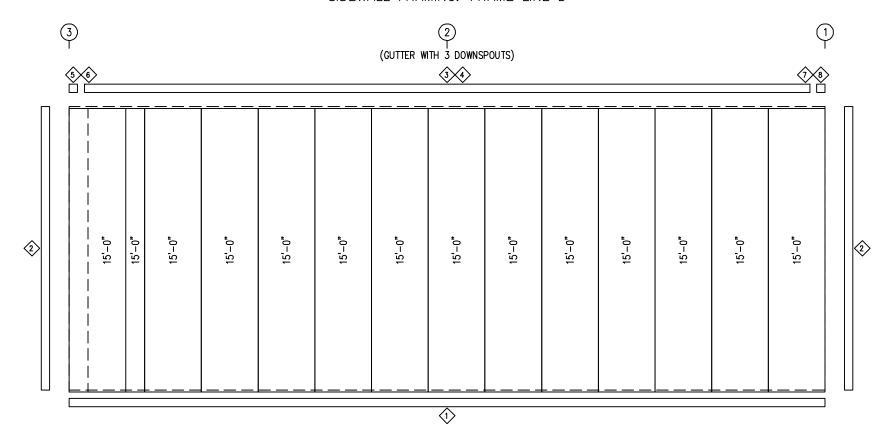


SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR – ASH GRAY

TRIM FRAM ◇ID 1 2 3 4 5 6 7 8	TABLE E LINE A QUAN PART 4 FL-60 2 FL-10 2 FL-32 4 FL-31 1 FL-33L 1 FL-33R MEMBER TABLE FRAME LINE A QUAN MARK 1 E-1 1 E-2 1 G-4 1 G-5 1 CB-3	LENGTH 10'-2" 15'-0" 10'-1" 10'-2" 11'-2" 8" 11'-2" 8" PART L08E16-2 L08E16-2 8X25Z16 8X25Z16 RD0500	DETAIL TD74 TD40 TD15 TD15 TD13 TD85 TD13 TD85 TD13 TD85 **TD13 **TD85 **TD13 **TD1	ISSUE DATE DWN. CHK. ENG. APPROVAL J_M MEZ MEZ RTS APPROVAL J_M M	
	1 CB-4	RD0500	25-11 1/2	MOSECHE NAME: PROJECT INJUST. PROJECT INJUST. SCALE. N.T.S. SCALE. N.T.S. SCALE. N.T.S. SCALE. N.T.S. SCALE NUMBER SCALE	NING &
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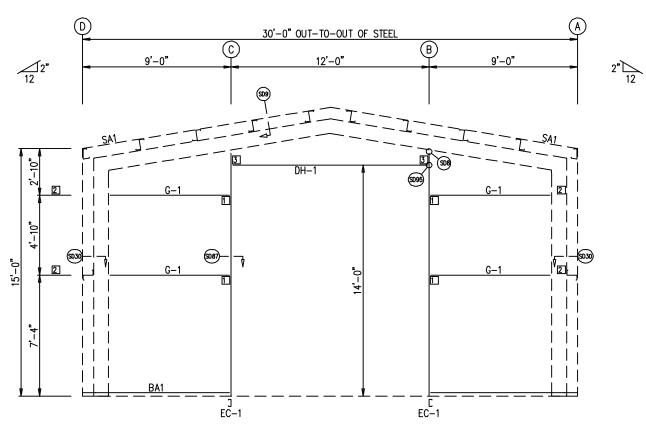


SIDEWALL FRAMING: FRAME LINE D

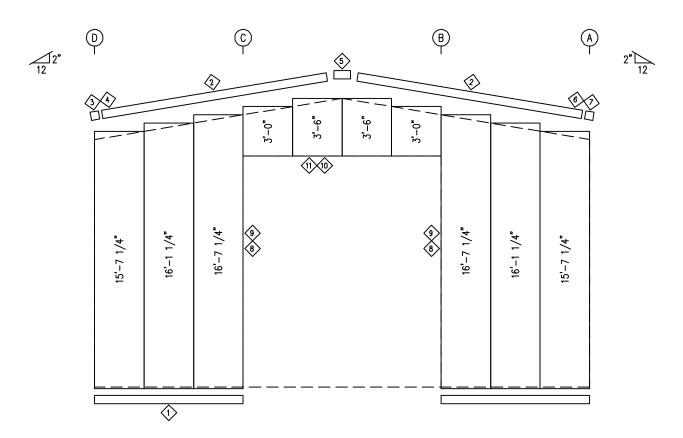


SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 GA. PBR - ASH GRAY

TRIM TABLE	ENG.
