

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	$F_{y} = 36 \text{ KSI}$
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	$F_V = 50 \text{ KSI}$
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529 / A572	$F_V = 55 \text{ KSI}$
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HÌGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN TENSILE STRENGTH

MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1" & & LESS) Fu = 60 KSI Fu = 120 KSI Fu = 105 KSI MACHINE BOLTS & NUTS
HIGH STRENGTH BOLTS (1"ø & LESS)
HIGH STRENGTH BOLTS (1"ø TO 1 1/2"ø)
ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)
A36/A307/F1554

1.3 PRIMER

SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSTIE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT

I.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :
A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N
B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANGE BRACE CONNECTIONS - 1/2" x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE— BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

I.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO

ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10).

CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY EFMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/FAID USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE ONCE THE BUTER/END USE COSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. THE BUTER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COURDINATION.
ALL INTERFACE, COMPATBILITY, AND DESIGN CONSIDERATIONS
CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE
CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA
CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR
FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
LATTER EDITION)

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE CONCRETE.

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUITING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOCVER IS RESPONSIBLE ETHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFTICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD EPRACTE LATEST ENTURN)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT. DRILL OR OTHERWISE NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wirning, or tubing onto galvalume should be

2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY, MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES, DAILY METINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

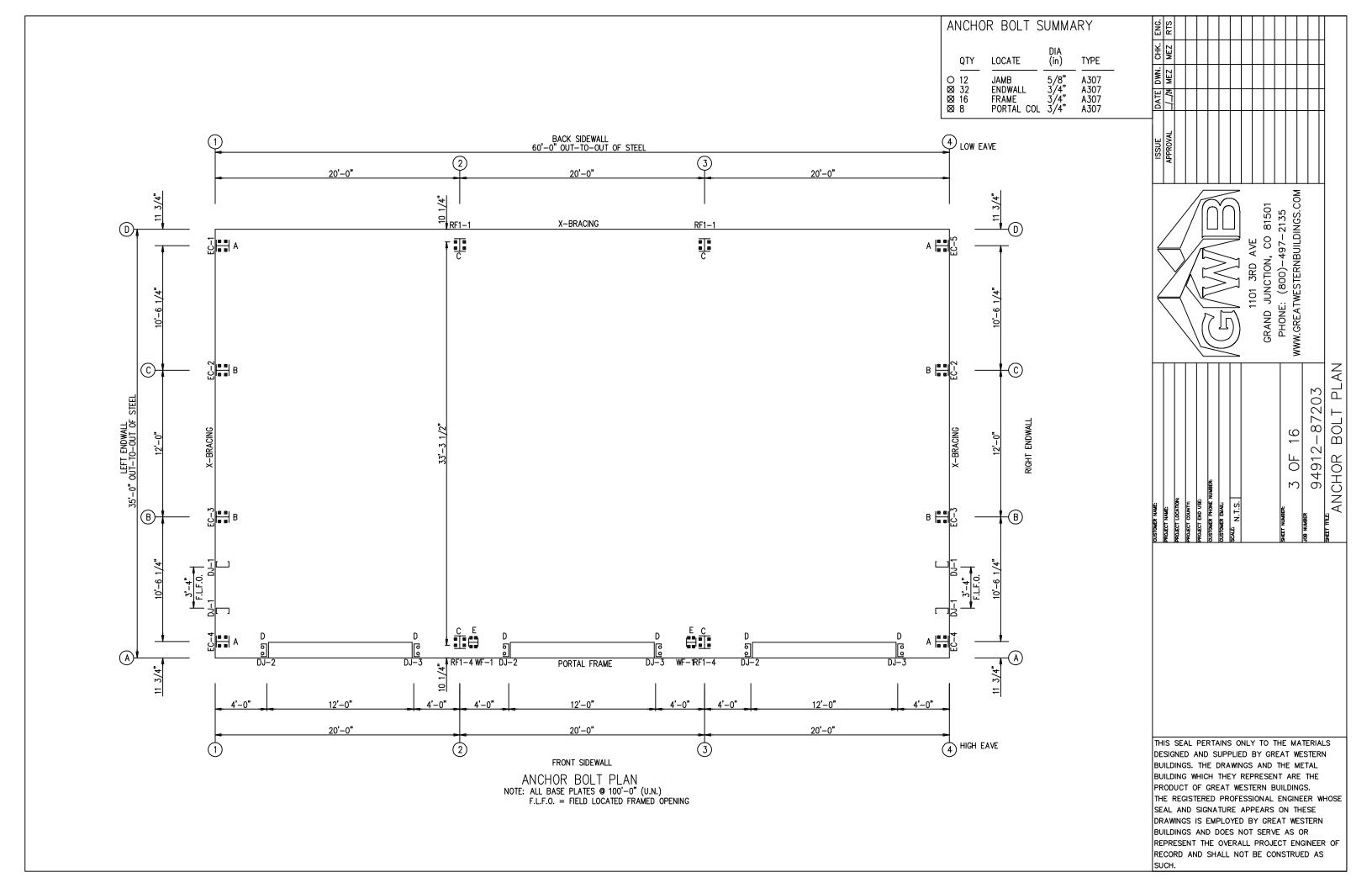
2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP, SEE TABLE BELOW.

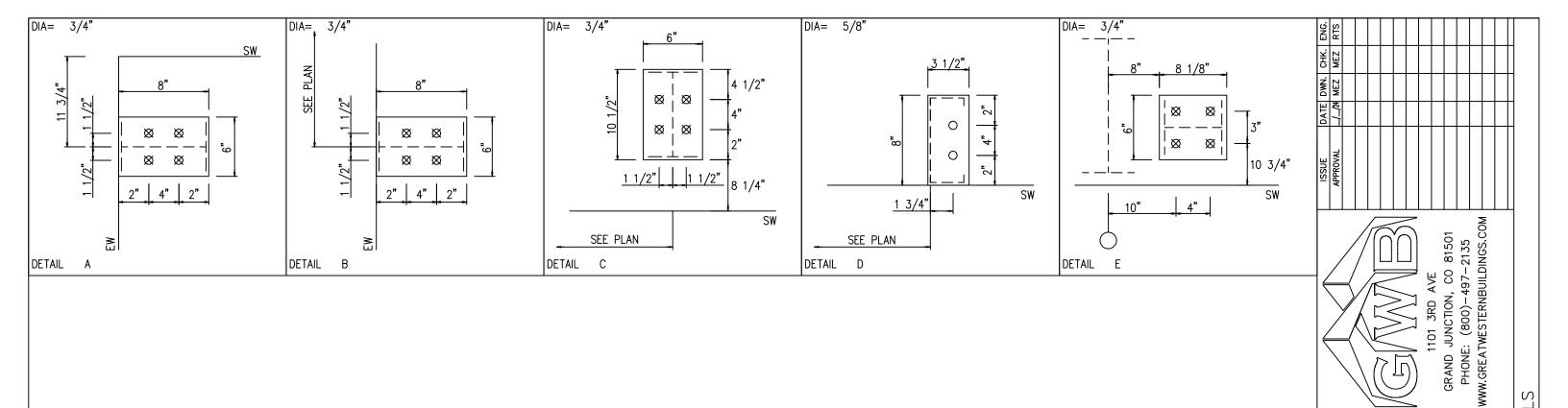
ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

