

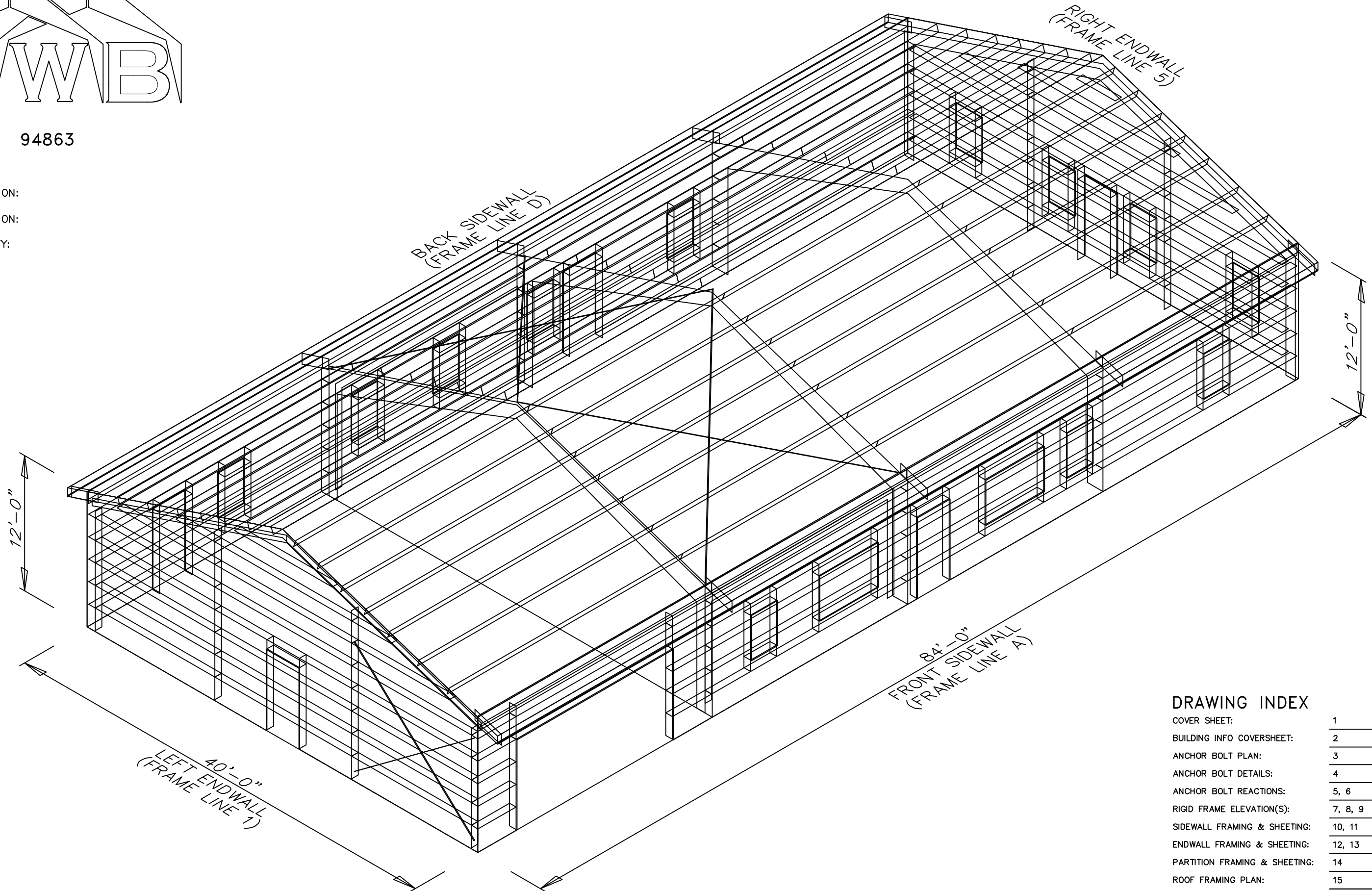
JOB NUMBER: 94863

PROJECT NAME:

PROJECT LOCATION:

PROJECT LOCATION:

PROJECT COUNTY:



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GENERAL NOTES

- FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1 AND D1.3".
- 1.2 MATERIALS
- | | <u>ASTM DESIGNATION</u> | <u>MIN. YIELD STRENGTH</u> |
|-------------------------------------|-------------------------|----------------------------|
| HOT ROLLED STEEL SHAPES (W. & C) | A572 | Fy = 50 KSI |
| HOT ROLLED STEEL ANGLES (L) | A36 | Fy = 36 KSI |
| STEEL PIPES | A500 | Fy = 42 KSI |
| STRUCTURAL TUBING | A500 | Fy = 42 KSI |
| STRUCTURAL STEEL WEB PLATE | A572/A1011 | Fy = 50 KSI |
| STRUCTURAL STEEL FLANGE PLATES/BARS | A529/A572 | Fy = 55 KSI |
| COLD FORMED LIGHT GAGE | A653/A1011 | Fy = 55 KSI |
| ROOF & WALL SHEETS | A792/A653 | Fy = 50, 80 KSI |
| CABLE BRACE | A475 - TYPE 1 | EXTRA HIGH STRENGTH |
| ROD BRACE | A36 | Fy = 36 KSI |
- MIN. TENSILE STRENGTH
- | | | |
|---------------------------------------|----------------|--------------|
| MACHINE BOLTS & NUTS | A307 | Fu = 60 KSI |
| HIGH STRENGTH BOLTS (1"ø & LESS) | A325-TYPE 1 | Fu = 120 KSI |
| HIGH STRENGTH BOLTS (>1"ø to 1 1/2"ø) | A325-TYPE 1 | Fu = 105 KSI |
| ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) | A36/A307/F1554 | Fu = 60 KSI |
- 1.3 PRIMER
- SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
- (AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
- NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT DOCUMENTS.
- 1.4 GALVANIZED OR SPECIAL COATINGS:
- SEE CONTRACT DOCUMENTS
- 1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT:
- A) ENDWALL RAFTER SPLICE - 5/8"ø x 0'-1 3/4" A325-N
 - B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION)
 - C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION
 - D) FLANGE BRACE CONNECTIONS - 1/2"ø x 0'-1 1/4" A325
- NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING
- 1.6 A325 BOLT TIGHTENING REQUIREMENTS
- ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
- STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.
- BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING
- 1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
- INSIDE - UNDER ROOF PANELS & BASE OF WALL PANELS
 - OUTSIDE - BETWEEN ROOF PANELS & RIDGE CAP
 - BETWEEN WALL PANELS & EAVE/GABLE TRIM
- 1.8 ERECTION NOTE:
- ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.
- 1.9 ERECTION AND UNLOADING NOT BY G.W.B.
- 1.10 SHORTAGES
- ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.
- 1.11 CORRECTIONS OF ERRORS AND REPAIRS (MWMA 6.10)
- CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING SUCH.
- 2.2 METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.) STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE, METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- 2.3 IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)
- 2.4 APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- 2.5 ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

- THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USER CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST EDITION)
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USER CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE CONCRETE. UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USER CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USER CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USER CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12.12 **WARNING** IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.
- 2.13 **SAFETY COMMITMENT** METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 **ROOF DRAINAGE SYSTEMS** (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SEE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002
EDITION, SECTION A8.4, PAGE XI-A8-2

BUILDING LOADS

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

DESIGN CODE / WIND CODE	:IBC-21
OCCUPANCY / RISK CATEGORY	:II-Normal
ENCLOSURE	:Enclosed
ROOF DEAD LOAD (D) (PSF)	:3.45
ROOF COLLATERAL LOAD (C) (PSF)	:22.00
WIND LOAD	
ULTIMATE WIND SPEED, (VULT) (MPH)	:115.00
WIND EXPOSURE CATEGORY	:C
INTERNAL PRESSURE COEFFICIENT, (GCpi)	:0.18/-0.18
WALL PANEL DESIGN WIND PRESSURE (PSF)	:28.53/-30.94
WIND ENCLOSURE CLASSIFICATION	:Enclosed

LIVE LOAD

PRIMARY FRAMING (PSF)	: 20.00
TRIB. AREA REDUCTION	: No
SECONDARY FRAMING (PSF)	: 20.00

SNOW LOAD

GROUND SNOW LOAD, (Pg) (PSF)	: 80.00
ROOF SNOW LOAD, (Pf) (PSF)	: 80.00
SNOW EXPOSURE FACTOR, (Ce)	: 1.00
SNOW IMPORTANCE FACTOR, (Is)	: 1.00
THERMAL FACTOR, (Ct)	: 1.20

SEISMIC LOAD

SEISMIC IMPORTANCE FACTOR, (Ie)	: 1.00	
SITE CLASSIFICATION	: D	
SPECTRAL RESPONSE ACCELERATION	: Ss = 0.303	: S1 = 0.083
SPECTRAL RESPONSE COEFFICIENTS	: Sds = 0.315	: Sd1 = 0.133
SEISMIC DESIGN CATEGORY	: B	
BASIC SEISMIC FORCE RESISTING SYSTEM	: STEEL SYSTEM NOT SPECIFICALLY	

TOTAL DESIGN BASE SHEAR, (V) (KIPS)

RESPONSE MODIFICATION FACTORS, (R)

SEISMIC RESPONSE COEFFICIENTS, (C_s)

ANALYSIS PROCEDURE USED :EQUIVALENT LATERAL FORCE PROCEDURE
OTHER LOADS/REQUIREMENTS

BUILDING DESCRIPTION:

WIDTH (FT) : 40.00
 LENGTH (FT) : 84.00
 EAVE HEIGHT AT BSW (FT) : 12.00
 EAVE HEIGHT AT FSW (FT) : 12.00
 ROOF SLOPE AT BSW : 4.0:12
 ROOF SLOPE AT FSW : 4.0:12
 BAY SPACING (FT) : 1 AT 24, 3 AT 20

COVERING AND TRIMS:

ROOF PANELS & TRIMS

PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: REGAL RED (KYNAR)
TRIM COLORS	
GABLE/EAVE	: SOLAR WHITE
EAVE GUTTER	: N/A

WALL PANELS & TRIMS

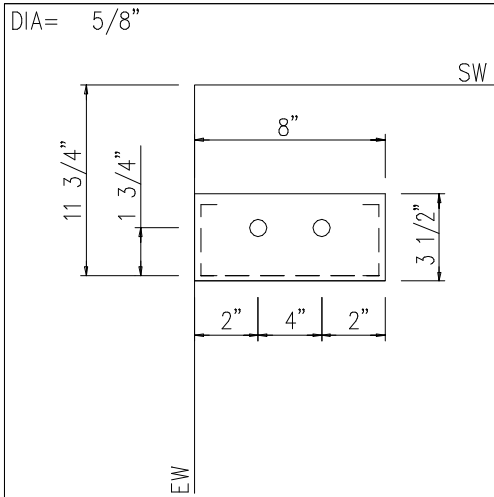
PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: ASH GRAY
TRIM COLORS	
CORNER	: SOLAR WHITE
FRAMED OPENING	: SOLAR WHITE
DOWNSPOUTS	: N/A
BASE	: ASH GRAY

INSULATION

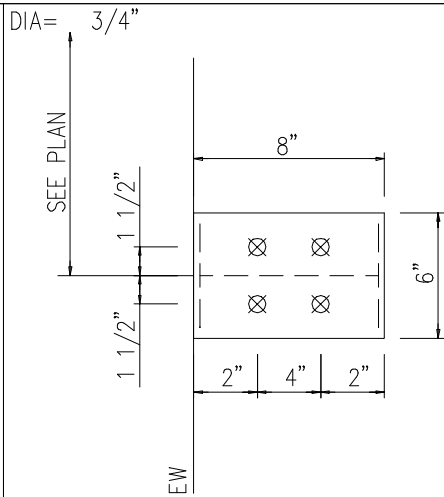
ROOF INSULATION : N/A
WALL INSULATION : N/A

CUSTOMER NAME:	 <p>1101 3RD AVE GRAND JUNCTION, CO 81501 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM</p>	ISSUE	DATE	DWN.	CHK.	ENG.
PROJECT NAME:		APPROVAL	04/08/74	MEZ	MEZ	RTS
PROJECT LOCATION:		PERMIT	07/16/74	SB	CAF	RTS
PROJECT COUNTY:		ERECTION	08/13/74	PKD	PKD	RTS
PROJECT END USE:		REVISED ERECTION	07/09/75	PKD	PKD	RTS
CUSTOMER PHONE NUMBER:						
CUSTOMER EMAIL:						
SCALE: N.T.S.						
SHEET NUMBER:	2 OF 21					
JOB NUMBER	94863					
SHEET TITLE:	BUILDING INFO COVER SHEET					

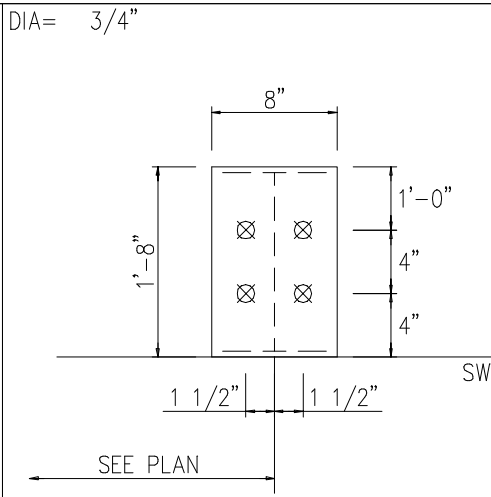
THIS SEAL PERTAINS ONLY TO THE MATERIALS
DESIGNED AND SUPPLIED BY GREAT WESTERN
BUILDINGS. THE DRAWINGS AND THE METAL
BUILDING WHICH THEY REPRESENT ARE THE
PRODUCT OF GREAT WESTERN BUILDINGS.
THE REGISTERED PROFESSIONAL ENGINEER WHOSE
SEAL AND SIGNATURE APPEARS ON THESE
DRAWINGS IS EMPLOYED BY GREAT WESTERN
BUILDINGS AND DOES NOT SERVE AS OR
REPRESENT THE OVERALL PROJECT ENGINEER OF
RECORD AND SHALL NOT BE CONSTRUED AS
SUCH.