FRAME LINE D ◇ID QUAN PART LENGTH DETAIL	MEZ R
1 4 FL-60 10'-2" TD74 2 2 FL-10 15'-0" TD40 3 2 FL-32 10'-1" TD15 4 4 FL-31 10'-2" TD15 5 1 FL-32L 11'-2" TD13	
1 4 FL-60 10'-2" TD74 2 2 FL-10 15'-0" TD40 3 2 FL-32 10'-1" TD15 4 4 FL-31 10'-2" TD15 5 1 FL-32L 11'-2" TD13 6 1 FL-33L 8" TD85 7 1 FL-32R 11'-2" TD13 8 1 FL-33R 8" TD85	E DWN.
5 1 FL-32L 11'-2" TD13 10 10 10 10 10 10 10	DATE
7 1 FL-32R 11'-2" TD13 8 1 FL-33R 8" TD85	
MEMBER TABLE	ISSUE APPROVAL
FRAME LINE D QUAN MARK PART LENGTH	
1 E-1 L08E16-2 19'-11 1/2"	∑
1 E-2 L08E16-2 19'-11 1/2" 1 G-4 8X25Z16 22'-1 1/2" 1 G-5 8X25Z16 22'-1 1/2" 1 CB-3 RD0500 23'-7 3/4"	2 % %