SYSTEMS MANUAL)

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

	BUILDING LOADS	RTS RTS
THIS STRUCTURE HAS BEEN DESIGNED	IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:	MEZ .
DESIGN LOADS: DESIGN CODE / WIND CODE OCCUPANCY / RISK CATEGORY ENCLOSURE ROOF DEAD LOAD (D) (PSF) ROOF COLLATERAL LOAD (C) (PSF) WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) WIND EXPOSURE CATEGORY	: IBC-21 : II-Normal : Enclosed : 2.00 : 2.50 : 115.00 : C	ISSUE DATE DWN. APPROVAL _/_/24 MEZ
WIND ENCLOSURE CLASSIFICATION LIVE LOAD PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF) SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) SNOW EXPOSURE FACTOR, (Ce) SNOW IMPORTANCE FACTOR, (Is)	: 0.18/-0.18 : 23.87/-25.86 : Enclosed : 20.00 : No : 20.00 : 5.00 : 30.0 : 1.00 : 1.00	GRAND JUNCTION, CO 81501 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM
THERMAL FACTOR, (Ct) SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM	:1.00 :1.00 :d :Ss = 1.146 :S1 = 0.443 :Sds = 0.917 :Sd1 = 0.548 :D :STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF)	COVERSHEET
TOTAL DESIGN BASE SHEAR, (V) (KIPS) RESPONSE MODIFICATION FACTORS, (R)	:LONGITUDINAL = 4.74 :TRANSVERSE = 4.77 :RIGID FRAMES = 3.25 :SW X-BRACING = 3.25 Ω = 2.00	16 8720
SEISMIC RESPONSE COEFFICIENTS, (Cs)	:SW WND BENT = 3.25 Ω = 3.00 :RIGID FRAMES = 0.2822 :SW X-BRACING = 0.2822 :SW WND BENT = 0.2822	AME. The state of
ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	:EQUIVALENT LATERAL FORCE PROCEDURE	CUSTOMER NAME: PROJECT NAME: PROJECT DOCATION: PROJECT COUNTY: PROJECT COUNTY: PROJECT COUNTY: PROJECT COUNTY: PROJECT COUNTY: PROJECT COUNTY: SCALE: OUSTOMER GLANL: SCALE: OUSTOMER COUNTY: SCALE: OUSTOMER COUNTY: SCALE: SHEET NUMBER: SHEET NUMBER: SHEET NUMBER: SHEET NUMBER: OUSTOMER COUNTY: SCALE: OUSTOMER COUNTY: STATEMENT COUNTY: SHEET NUMBER: OUSTOMER COUNTY: STATEMENT CO
BUILDING DESCRIPTION: WIDTH (FT) : 35.00 LENGTH (FT) : 60.00 EAVE HEIGHT AT BSW (FT): 15.10 EAVE HEIGHT AT FSW (FT): 18.00 ROOF SLOPE AT BSW : 1.0:12 ROOF SLOPE AT FSW : N/A BAY SPACING (FT) : 3 AT 20.00 COVERING AND TRIMS: ROOF PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : POLAR WHITE TRIM COLORS GABLE/EAVE : POLAR WHITE EAVE GUTTER : N/A WALL PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : RUSTIC RED TRIM COLORS CORNER : POLAR WHITE FRAMED OPENING : POLAR WHITE		THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL
DOWNSPOUTS : N/A BASE : POLAR WHITE WAINSCOT PANELS & TRIMS PANEL TYPE : 26 GA. PBR PANEL COLOR : POLAR WHITE TRIM COLORS : POLAR WHITE INSULATION ROOF INSULATION : N/A WALL INSULATION : N/A		BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.





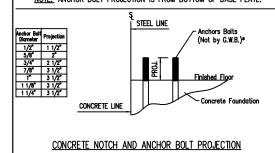
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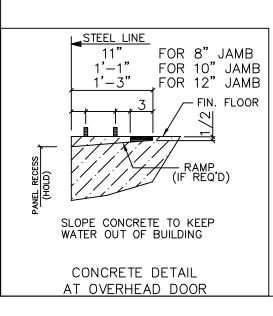
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDINGS WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

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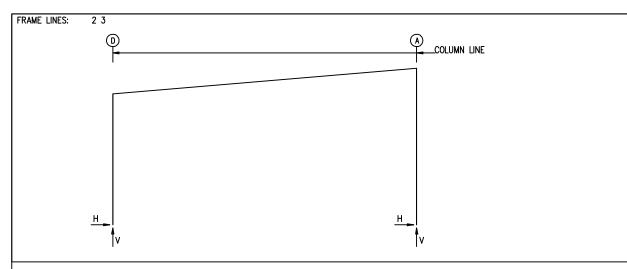
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RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BAS	E PLATE	:S				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolt QTY	i(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	D	2	3.6 3.3	8.6 12.4	3 5	-2.7 0.4	-2.4 -4.2	4	0.750	6.000	10.50	0.375	0.0
2*	Α	4 1	2.2 -3.3	-1.1 12.6	1 6	-3.3 -0.2	12.6 -3.4	4	0.750	6.000	10.50	0.375	0.0
2*	FRAME lin	nes:	2 3										

WIND BENT REACTIONS # Reactions Seismic(k) Vert Loc Line A A Wind(k) Vert Col Line 2 3 Base_Plate(in) Width Length Bolt(in) QTY DIA Horz " Thick 1.6 2.9 2.9 8.125 8.125 0.750 0.750 6.000 6.000 + v

NOTES FOR REACTIONS Building reactions are based on the following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure = 35.00 = 60.00 = 15.10/18.00 = 1.0:12 = 2.00 = 2.50 = 20.00 = 30.00 = 115.00 = IBC-21 = C = Fnclosed Exposure

	Closed/Open Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)	= = =	Enclosed 1.00 1.00 D 1.38
ID	Description		
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18	Dead+Collateral+Snow+Slide_Snow Dead+Collateral+0.75Snow+0.45Wind_ 0.6Dead+0.6Wind_Left2 0.6Dead+0.6Wind_Long1L 0.6Dead+0.6Wind_Long1L 0.6Dead+0.6Wind_Long1R 0.6Dead+0.6Wind_Suction+0.6Wind_Long1Dead+0.6Wind_Suction+0.6Wind_Long1Dead+0.6Wind_Suction+0.6Wind_Long1Dead+0.6Wind_Suction+0.	ong1l ongi ion 2 tion 1 2 1 2	

——Wa		– Col		Reacti		smic -	Panel_Shear - (lb/ft)	
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind Seis	Note
L_EW F_SW	1 A	C,B 2.3	2.0	2.7	1.2	1.5		(a)
R_EW B_SW	A 4 D	C,B 2,3 B,C 3,2	2.0 2.3	2.7 1.6	1.2 3.0	1.5 2.0		(-)
(a)Wind	bent	in bav						

ANCHO	R BOLT S	UMM	ARY	
QTY ————————————————————————————————————	LOCATE JAMB ENDWALL FRAME PORTAL COL	DIA (in) 5/8" 3/4" 3/4" 3/4"	A307 A307 A307 A307 A307	