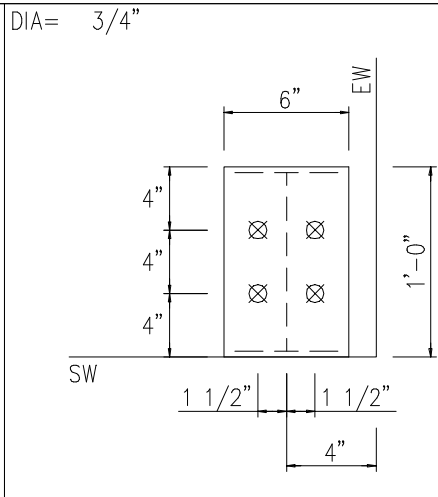
DETAIL A



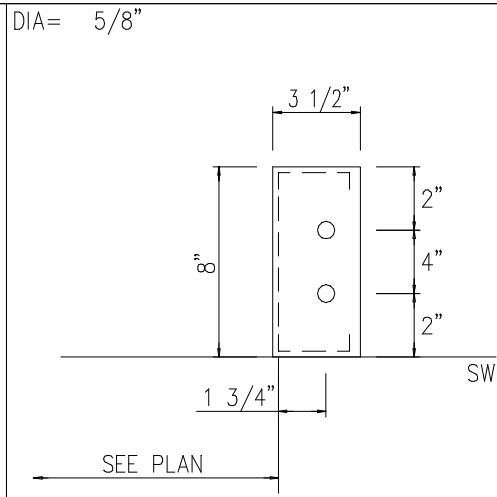
DETAIL B



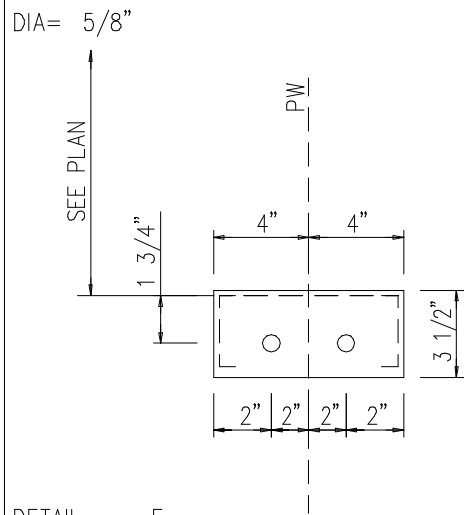
DETAIL C



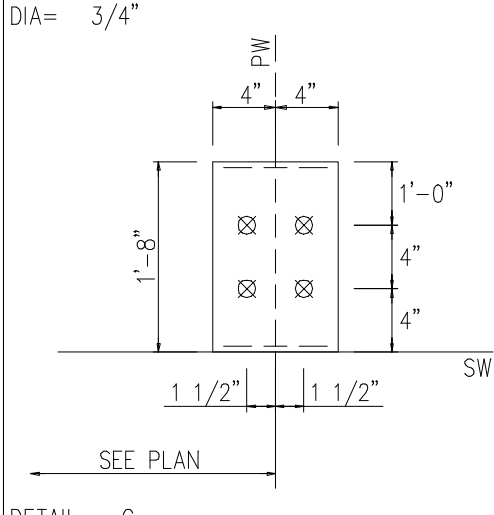
DETAIL D



DETAIL E



DETAIL



DETAIL G

NOTE:

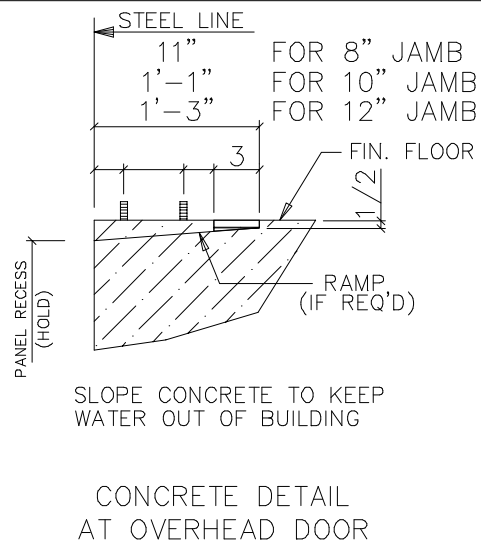
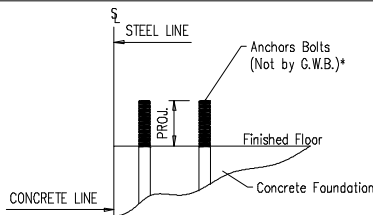
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	2 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/8"	3 1/2"
1 1/4"	3 1/2"

[illegible]

1101 3RD AVE
GRAND JUNCTION, CO 81501
PHONE: (800)-497-2135
WWW.GREATWESTERNBUILDINGS.COM

CUSTOMER NAME:	
PROJECT NAME:	
PROJECT LOCATION:	
PROJECT COUNTY:	
PROJECT END USE:	
CUSTOMER PHONE NUMBER:	
CUSTOMER EMAIL:	
SCALE:	N.T.S.
SHEET NUMBER:	4 OF 21
JOB NUMBER	94863