1 E-1 L08E16-2 19'-11 1/2" 1 E-2 L08E16-2 19'-11 1/2" 1 G-4 8X25Z16 22'-1 1/2" 1 G-5 8X25Z16 22'-1 1/2" 1 CB-3 RD0500 23'-7 3/4" 1 CB-4 RD0500 23'-11 1/2"	DING
1 CB-4 RD0500 23'-11 1/2"	A A VE BUIL
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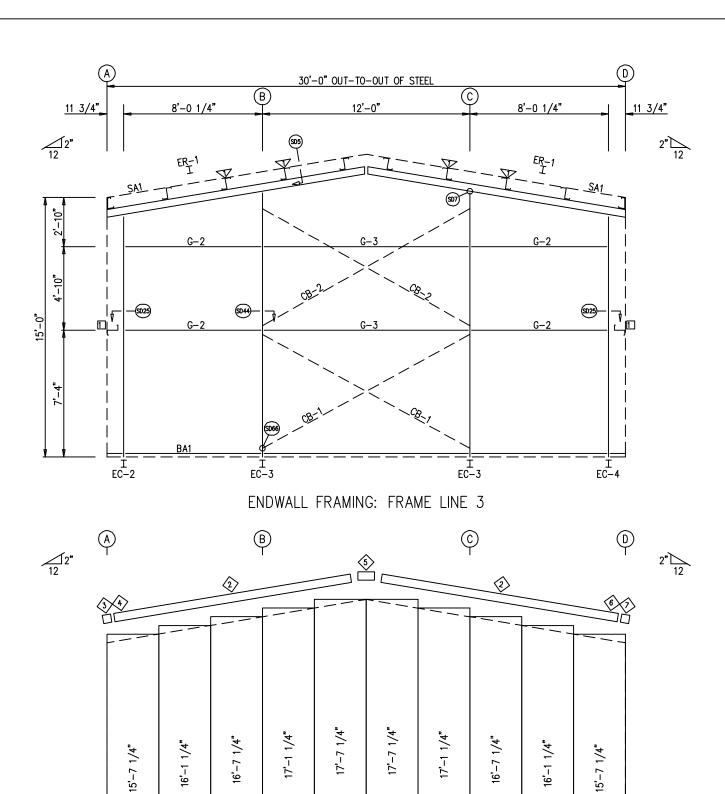


ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - ASH GRAY

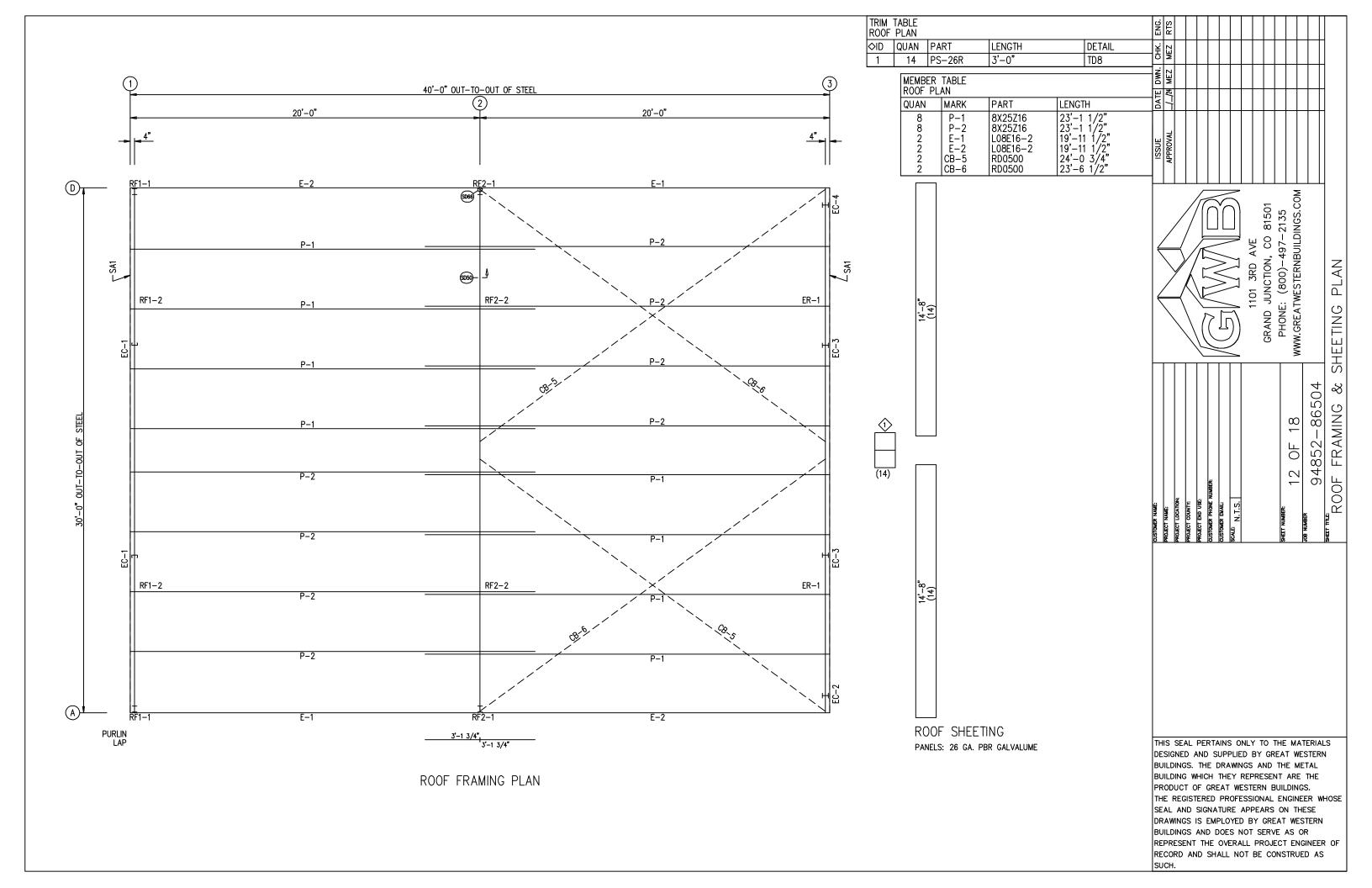