RIGI	D FRAM	ΜE:	BAS	IC COLUN	IN REACT	IONS (k)								
FRAMI Line 2* 2*	E Column Line D A	Horz 0.2 -0.2	-Dead Vert 1.1 1.2	Coll Horz 0.2 -0.2	ateral- Vert 0.9 0.9	Horz 1.9 –1.9	-Live Vert 7.0 7.0	Horz 2.8 –2.8	-Snow Vert 10.4 10.6	Wind Horz -4.0 -1.3	_Left1- Vert -7.8 -5.6	-Wind_l Horz 2.1 3.5	Right1- Vert -2.7 -5.8		
FRAMI Line 2* 2*	E Column Line D A	Wind Horz -4.7 -0.7	I_Left2- Vert -5.0 -2.9	-Wind_ Horz 1.7 3.9	Right2- Vert 0.1 -3.0	Wind Horz 0.3 0.0	l_Long1- Vert -8.1 -6.9	Wind Horz 0.8 -0.9	l_Long2- Vert -5.9 -4.2	-Seism Horz -1.0 -0.9	ic_Left Vert -0.9 0.9	Seismic Horz 1.0 0.9	_Right Vert 0.9 -0.9		
FRAMI Line 2* 2*	E Column Line D A	-Seism Horz 0.0 0.0	vert Vert -2.0 0.0	-MIN_: Horz 0.5 -0.5	SNOW Vert 1.7 1.8										
2*	FRAME li	nes:	2 3												

2* 2*	D A	0.0		-2.0 0.0	-0.5 -0.5	1. / 1.8												
2*	FRAME	lines:	2	2 3														
FND'	WALL	COLL	JMN:		RA	SIC COLU	IMN I	REACTIO	ONS (k)								
Frm	Col		/ IVII V.	Collat								Wind_Ri	ah +1	Wind_L	~#1)	Win	J_Right2	Wind Press
Line 1	Line	Dead Vert 0.3		Vert 0.1	Li [,] Ve 1.	rt	Sno Ver 1.5	t	Wind_ Horz 0.0	Vei −1.1		Horz 0.0	Vert -0.8	Horz 0.0	Vert −0.7	Horz		Horz -0.8
i 1	D C B	0.5 0.5		0.3 0.3	2. 2.	ŝ	3.8 3.8		-1.5 0.0	-5. -1.	0	0.0 2.0	1.0 -4.5	-1.5 0.0	-4.0 -0.3	0.0 1.9	1.9 -3.3	-1.6 -1.7
1	Α	0.3		0.1	1.0)	1.5		0.0	-1.1		0.0	-0.8	0.0	-0.8	0.0	-0.4	-1.0
Frm	Col	Wind Suct		Wind_L	ong1 Vert	Win	d_Lo	ng2		is_Left	i		Right	Seis Long		MIN_SN		
Line 1	Line D	Horz 0.9		0.0	-1.3	0.0	Z	Vert -0.8	Hor 0.0		Vert 0.0	Horz 0.0	Vert 0.0	Horz 0.0	0	orz .0	Vert 0.2	
1 1 1	C B A	1.8 1.9 1.1		0.0 0.5 0.0	-2.4 -3.8 -1.1	0.0 0.2 0.0		-1.5 -2.1 -0.6	-1.2 0.0 0.0		1.5 1.5 0.0	0.0 1.2 0.0	1.6 -1.6 0.0	0.1 0.1 0.0	0	.0 .0 .0	0.6 0.6 0.3	
Frm	Col		Γ_SL_1		E1PAT_			0.0 [1PAT_			PAT_SI		0.0	0.0	Ū	.0	0.0	
Line 1	Line D	Horz 0.0		ert	Horz 0.0	Vert 0.0	H	lorz).0	Vert 0.7	Ho 0.0	rz	Vert -0.1						
1	C B	0.0 0.0	1.1 -0.	1 .1	0.0 0.0	-0.1 1.0	().0).0	2.1 0.9	0.0 0.0))	0.9 2.1						
1	A	0.0	0.	0	0.0	0.8	(0.0	-0.1	0.0)	0.7						
Frm	Col	Dead		Collat	Li	ve _.	Sno		Wind_			Wind_Ri		Wind_L			J_Right2	Wind Press
Line 4 4	Line A B	Vert 0.3 0.5		Vert 0.1 0.3	V∈ 1.0 2.)	Ver 1.5 3.8		Horz 0.0 -2.0	Ver −0. −4.	8	Horz 0.0 0.0	Vert −1.1 −1.3	Horz 0.0 -1.9	Vert −0.4 −3.3	Hora 0.0 0.0	z Vert -0.8 -0.3	Horz -1.0 -1.7
4	C D	0.5 0.3		0.3 0.1	2. 1.	5	3.8 1.5		0.0 0.0	1.0 -0.)	1.5 0.0	-5.0 -1.1	0.0 0.0	1.9 -0.3	1.5 0.0	-4.0 -0.7	-1.6 -0.8
		Wind												Seis				
Frm Line	Col Line	Suct Horz		Wind_L Horz	Vert	Hor	d_Lo z	Vert	Hor		Vert	Horz	Right Vert	Long Horz	Н	MIN_SN orz	Vert	
4 4 4	A B C	1.1 1.9 1.8		0.0 -0.5 0.0	-1.1 -3.8 -2.4	0.0 -0.2 0.0	!	-0.6 -2.1 -1.5	0.0 -1.2 0.0	_	0.0 1.6 1.6	0.0 0.0 1.2	0.0 1.5 -1.5	0.0 0.1 0.1	0	.0 .0 .0	0.3 0.6 0.6	
4	Ď	0.9		0.0	-1.3	0.0		-0.8	0.0		0.0	0.0	0.0	0.0		.0	0.2	
Frm Line	Col Line	E2PA [*] Horz	T_SL_ ⁻ Ve	1– ert	E2PAT_ Horz	SL_2- Vert		2PAT_ lorz	SL_3- Vert	E2l Ho	PAT_S rz	L_4- Vert						
4	A B	0.0 0.0	0. 1.	0	0.0	0.0 -0.1	().0).0	0.7 2.1	0.0)	-0.1 0.9						
4 4	C D	0.0 0.0	-0. 0.		0.0 0.0	1.1 0.8).0).0	0.9 -0.1	0.0 0.0		2.1 0.7						
END'	WALL	COLU	JMN:		MAX	IMUM RE	ACTI	ONS, A	NCHOR	BOLTS,	& BA	SE PLA	TES					
					Column _.	_Reaction V L	ns(k)										
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4	,	4	14 7_	0.0 0.7	7 -	2.0 -0.5	8	0.7 -0.6	3 -	-0.5 -0.5	4	0.750	6.000	8.000	0.3	75	0.0	
4		3	15 10	0.0 1.1	-	2.0 -2.4	7 8	0.7 -1.0) –	-0.5 -1.9	4	0.750	6.000	8.000	0.3	75	0.0	
4	,	_	16	0.0)	4.8	10	1.1	-	-2.4	4	0.750	6 000				0.0	

-1.1 -2.7

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4 0.750 6.000 8.000 0.375 0.0