ENDWALL COLUMN:			BASIC COLUMN REACTIONS (k)												
Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Wind Press	
1	D	0.4	1.5	1.8	7.2	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	
1	C	1.0	4.2	3.6	14.2	0.0	-2.7	0.0	-2.3	0.0	-1.8	0.0	-1.4	-0.8	
1	B	1.0	4.2	3.6	14.2	0.0	-4.4	0.0	-2.5	0.0	-3.2	0.0	-1.3	-2.2	
1	A	0.4	1.5	1.8	7.2	-1.6	-5.1	0.0	-2.4	-1.6	-3.9	0.0	-1.2	-2.2	
						0.0	0.2	1.6	-4.8	0.0	1.2	1.6	-3.8	-0.8	
Frm Line	Col Line	Wind Suct	Wind_Long1		Wind_Long2		Seis_Left		Seis_Right		Seis Long	E1UNB_SL_L-			
1	D	Horz	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Horz	Vert		
1	D	0.9	0.0	-2.4	0.0	-1.6	0.0	0.2	0.0	-0.2	0.0	0.0	7.2		
1	C	2.4	0.0	-3.8	0.0	-2.3	0.0	-0.3	0.0	0.3	0.1	0.0	16.8		
1	B	2.4	0.0	-1.3	-0.9	-5.2	-2.9	-4.2	0.0	3.3	0.1	0.0	5.8		
1	A	0.9	0.9	-2.6	0.0	-1.0	0.0	4.3	2.9	-3.3	0.0	0.0	1.9		
Frm Line	Col Line	E1UNB_SL_R-													
1	D	Horz	Vert	0.0	1.9										
1	C	0.0	5.8												
1	B	0.0	16.8												
1	A	0.0	7.1												

ENDWALL COLUMN:		MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES											
Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)		Thick	Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	QTY	DIA	Width	Length		
1	D	13	0.6	-1.4	14	-0.5	-1.2	2	0.625	3.500	8.000	0.250	0.0
		1	0.0	9.0	13	0.6	-1.4						
1	C	13	1.4	-2.0	14	-1.3	-1.7	4	0.750	6.000	8.000	0.375	0.0
		15	0.0	22.0	13	1.4	-2.0						
1	B	16	1.4	-2.5	17	-1.3	-2.5	4	0.750	6.000	8.000	0.375	0.0
		18	0.0	22.0	16	1.4	-2.5						
1	A	19	0.6	-2.6	14	-0.5	-1.4	2	0.625	3.500	8.000	0.250	0.0
		1	0.0	9.0	19	0.6	-2.6						

ISSUE		DATE	DWN.	CHK.	ENG.
APPROVAL	MEZ	04/08/24	MEZ	RTS	RTS
	CAF	07/26/24	SB	CAF	RTS
	PKD	08/13/24	PKD	PKD	RTS
ERECTION					



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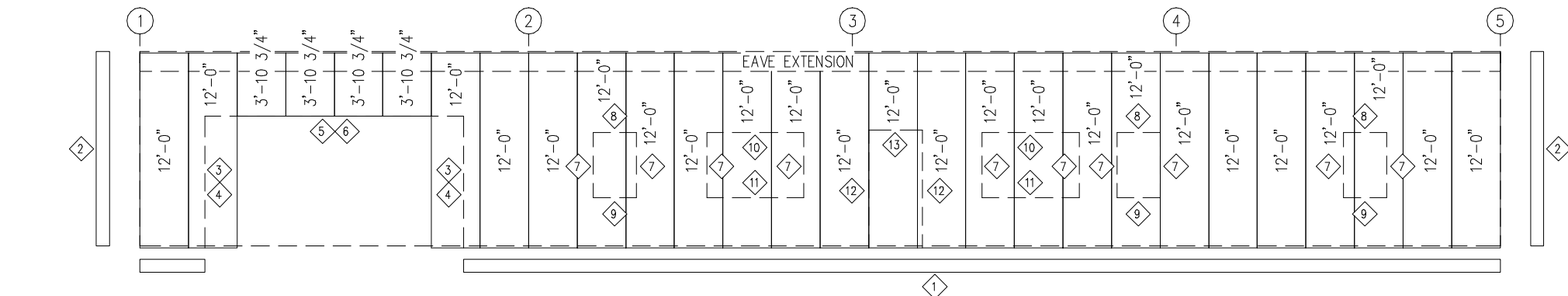
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SHEET TITLE:	ANCHOR BOLT REACTIONS

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NOTE: F.L.F.O. = FIELD LOCATED FRAME OPENING

SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - ASH GRAY

TRIM TABLE				
FRAME LINE A				
◇ID	QUAN	PART	LENGTH	DETAIL
1	7	FL-60	10'-2"	TD75
2	2	FL-10	12'-0"	TD40
3	2	FL-55	8'-2"	TD51
4	2	FL-48	8'-2"	TD51
5	1	FL-55	16'-7"	TD52
6	1	FL-52	16'-4"	TD52
7	10	FL-48	4'-0"	TD51
8	3	FL-52	3'-0"	TD52
9	3	FL-50	3'-0"	TD52
10	2	FL-52	6'-4"	TD52
11	2	FL-50	6'-4"	TD52
12	2	FL-48	7'-4"	TD51
13	1	FL-52	3'-8"	TD52

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	1"	3 1/4"
WF-1 - RF1-4	6	A325	5/8"	1 1/2"
WF-1 - RF2-1	6	A325	5/8"	1 1/2"

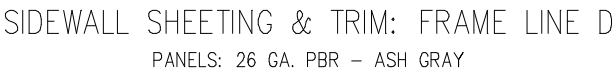
MEMBER TABLE			
FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	WF-1	W10X15	10'-2"
1	WF-2	W10X22	18'-2 1/4"
2	DJ-1	8x25C16	7'-8"
1	DJ-2	8x25C16	11'-1 3/4"
1	DJ-3	8x25C16	11'-1 3/4"
10	DJ-4	8x25C16	5'-4"
1	DH-1	8x25C16	3'-3 1/2"
1	DH-2	8x25C16	15'-11 1/2"
3	DH-3	8x25C16	2'-7 1/2"
2	DH-4	8x25C16	5'-11 1/2"
3	DS-1	8x25C16	2'-7 1/2"
2	DS-2	8x25C16	5'-11 1/2"
1	E-1	L12E16-4	22'-11 13/16"
2	E-2	L12E16-4	19'-3 13/16"
1	E-3	L12E16-4	18'-11 13/16"
5	G-10	8X25Z16	3'-7 11/16"
1	G-11	8X25Z16	15'-11 1/2"
5	G-12	8X25Z16	3'-2 13/16"
4	G-13	8X25Z16	18'-3 3/16"
4	G-15	8X25Z16	19'-1 11/16"
1	G-16	8X25Z14	19'-1 11/16"
5	G-17	8X25Z16	18'-10 3/4"

CONNECTION PLATES	
FRAME LINE	A
□ID	QUAN MARK
1	58 CL-103
2	26 CL-100
3	10 4" ZEE - 9"
4	1 4" ZEE - 18'-0"

[illegible]

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CONNECTION		PLATES
FRAME	LINE	D
□ID	QUAN	MARK
1	56	CL-103
2	24	CL-100
3	10	4" ZEE - 9"
4	1	4" ZEE - 18'-0"