RIM TABLE RAME LINE 1 ID QUAN PART 1 2 FL-6 2 2 FL-2 3 1 FL-2 4 1 FL-3 5 1 FL-2 6 1 FL-2 7 1 FL-3 8 2 FL-5 9 2 FL-5 10 1 FL-5 11 1 FL-5	0 10'-2" 11 5'-3" 128L 9 1/2" 3 1'-4" 11R 11'-2" 128R 9 1/2" 5 14'-2" 8 14'-2" 12'-7"	DETAIL TD74 TD35 TD85 TD13 TD85 TD13 TD51 TD51 TD52 TD52	ISSUE DATE DWN. CHK. ENG.	1 ///24 MEZ							
2 E	TER 2 / A BLE 1 1 ARK PART C-1 8X35C16 H-1 8x25C16 G-1 8X25Z16 CONNEC FRAME I	LENGTH	₹ 	PROJECT NAME.	PROJECT COUNTY:	OUSTOWER PHONE NUMBER.	SCAME N.T.S.	GRAND JUNCTION, CO 81501	SHET NUMBER 10 OF 18 WWW.GREATWESTERNBUILDINGS.COM	94852-86504	ENDWALL FRAMING & SHEETING
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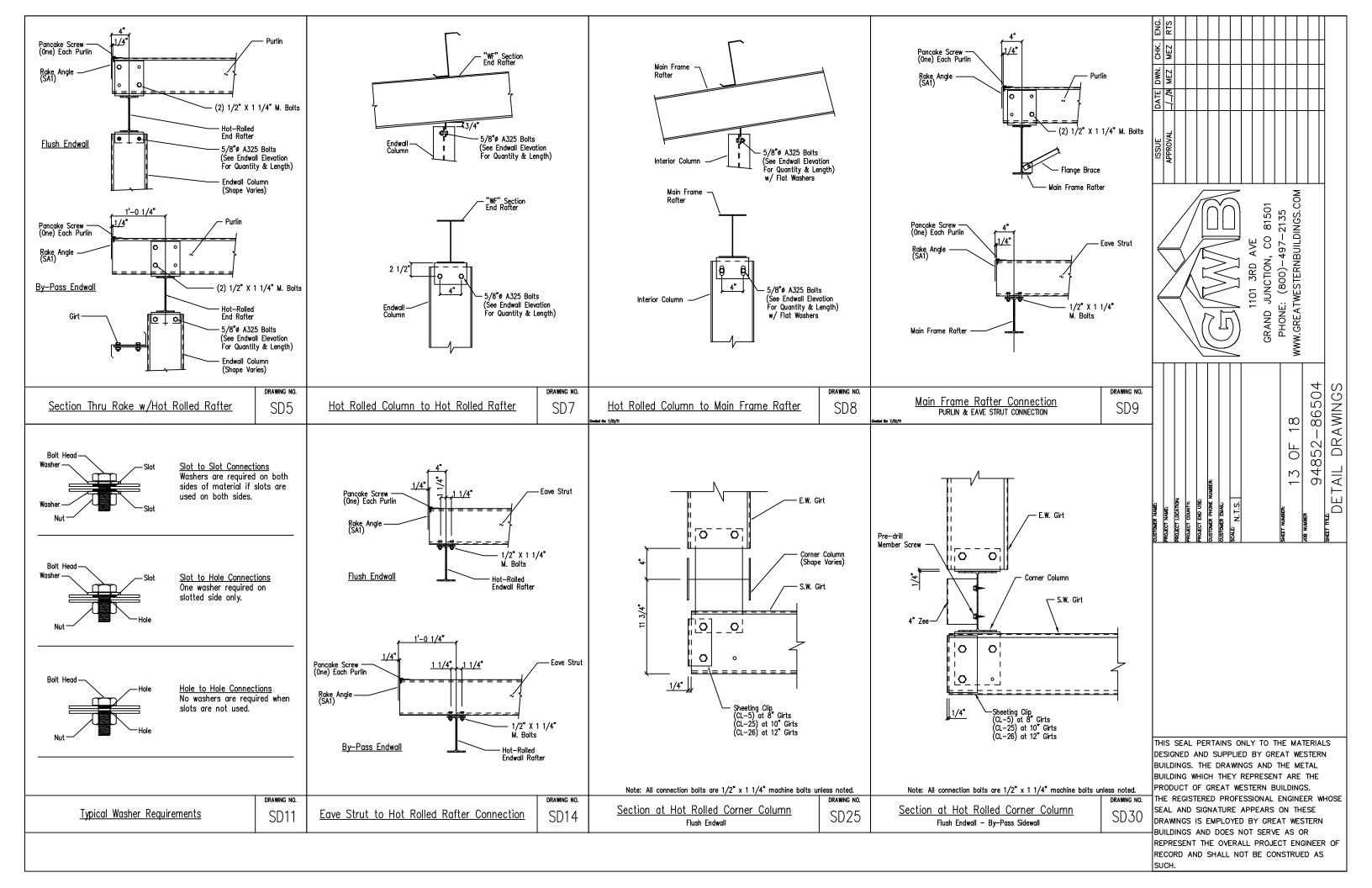