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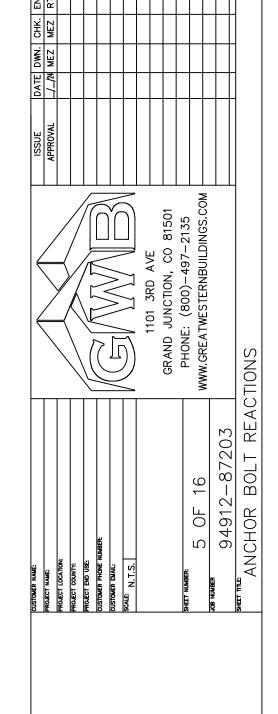
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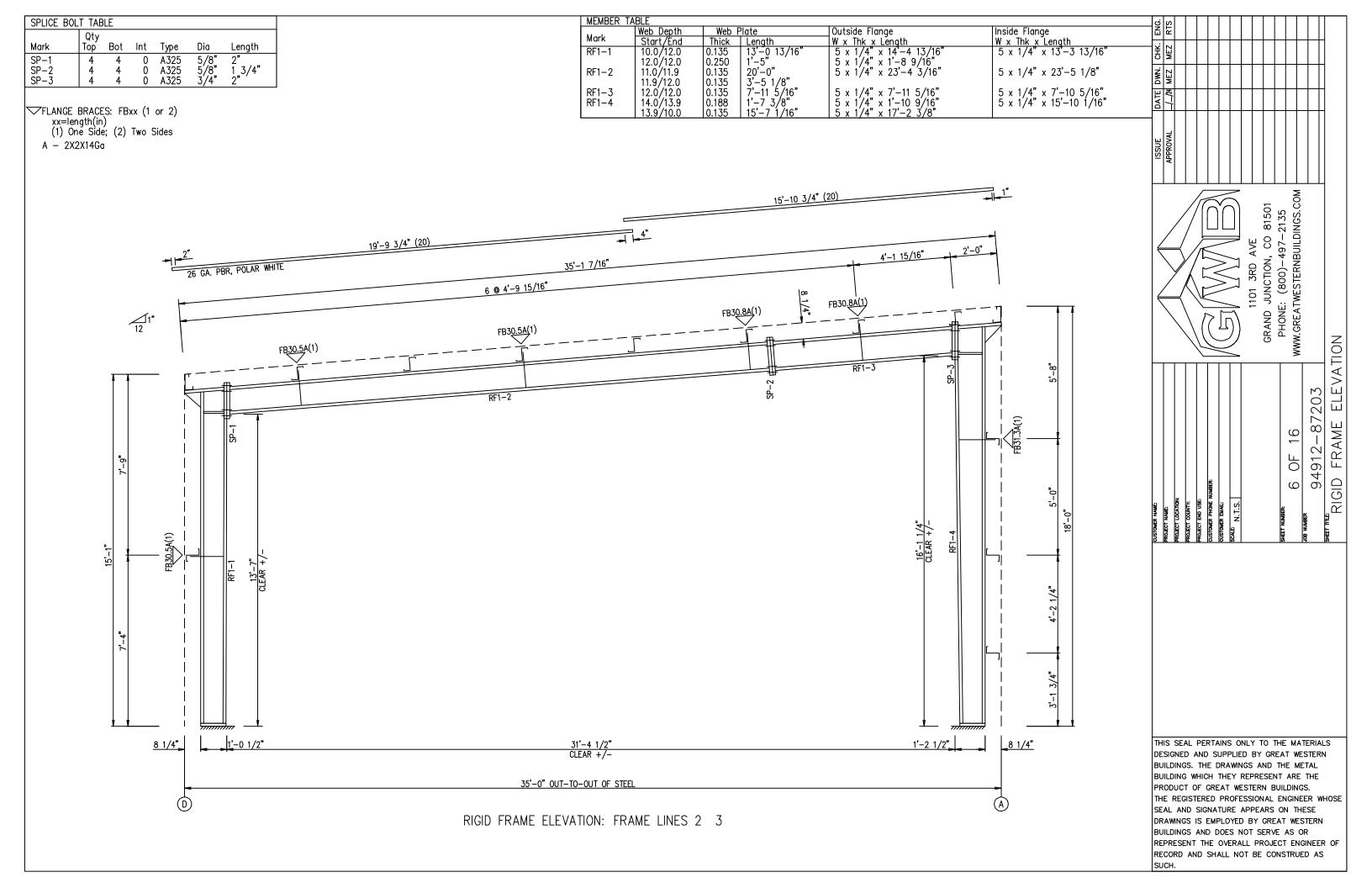
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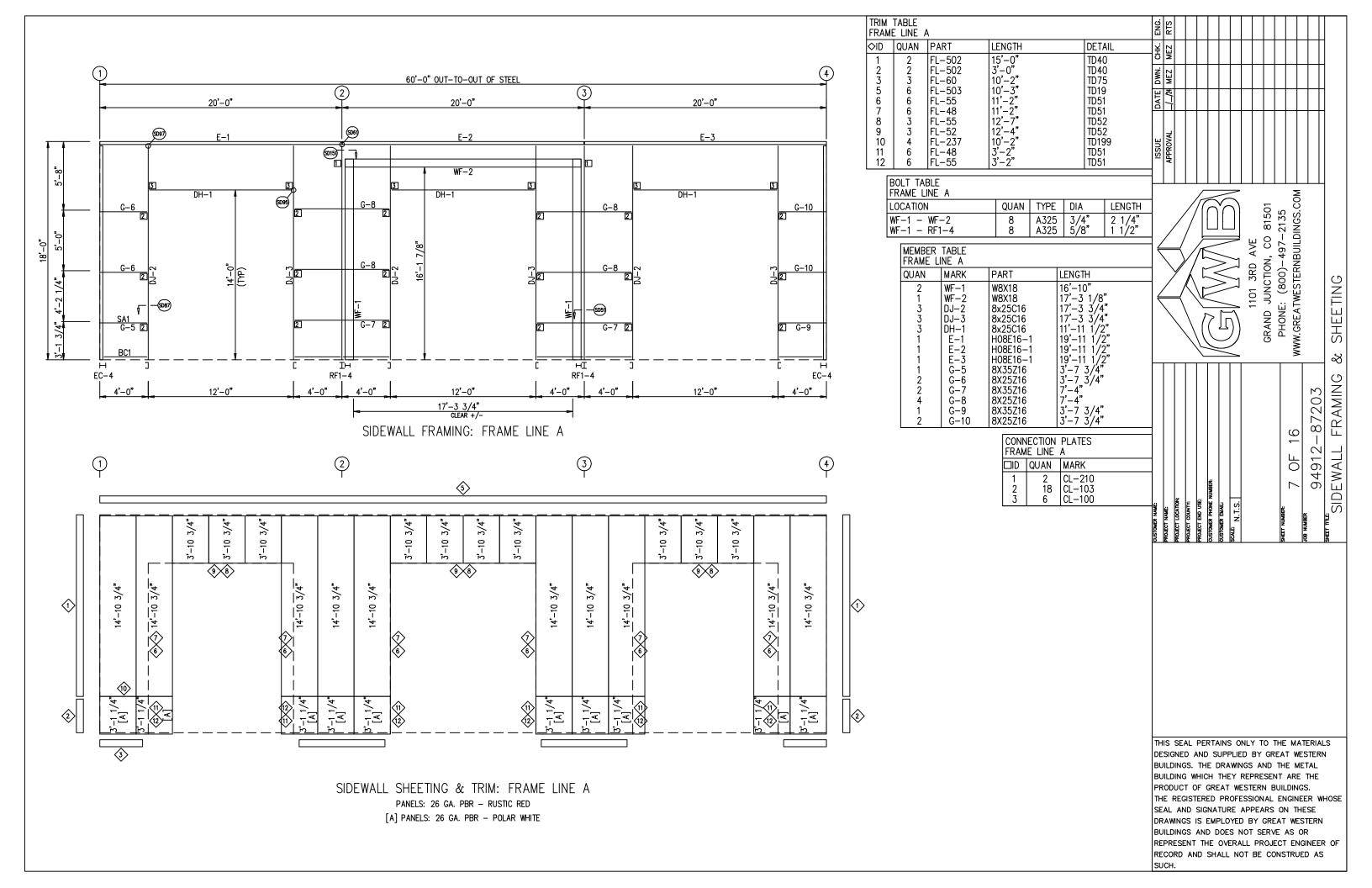
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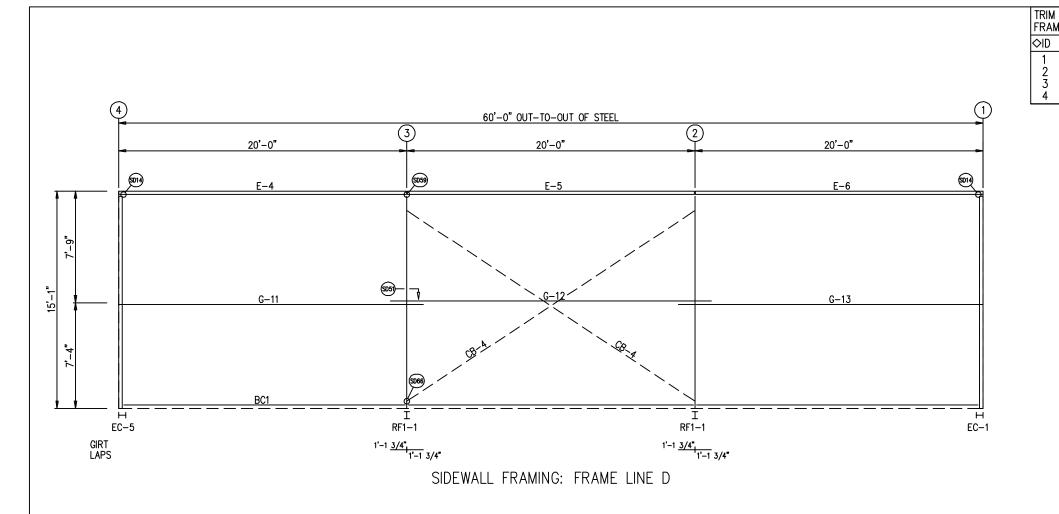
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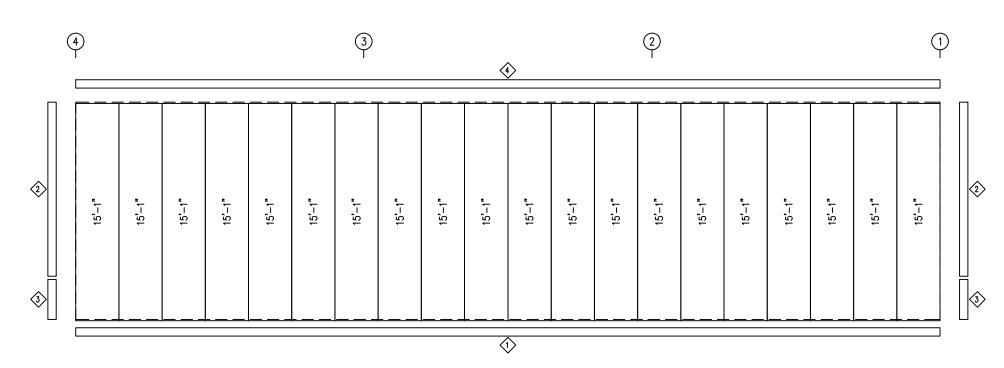