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PROJECT NAME:	
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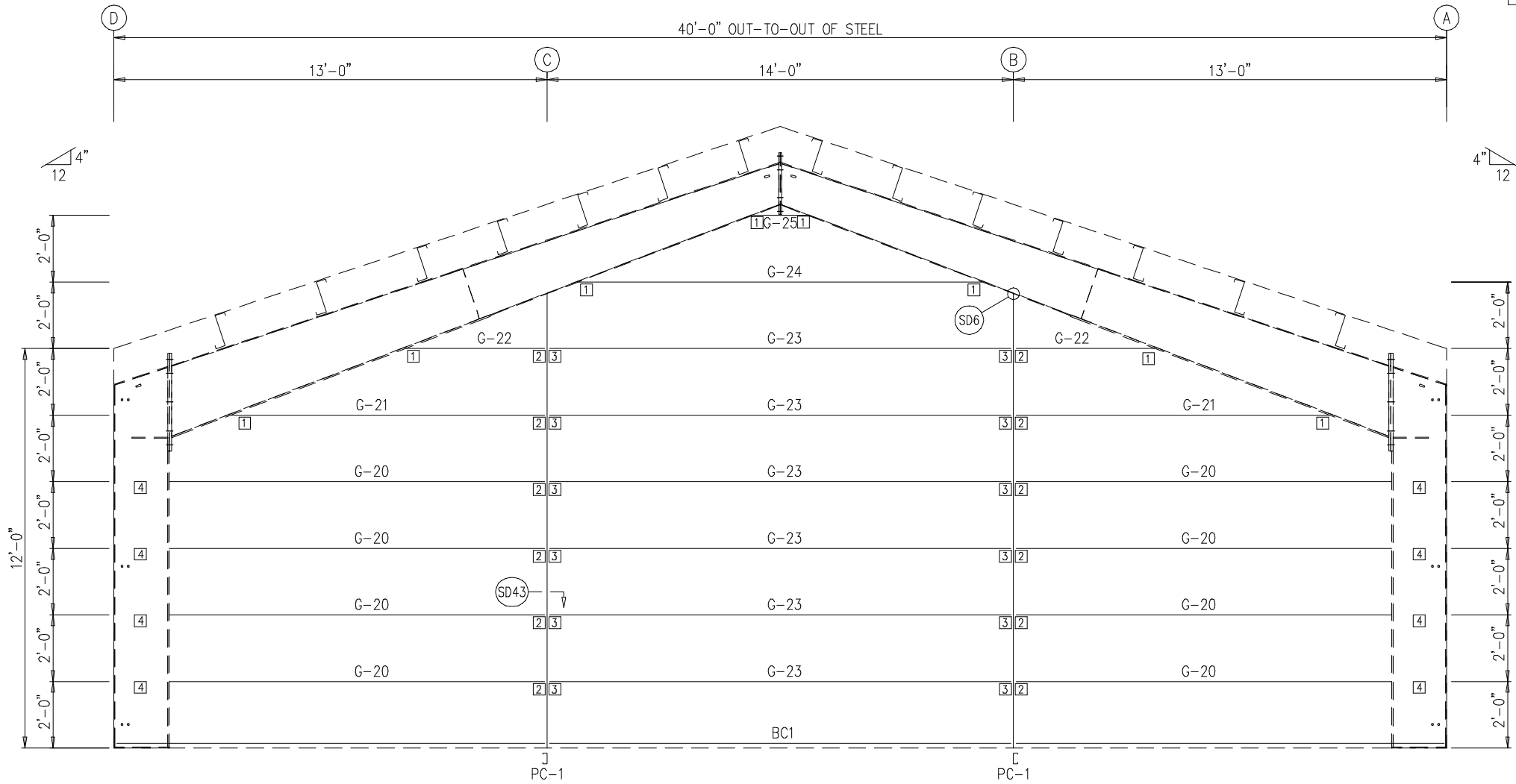
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FLANGE BRACE TABLE		
FRAME LINE 1		
▽ID	QUAN	MARK
1	2	FB30.5

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PARTITION 1 FRAMING: FRAME LINE 2

MEMBER TABLE FRAME LINE 2			
QUAN	MARK	PART	LENGTH
2	PC-1	8x25C16	13'-6 15/16"
8	G-20	8X25Z16	10'-11 7/8"
2	G-21	8X25Z16	8'-10 9/16"
2	G-22	8X25Z16	3'-7 5/8"
6	G-23	8X25Z16	13'-11 1/2"
1	G-24	8X25Z16	11'-7"
1	G-25	8X25Z16	1'-1 3/16"

BOLT TABLE FRAME LINE 2				
LOCATION	QUAN	TYPE	DIA	LENGTH
COLUMNS/RAFTER	2	A325	5/8"	1 1/2"

CONNECTION PLATES FRAME LINE 2		
ID	QUAN	MARK
1	8	CL-109F
2	12	CL-103
3	12	CL-100
4	8	4" ZEE - 1'-6"

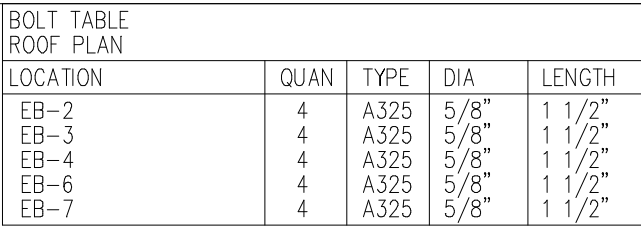
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MEMBER TABLE ROOF PLAN			
QUAN	MARK	PART	LENGTH
1	EB-2	W12X14	3'-5 3/8"
6	EB-3	W12X14	3'-5 3/8"
1	EB-4	W12X14	3'-5 3/8"
1	EB-6	W12X14	3'-5 3/8"
1	EB-7	W12X14	3'-5 3/8"
7	P-1	12X25Z12	26'-1 1/2"
14	P-2	12X25Z12	23'-3 1/2"
14	P-3	12X25Z12	23'-3 1/2"
7	P-4	12X25Z12	22'-1 1/2"
7	P-5	12X25Z12	26'-1 1/2"
7	P-6	12X25Z12	22'-1 1/2"
1	E-1	L12E16-4	22'-11 13/16"
4	E-2	L12E16-4	19'-3 13/16"
2	E-3	L12E16-4	18'-11 13/16"
1	E-4	L12E16-4	22'-11 13/16"
1	E-5	L12E12-4	23'-7 5/8"
4	E-6	L12E14-4	19'-3 13/16"
1	E-7	L12E14-4	19'-7 5/8"
1	E-8	L12E14-4	19'-7 5/8"
1	E-9	L12E12-4	23'-7 5/8"
4	CB-3	CBO313	28'-8 1/2"

[illegible]

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ROOF FRAMING PLAN

Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Section at Hot Rolled Corner Column
Flush Endwall - Flush Sidewall

SD33

Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Girt to Hot Rolled Endwall Column Connection

SD44

(4) 1/2"Ø x 1 1/4" M. Bolts

Endwall Girt to Hot Rolled Rafter

SD45

Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Section at Interior MF Column
Flush Sidewall

SD49

Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Interior Bay Purlin Framing

SD50

Cable or Rod Brace to Frame Connection

SD66

ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE BELOW THE INTENDED GIRT LINE, THE CLIP(S) IN REFERENCE ARE TOED DOWN. IF ABOVE THE GIRT LINE, THEN TOED UP. THE DRAWINGS ABOVE ARE TOED DOWN FOR REFERENCE.