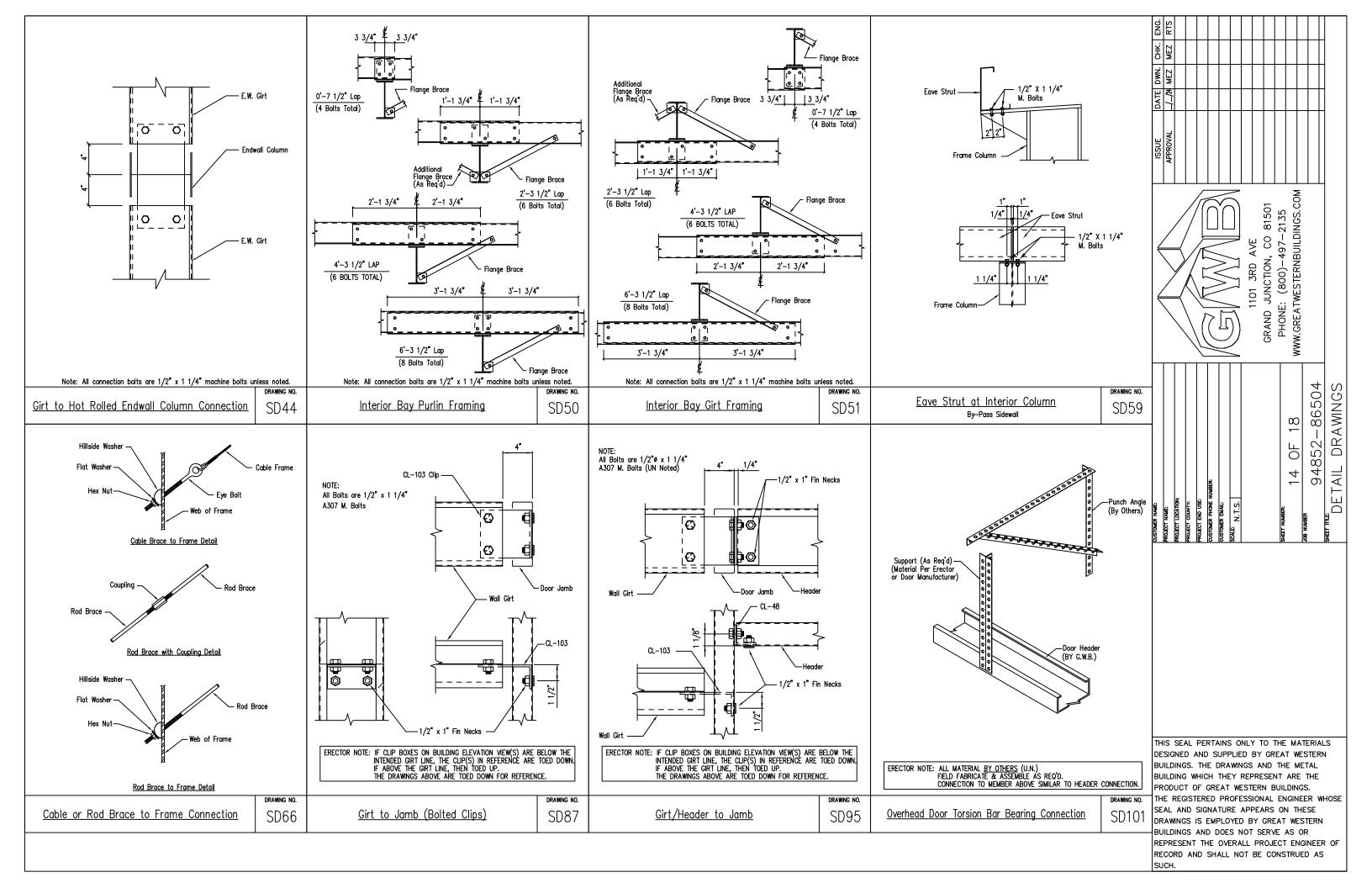
ENDWALL SHEETING & TRIM: FRAME LINE 3
PANELS: 26 GA. PBR - ASH GRAY

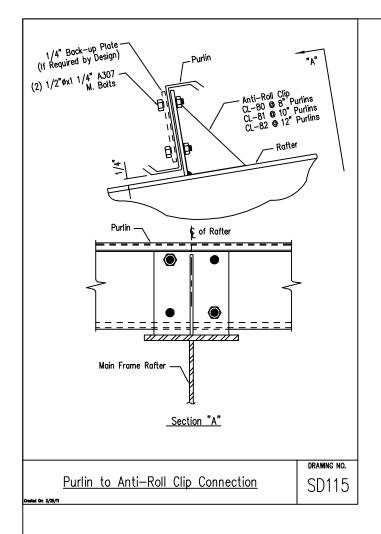
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3		FL-21L	11'-2"	TD85	DWN.											
4 5	1 1	FL-21L FL-328L FL-23	9 1/2" 1'-4"	TD13	DATE	10/	į									