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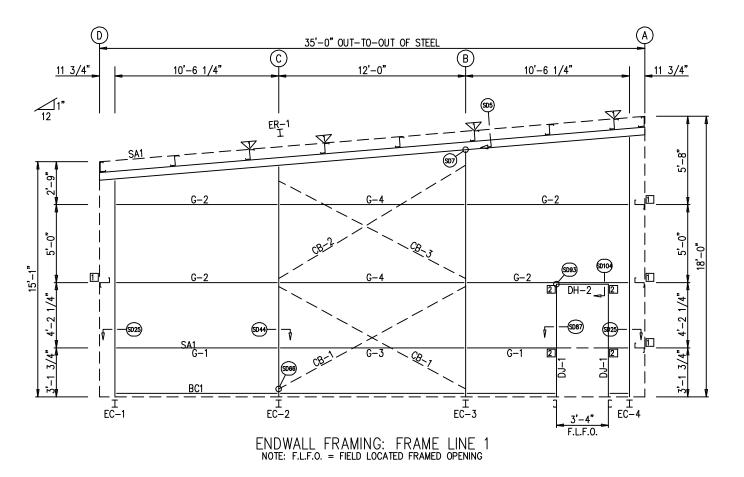


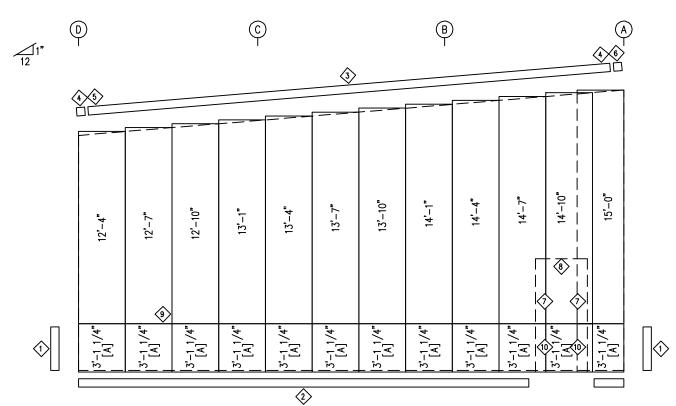


SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 GA. PBR - RUSTIC RED

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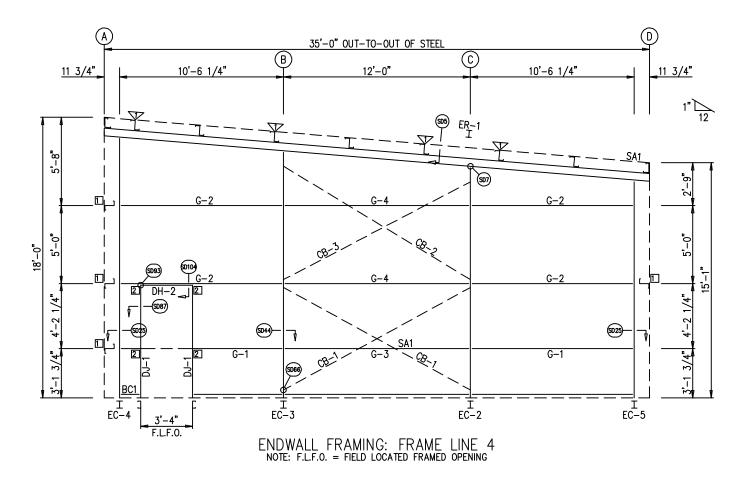