Girt to Jamb (Bolted Clips)

SD87

ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE LEFT OF THE INTENDED JAMB LINE, THE CLIP(S) IN REFERENCE ARE TOED LEFT. IF RIGHT OF THE JAMB LINE, THEN TOED RIGHT. THE DRAWINGS ABOVE ARE TOED LEFT FOR REFERENCE.

Jamb to Girt

SD93

CHK. ENG.

MEZ RTS

CAF RTS

PKD RTS

DATE

04/08/24

07/26/24

08/15/24

DWN.

MEZ

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DETAIL DRAWINGS

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The image contains two technical drawings illustrating structural connections for door and window frames.

Top Drawing (Door Frame Connection):

- Shows a cross-section of a door frame assembly.
- Labels include: "All Bolts are 1/2" ϕ x 1 1/4" A307 M. Bolts (UN Noted)", "4" (dimension for door jamb width), "1/4" (dimension for fin neck thickness), "1/2" x 1" Fin Necks", "Wall Girt", "Door Jamb", and "Header".
- Illustrates the connection between the door jamb, wall girt, and header using bolts and fin necks.

Bottom Drawing (Window Frame Connection):

- Shows a cross-section of a window frame assembly.
- Labels include: "CL-103" (pointing to the wall girt), "CL-48" (pointing to the header), "Header", "1/2" x 1" Fin Necks", "Wall Girt", and "1 1/8" (dimension for fin neck height).
- Illustrates the connection between the window frame, wall girt, and header using bolts and fin necks.

Girt/Header to Jamb

The diagrams show a cross-section of a door jamb and eave strut assembly. In the 'Low Eave Condition' (top), the eave strut is positioned lower, and the door jamb is shown with a 1/2" gap. In the 'High Eave Condition' (bottom), the eave strut is positioned higher, and the door jamb is shown with a larger gap. Both diagrams include labels for 'Eave Strut', 'W.P.', 'Door Jamb', and 'Eave Strut To Jamb Clip CL-8'. A note specifies: 'NOTE: All Bolts are 1/2" x 1 1/4" A307 Mach. Bolts'.

Jamb To Eave Strut
2:12 Roof Pitch and Higher

ERECTOR NOTE: ALL MATERIAL BY OTHERS (U.N.)
FIELD FABRICATE & ASSEMBLE AS REQ'D.
CONNECTION TO MEMBER ABOVE SIMILAR TO HEADER CONNECTION.

Overhead Door Torsion Bar Bearing Connection

Note: No more than (4) purlin spaces before criss-cross.

Installation for more than (6) Purlins

Strapping
Top & Bottom
of Purlins

Peak

Peak

Installation for (6) or less Purlins

Roof Uplift Strap Installation
(Refer to Roof Plan for Locations)

Note: 1) Attach straps w/#10-16 x 1" pancake self driller (RF1) at purlins or girts.
2) No criss-cross straps in walls.

Wall Suction Strap Installation
(Refer to Wall Elevations for Location)

DRAWING NO.
SD102

1/4" Back-up Plate
(If Required by Design)

(2) 1/2" x 1 1/4" A307 M. Bolts

Purlin

Anti-Roll Clip

CL-80 @ 8" Purlins
CL-81 @ 10" Purlins
CL-82 @ 12" Purlins

Rafter

1/4"

1/2" of Rafter

Main Frame Rafter

Section "A"

Purlin to Anti-Roll Clip Connection

DRAWING NO.
SD115

The drawing consists of two parts: a side elevation and a section view.

Side Elevation: This view shows the profile of the eave canopy. Key components and labels include:

- Eave Strut:** Indicated at the left end of the canopy.
- 1/2" ϕ X 1" Fin Neck Bolts:** Pointing to the bolts connecting the canopy to the main frame.
- Per Engineer:** A dimension line indicating the length of the canopy.
- Edge Distance:** A dimension line indicating the distance from the edge of the canopy to the centerline of the bolts.
- Canopy Rafter:** The main structural member of the canopy.
- M.F. Rafter:** The main frame rafter, shown below the canopy rafter.
- 5/8" ϕ A325 Bolts (See Roof Framing Plan For Quantity & Length):** Pointing to the bolts connecting the canopy rafter to the main frame rafter.
- By-Pass:** A dimension line indicating the distance from the main frame to the start of the canopy.
- Projection:** A dimension line indicating the horizontal distance from the main frame to the end of the canopy.
- "A":** A section line with an arrow pointing to the right, indicating the location of Section "A".

Section "A": This view shows the canopy rafter and eave strut in cross-section. Key components and labels include:

- Eave Strut:** The vertical member on the left.
- Canopy Rafter:** The horizontal member on the right.
- 4" 4":** Dimensions indicating the width of the canopy rafter and the spacing between the eave strut and the canopy rafter.
- Section "A":** The section line with an arrow pointing to the left, indicating the location of Section "A".

Eave Canopy at Main Frame

Eave Canopy Framing Detail

DRAWING NO.
SD125

Figure 10 consists of three diagrams illustrating the construction of a wind bent:

- Wind Bent Elevation:** A side view of the wind bent showing its connection to the main frame column and building columns. Labels include "B" (top), Eave Strut, Building Column, Wind Bent, Fin. Flr. (Finish Floor), Bay Spacing, and Detail "A".
- Section "B":** A cross-section showing the wind bent column and main frame column. Labels include Main Frame Column, Wind Bent Column, and Section "B".
- Detail "A":** A detailed view of the connection between the wind bent column and the main frame column. Labels include Eave Strut, Wind Bent Rafter, Main Frame Column, Wind Bent Column, and Detail "A".
- Section "C":** A cross-section showing the wind bent column and main frame column. Labels include Wind Bent Column, Main Frame Column, Steel Line, Bay, and Section "C".

Additional labels and notes include:

- Erection Drawings (Sidewall Elevation Quantity & Length)
- 2" (twice)
- 8" (twice)
- Equal
- Bay
- See Anchor Rod Plan

Flush Wind Bent (Floating)

DRAWING NO.
SD150

NOTE: FIELD SLOT THE GIRT USING THE CORRESPONDING DIMENSIONS AS SHOWN ON THE "FIELD WORK TABLE" ON THE WALL ELEVATION.