2 3 4 5 6 7	1 1	FL-21R FL-328R	10'-2" 5'-3" 11'-2" 9 1/2" 1'-4" 11'-2" 9 1/2"	TD85 TD13	۴		╫		Н		\mathbf{H}		+	+	H	
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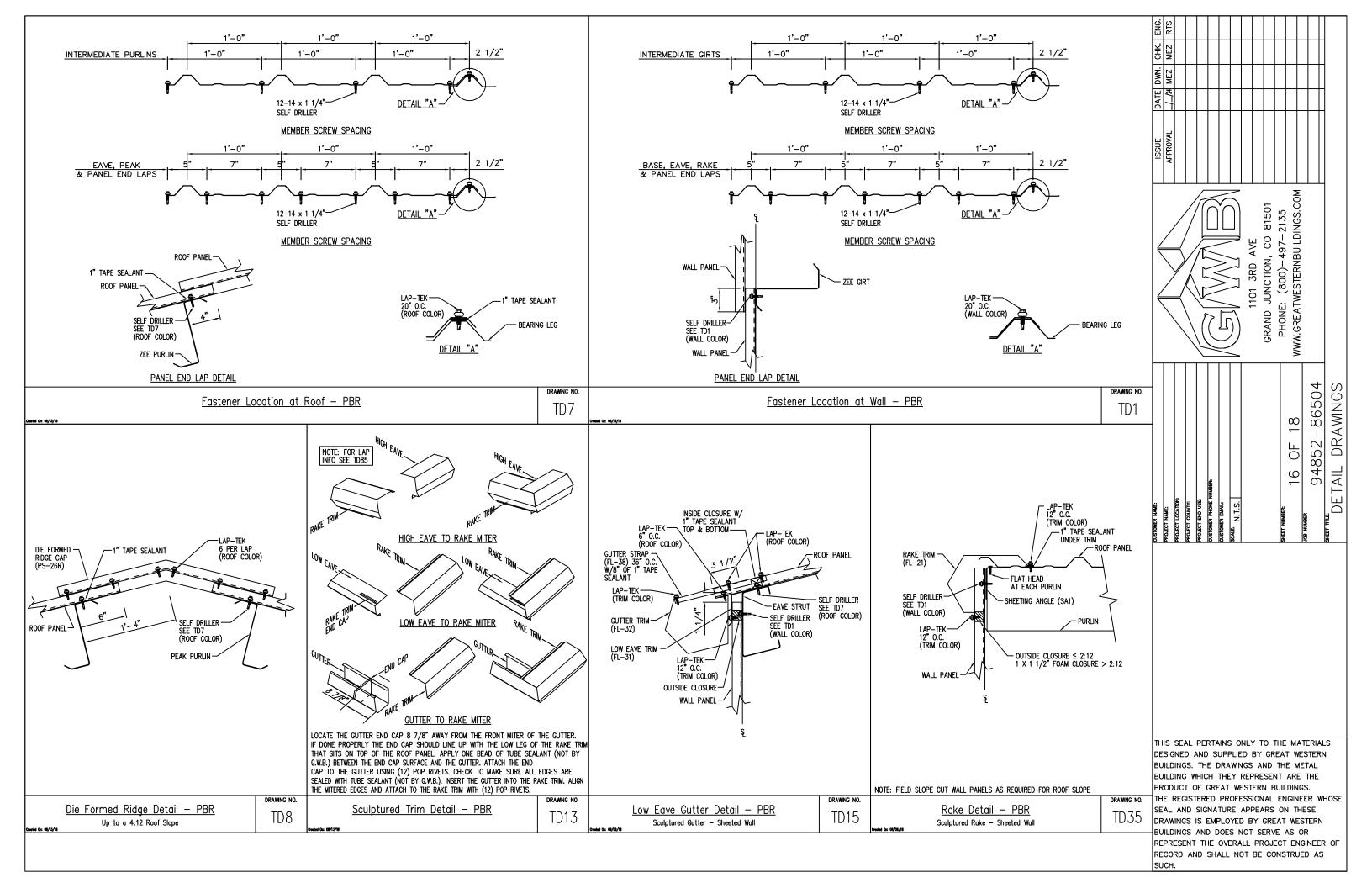


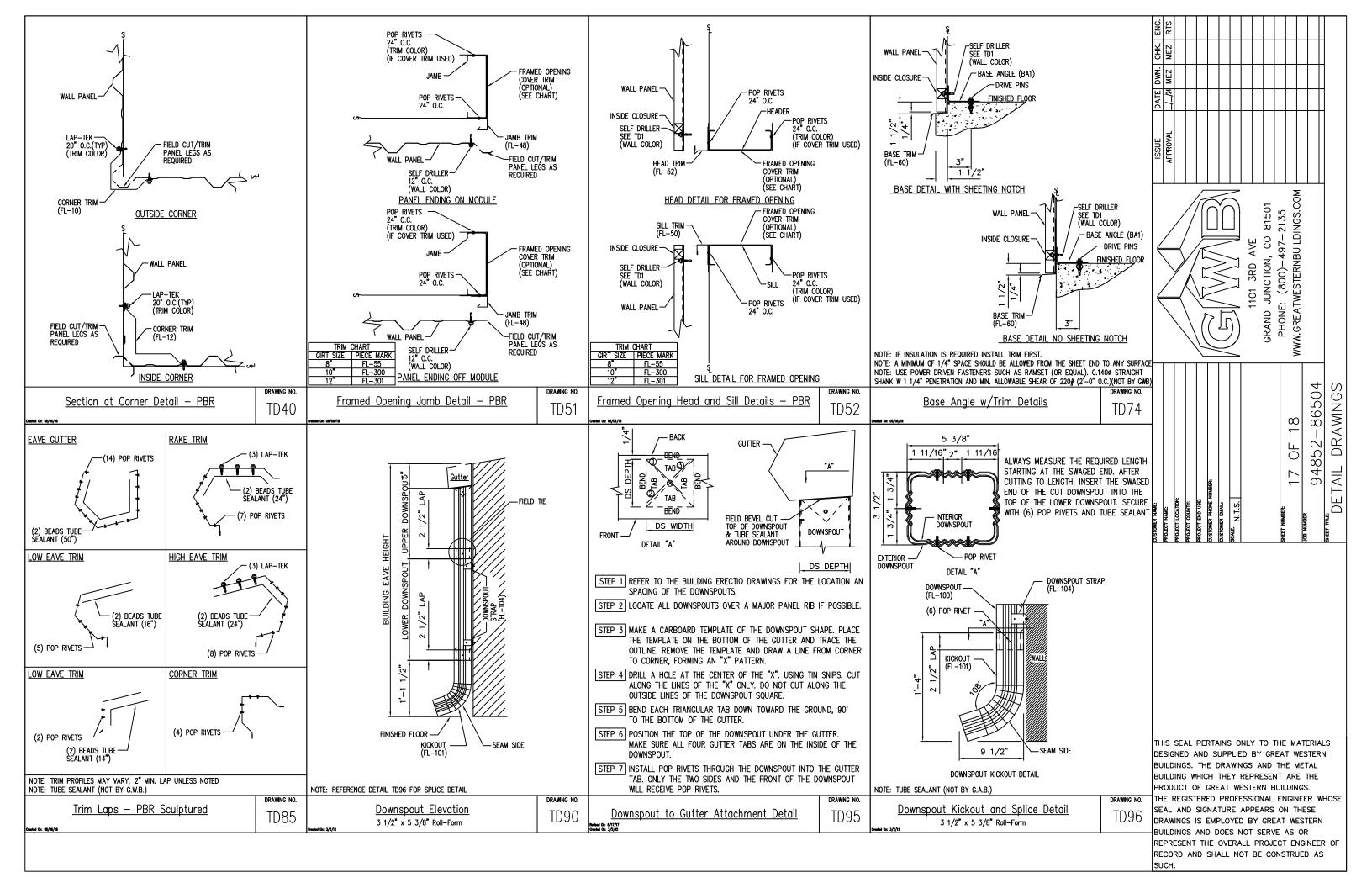


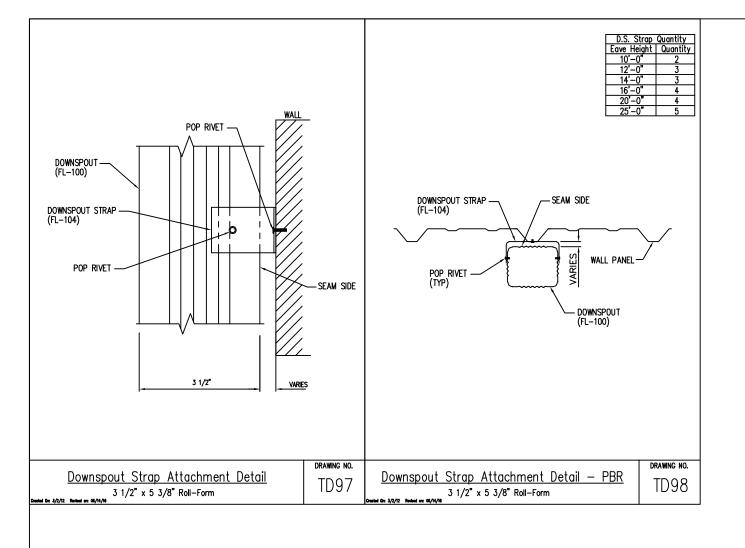


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