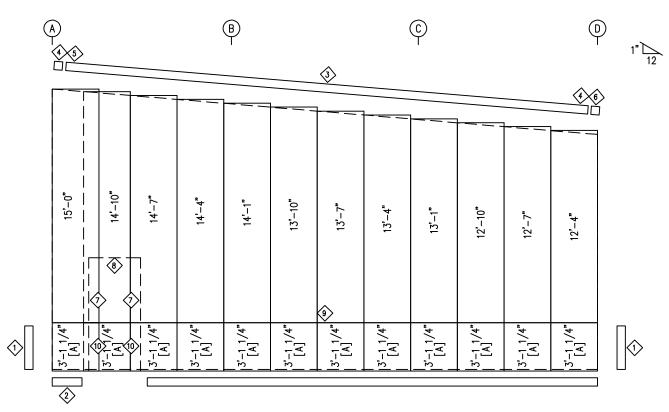
ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 GA. PBR - RUSTIC RED

[A] PANELS: 26 GA. PBR - POLAR WHITE

TRIM	TABLE					ıن	Ŋ			Т							
FRAM >ID	E LINE 1		LENOTH	DETAIL		_	RTS			_	_	\perp	_				
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2	2 4 4 2 1	ובו בה	3'-0" 10'-2" 9'-2" 7 3/4" 5 5/8" 5 5/8" 4'-4" 3'-8" 10'-2" 3'-2"	TD75		DWN.	MEZ			T							
2 3 4 5 6 7 8 9	2	FL-50 FL-15 FL-601B FL-600L FL-600R FL-48 FL-52 FL-237	9 -2 7 3/4"	TD36 TD85					\dashv	4	+	+	-	++	++		
5	1	FL-600L	5 5/8"	l TD12		DATE	1_/24										
ь 7	1 2	FL-600R FL-48	5,5/8 4'-4"	TD12 TD51													
8	2	FL-52	3'-8"	TD51 TD52 TD199		ļ.,,	/AL										
10	4 2	FL=237 FL=48	3'-2"	TD51		ISSUE	APPROVAL										
E	BOLT TAE	BLE		•		-	ΑF										
F	RAME LI	INE 1	OLIANI TYDI	· IDIA II	CNOTH	_			Ш	\neq	_	-			┰		
_	OCATION COLUMNS	I S/RAFTER	QUAN TYPE		ENGTH I 1/2"			,	//		$\sqrt{}$	7)		100	PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM		
_	мемві	ER TABLE	<u> </u>				/	/	/	ļL		╝		81501	PHONE: (800)-497-2135 GREATWESTERNBUILDINGS.		
	FRAME QUAN	E LINE 1 MARK	PART	LENGTH		l €	_	${\prec}'$				=	1101 3RD AVE	GRAND JUNCTION, CO	-76: JILD		
	QUAIN 1	FC-1	W8X10	13'-9 1/16"		`	\setminus		//	^ا ~	\geq	\supset	<u> </u>	Š.)–4 3NB		
	1 1	EC-2	W8X10	14'-7 9/16"				y 1	Λ	١,	\leq		38	CJ	300 31E 31E		0
	1 1	EC-3	W8X10 W8X10	16'-6 1/16"	_	€	_	⋞	/	Æ	<		101	Ŋ,	<u>ئۆ</u> ئ		
	1 1	EC-1 EC-2 EC-3 EC-4 ER-1 DH-2	W8X10 GH-1	35'-0 15/16 3'-4"	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		\setminus	` \	\	7	> [$\widehat{\overline{\gamma}}$	_	9			
	2	100-1	8x25C16	7'-0"					/	(ĬΠ	((ا		RA	PH(R.GR		SHEETING
	4	G-1 G-2 G-3	8X25Z16	9-10-1/4"					/	1	$\stackrel{\smile}{st}$	2		G	⋛		
	1 2 2 4 1 2 2	G-3 G-4	8X25Z16 8X35Z16 8X35Z16 8X35Z16 8X35Z16 8X35Z16	11'-4" 11'-4"		_				$\overline{}$	Т						ઝ
	2	CB-1	טטכטעאן	14'-0 3/4"													9
		CB-2 CB-3	RD0500 RD0500	13'-9 1/16" 14'-7 9/16" 15'-7 9/16" 16'-6 1/16" 35'-0 15/16 3'-4" 7'-0" 9'-10 1/4" 9'-10 1/4" 11'-4" 11'-4" 14'-0 3/4" 14'-6 1/4"												87203	FRAMING
			CONNECTION													72	A
			FRAME LINE	1											16	∞	FR
			□ID QUAN	MARK CL-5												7	ابــا
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			FLAN	GE BRACE TAI	BLE					Z.					၂ တ	6	\sim
			FRAM	E LINE 1		ij	ü	NOT!	Σ. L	USE: ONF NIMBER:	AIL:	Ŋ.			liz l		ENDWALL
			∇ID 1	QUAN MARI 4 FB29	1.3	CUSTOMER NAME	PROJECT NAME	PROJECT LOCA	PROJECT COUR	PROJECT END	CUSTOMER EM/	Z.			SHEET NUMBER	JOB NUMBER	SHEET TILE:
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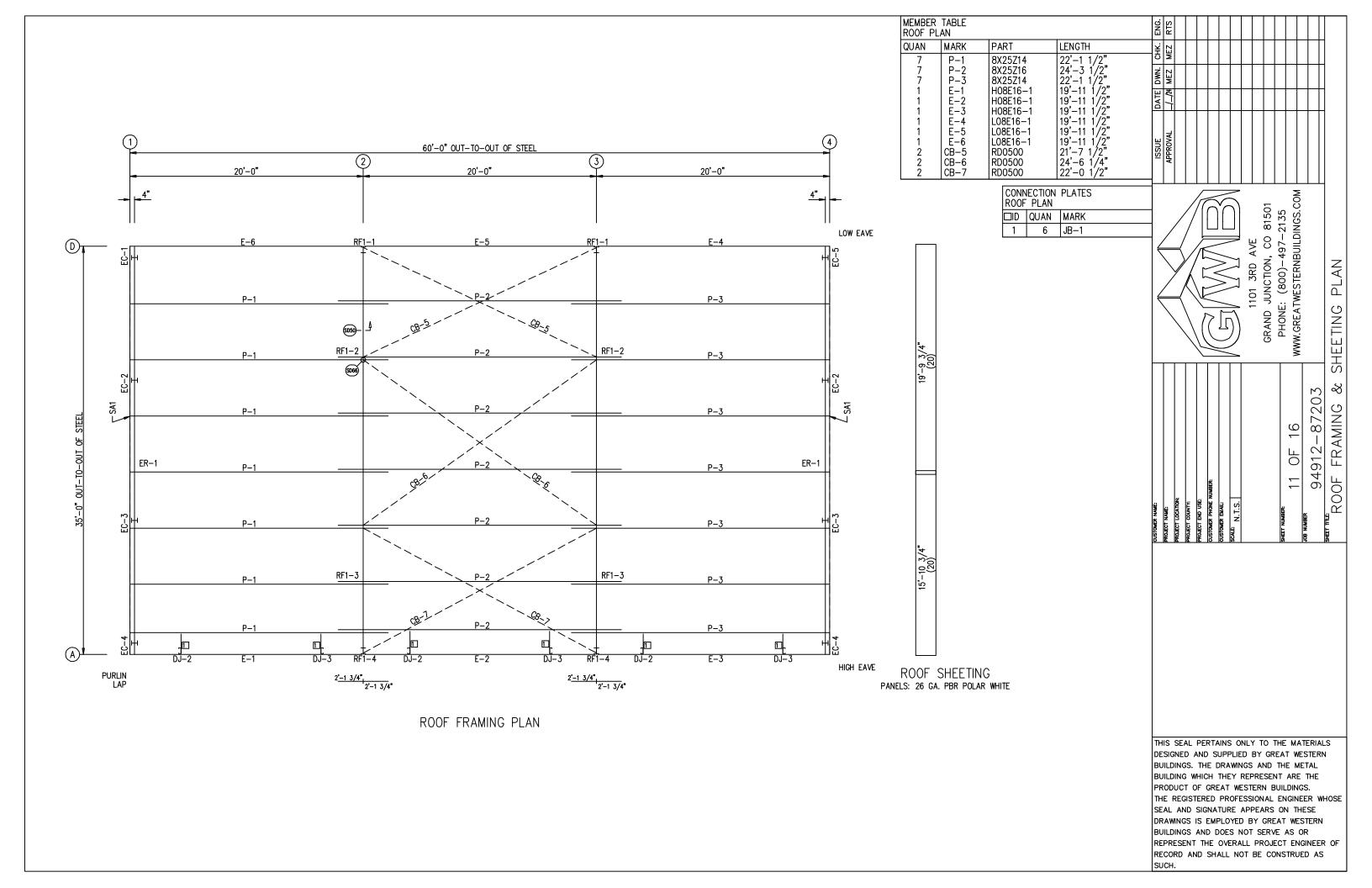