Field Work Slotting for Bracing

DRAWING NO.
SD202

[illegible]

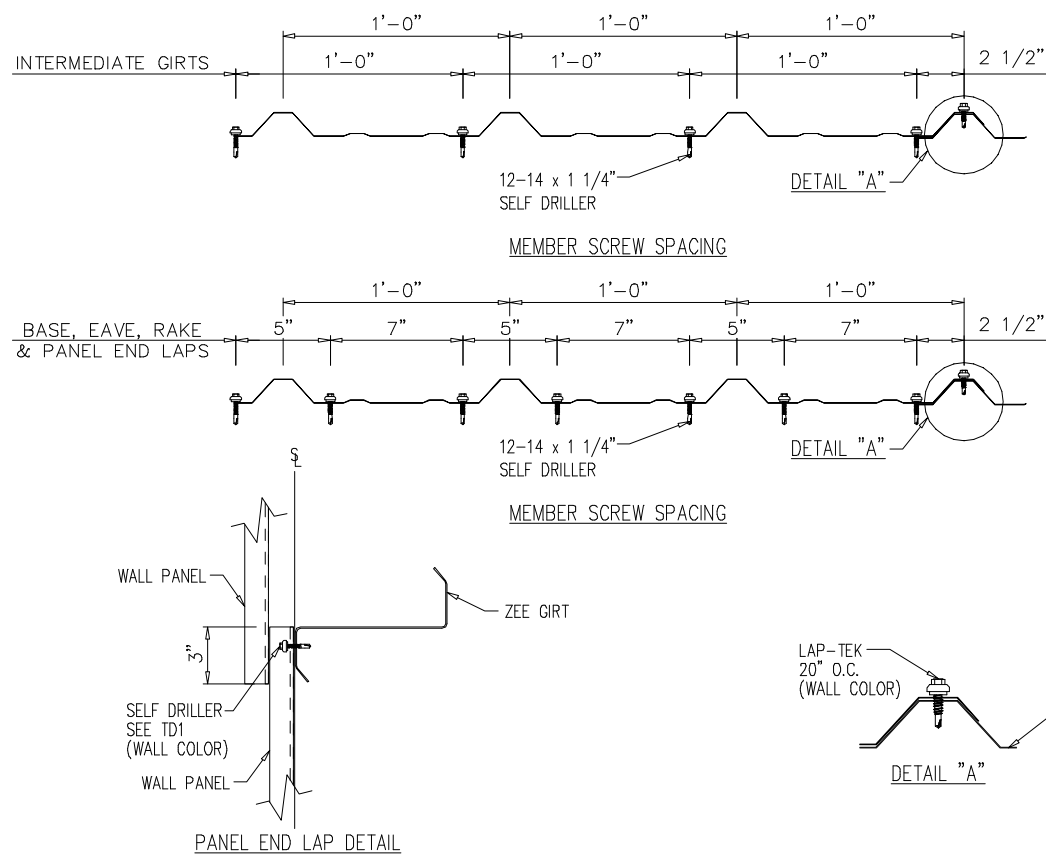
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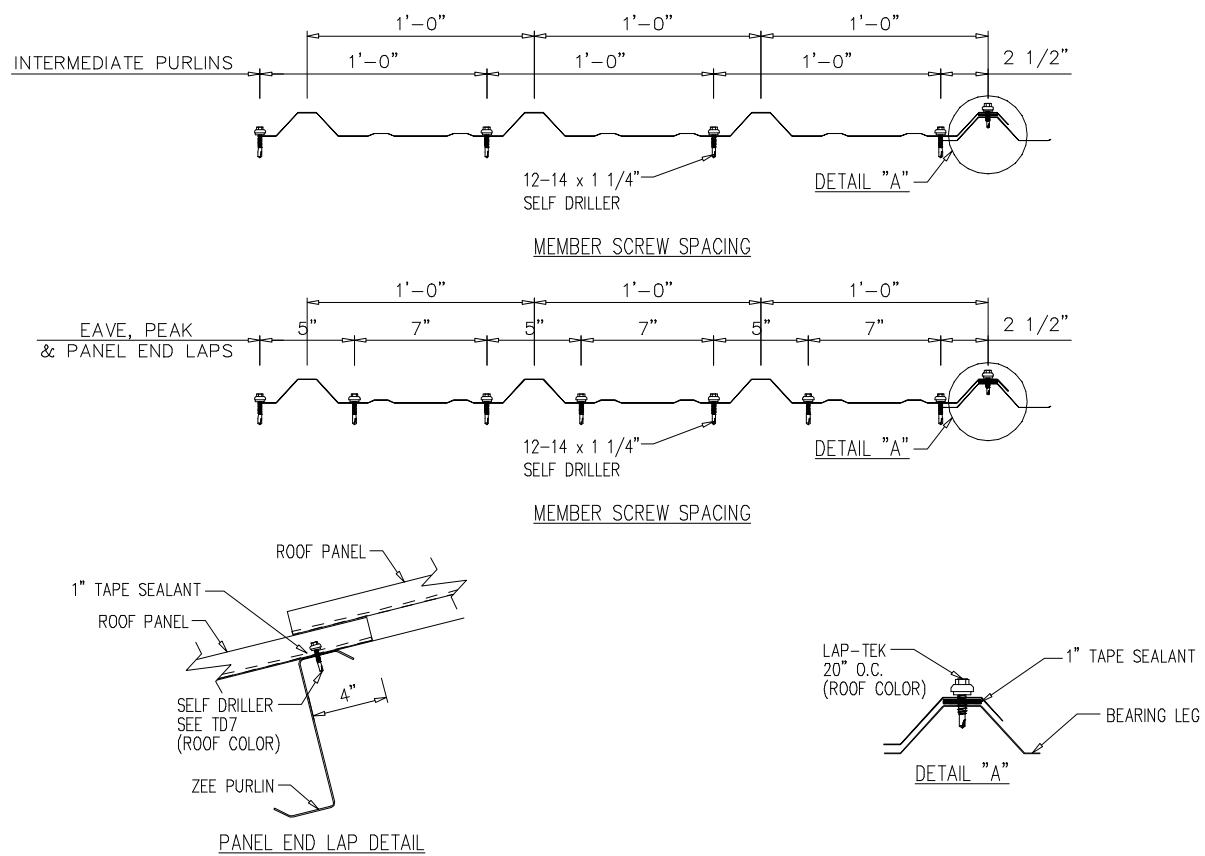
DETAIL DRAWINGS



Fastener Location at Wall – PBR

DRAWING NO.

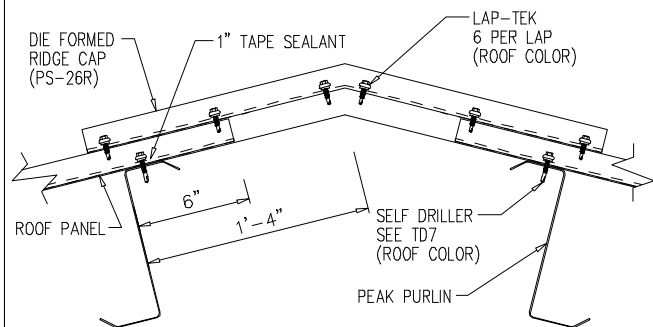
TD1



Fastener Location at Roof – PBR

DRAWING NO.