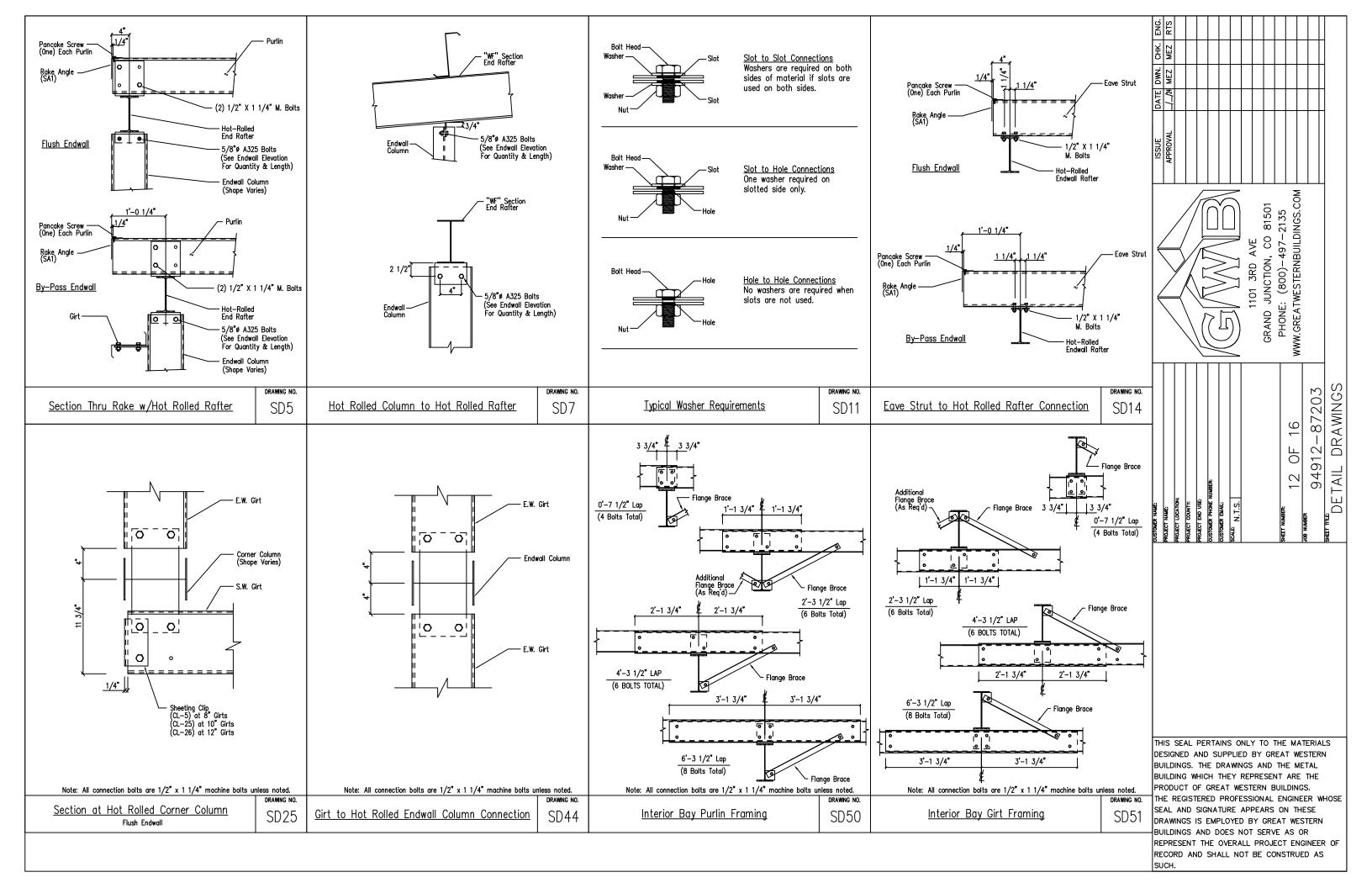
ENDWALL SHEETING & TRIM: FRAME LINE 4

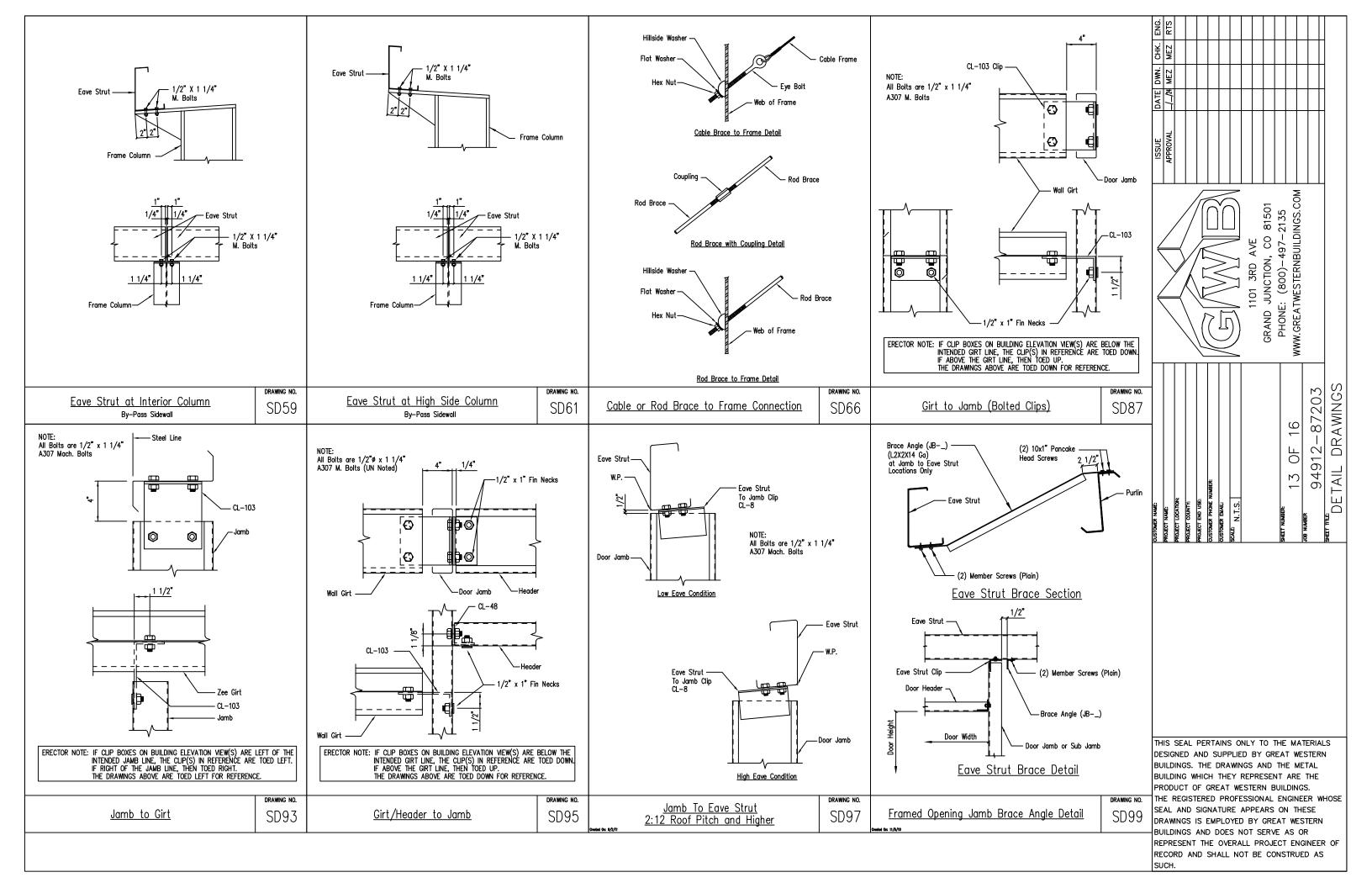
PANELS: 26 GA. PBR - RUSTIC RED

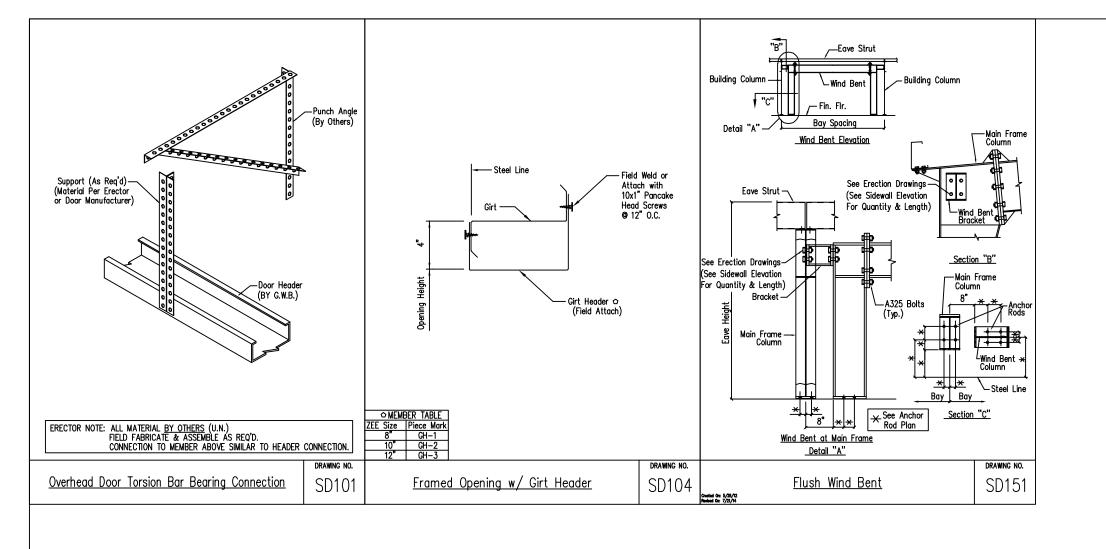
[A] PANELS: 26 GA. PBR - POLAR WHITE

TRIM	TABLE	•			Š.	RTS							П	T	П	
FRAM ⇔ID	E LINE 4	PART	LENGTH	DETAIL	동 - E	-	-	Н		+	+	+	\forall	+	++	
1	2	FL-502	3'-0" 10'-2" 9'-2" 7 3/4" 5 5/8" 5 5/8" 4'-4" 3'-8" 10'-2" 3'-2"	TD40	_	+-	+-			1			\sqcup	\perp	\perp	
2 3 4 5 6 7	2 3 4 2	FL-60 FL-15 FL-601B	9'-2"	TD75 TD36	DW.	MEZ										
4 5	2	FL-601B	7 3/4"	TD85 TD12	DATE	124		П								
6	1 1	FL-600L FL-600R	5, 5/8"	l TD12			-	Н	+	+	+		++	++	++	
/ 8	2	FL-48 FL-52 FL-237	4-4 3'-8"	TD51 TD52		یا										
8 9 10	4 2	FL-237 FL-48	10'-2"	TD199 TD51	ISSUE	APPROVAL										
	BOLT TAI			1001	վջ	APF										
_F	RAME L	INE 4	OLIAN TYPE	DIA LENGTH	╀		_		\perp	<u> </u>	$\stackrel{\sqcup}{=}$		Ш	⊥⊥ ≥		
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	MEMB	ER TABLE E LINE 4				/	/					1.1	GRAND JUNCTION, CO 81501	PHONE: (800)-497-2135 /.GREATWESTERNBUILDINGS.		
	QUAN	MARK	PART	LENGTH	┥╡	_	\dashv	/	<u>/</u> [\leq	<u> </u>	1101 3RD AVE	č,	497 BUIL		
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	1 1	EC-5	W8X10 W8X10	13'-9 1/16" 35'-0 15/16"	€	\	\dashv	' \	لإ	\leq	_	110	\exists			
		DH-2	GH-1	3'-4"		\	\		17	ا ﴿	\vec{a}		N N	S A		
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	1 2 2 4 1 2 2	G-4 CB-1	8X25Z16 RD0500	11'-4"												
	1 1	CB-2 CB-3	RD0500 RD0500	14'-7 9/16" 15'-7 9/16" 16'-6 1/16" 13'-9 1/16" 35'-0 15/16" 3'-4" 7'-0" 9'-10 1/4" 9'-10 1/4" 11'-4" 11'-4" 14'-0 3/4" 14'-6 1/4")3	FRAMING
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