TD7



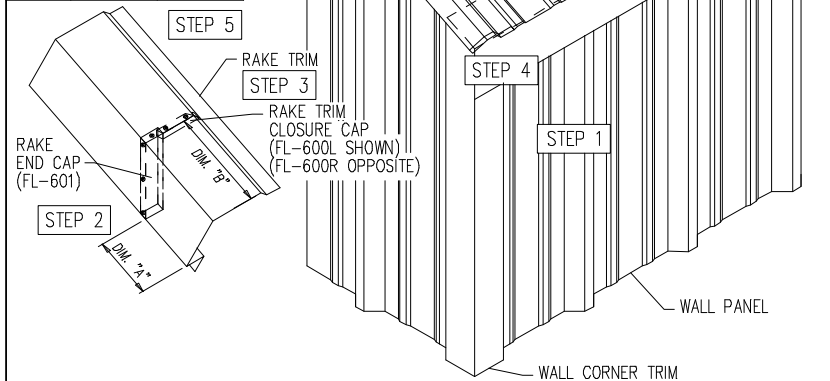
Die Formed Ridge Detail – PBR

Up to a 4:12 Roof Slope

DRAWING NO.

TD8

Slope	Dim. "A"	Dim. "B"
1 : 12	3/16"	3/4"
2 : 12	3/8"	1 1/2"
3 : 12	9/16"	2 1/4"
4 : 12	3/4"	3"
5 : 12	15/16"	3 3/4"
6 : 12	1 1/8"	4 1/2"
7 : 12	1 5/16"	5 1/4"
8 : 12	1 1/2"	6"
9 : 12	1 11/16"	6 3/4"
10 : 12	1 7/8"	7 1/2"
11 : 12	2 1/16"	8 1/4"
12 : 12	2 1/4"	9"



STEP 1 INSTALL FLAT EAVE TRIM IN BETWEEN THE ROOF PANEL AND LOW EAVE MEMBER. BE SURE THE END OF THE FLAT EAVE TRIM IS FLUSH WITH THE WALL CORNER TRIM.

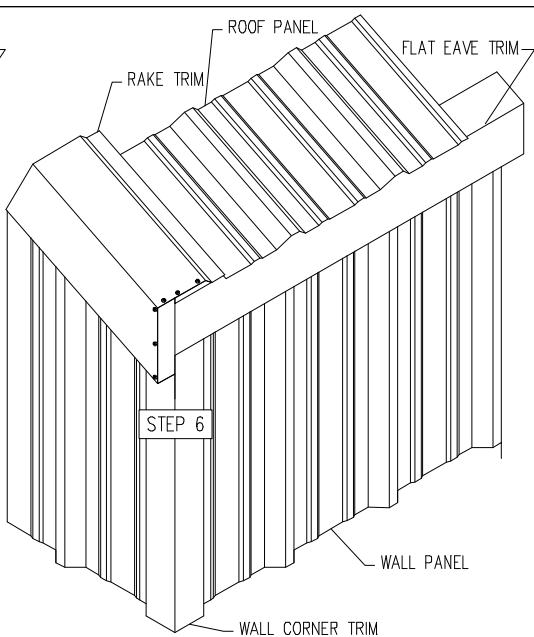
STEP 2 INSTALL RAKE END CAP (FL-601), INTO RAKE TRIM USING (5) POP RIVETS. USE CHART TO DETERMINE HOW FAR THE RAKE END CAP IS POSITIONED INTO THE RAKE TRIM. WITH DIM. "A" BEING THE BOTTOM FACE OF THE RAKE TRIM & DIM. "B" BEING THE TOP FACE OF THE RAKE TRIM.

STEP 3 INSTALL RAKE TRIM CLOSURE CAP (FL-600L/R), FLUSH WITH THE RAKE
END CAP USING (3) POP RIVETS.

STEP 4 FIELD CUT/NOTCH THE END OF THE ROOF PANEL BACK 1". THIS IS TO ALLOW THE RAKE TRIM CLOSURE CAP FROM HITTING THE ROOF PANEL.

STEP 5 INSTALL RAKE TRIM. BE SURE THE BOTTOM END OF THE RAKE TRIM, THAT ATTACHES TO THE HIGH RIBS OF THE WALL PANEL, IS FLUSH WITH THE WALL CORNER TRIM.

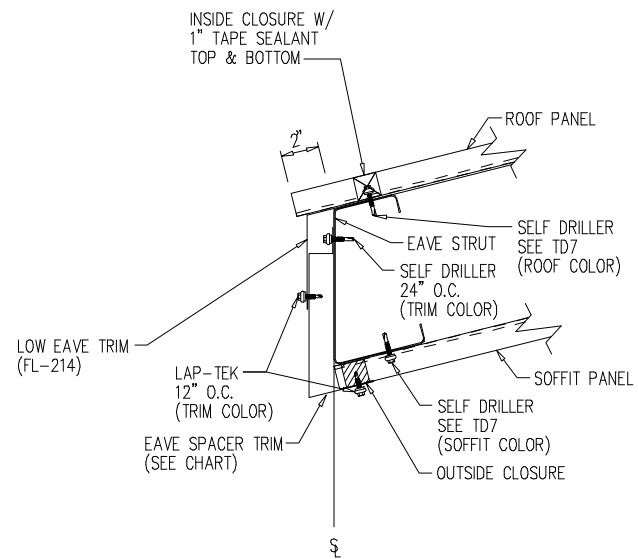
STEP 6 FIELD CUT/NOTCH THE FACE OF THE RAKE TRIM TO BE FLUSH WITH THE WALL CORNER TRIM. BE SURE WHEN CUTTING/NOTCHING THE TOP OF THE RAKE TRIM, THAT SITS ON TOP OF THE ROOF PANEL, SHOULD BE LINED UP WITH THE END OF THE ROOF PANEL.



Flat Eave Corner Trim Installation – PBR

DRAWING NO.

TD12



Low Eave Detail – PBR

Simple Eave – Open Wall – With Soffit

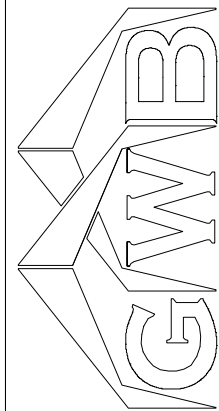
TRIM CHART	
PURLIN SIZE	PIECE MARK
8"	FL-500
10"	FL-500
12"	FL-500

DRAWING NO.

TD21

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DETAIL DRAWINGS

SHEET TITLE:

