

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2	MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
	STEEL PIPES	A500	Fy = 42 KSI
	STRUCTURAL TUBING	A500	Fy = 42 KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
	COLD FORMED LIGHT GAGE	A529/A572 A653/A1011	Fy = 55 KSI
	ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI

MACHINE BOLTS & NUTS HIGH STRENGTH BOLTS (1"Ø & LESS) HIGH STRENGTH BOLTS (1"Ø TO 1 1/2"Ø) A325-TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554 Fu = 60 KSIFu = 120 KSI Fu = 105 KSI Fu = 60 KSI

MIN. TENSILE STRENGTH

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOSSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.

(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT

- 4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS
- .5 ALL BOLTS ARE 1/2" Ø x 0'-1 1/4" A307 EXCEPT:
 A) ENDWALL RAFTER SPLICE 5/8" Ø x 0'-1 3/4" A325-N
 B) ENDWALL COLUMN TO RAFTER CONNECTION (SEE WALL ELEVATION) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANGE BRACE CONNECTIONS - 1/2" x 0'-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE— UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE — BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES

ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING

DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
 STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
 DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
 METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATI
 TO THE CONTRARY NOTWITHSTANDING, IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
 BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
 EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

- 2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. THE BUTER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION.

 ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS

 CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE

 CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER, SPECIFIC DESIGN CRITERIA

 CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR

 FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,

 ATEST ENTITION)
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY MITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND
- UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD PRACTICE LATEST FOILION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FUNNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO DEPENDATION OF SUCH SOURCE ALSO COND. PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>WARNING</u> IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLQY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RIVN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFETS AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERCENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLICHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR INCEDIBLING. SEE TABLE DELOW. ICE BUILDUP. SEE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

BUILDING LOADS THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED: **DESIGN LOADS:** DESIGN CODE / WIND CODE : IBC-21 OCCUPANCY / RISK CATEGORY : II-Normal **ENCLOSURE** : Enclosed ROOF DEAD LOAD (D) (PSF) : 2.00 ROOF COLLATERAL LOAD (C) (PSF) : 1.00 WIND LOAD ULTIMATE WIND SPEED, (VULT) (MPH) :115.00 WIND EXPOSURE CATEGORY : C :0.18/-0.18 INTERNAL PRESSURE COEFFICIENT, (GCpi) WALL PANEL DESIGN WIND PRESSURE (PSF) : 25.32/-27.46 WIND ENCLOSURE CLASSIFICATION : Enclosed LIVE LOAD PRIMARY FRAMING (PSF) : 20.00 TRIB. AREA REDUCTION :No SECONDARY FRAMING (PSF) : 20.00 SNOW LOAD GROUND SNOW LOAD, (Pg) (PSF) ROOF SNOW LOAD, (Pf) (PSF) : 55.00 : 55.00 SNOW EXPOSURE FACTOR, (Ce) :1.00 SNOW IMPORTANCE FACTOR, (Is) : 1.00 THERMAL FACTOR, (Ct) : 1.20 SEISMIC LOAD SEISMIC IMPORTANCE FACTOR, (Ie) :1.00 SITE CLASSIFICATION : D SPECTRAL RESPONSE ACCELERATION : Ss = 0.245 : S1 = 0.086SPECTRAL RESPONSE COEFFICIENTS : Sds = 0.261 : Sd1 = 0.138SEISMIC DESIGN CATEGORY : C BASIC SEISMIC FORCE RESISTING SYSTEM :STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMF) :LONGITUDINAL = 3.34 TOTAL DESIGN BASE SHEAR, (V) (KIPS) :TRANSVERSE = 3.41 :RIGID FRAMES = 3.00 RESPONSE MODIFICATION FACTORS, (R) $\Omega = 3.00$:SW X-BRACING = 3.00 $\Omega = 3.00$ SEISMIC RESPONSE COEFFICIENTS, (Cs) :RIGID FRAMES = 0.0872:SW X-BRACING = 0.0872ANALYSIS PROCEDURE USED : EQUIVALENT LATERAL FORCE PROCEDURE OTHER LOADS/REQUIREMENTS **BUILDING DESCRIPTION:** WIDTH (FT) : 43.00 : 46.00 : 3.0:12 : 3.0:12 BAY SPACING (FT) : 4 AT 11.50

LENGTH (FT) EAVE HEIGHT AT BSW (FT): 15.30 EAVE HEIGHT AT FSW (FT): 15.30 ROOF SLOPE AT BSW ROOF SLOPE AT FSW

COVERING AND TRIMS:

ROOF & SOFFIT PANELS & TRIMS

PANEL TYPE :26 GA. PBR PANEL COLOR : FERN GREEN TRIM COLORS

GABLE/EAVE :FERN GREEN EAVE GUTTER :N/A

WALL PANELS & TRIMS

PANEL TYPE :26 GA. PBR

PANEL COLOR : KOKO BROWN TRIM COLORS

: FERN GREEN CORNER :FERN GREEN FRAMED OPENING

DOWNSPOUTS : N/A : KOKO BROWN BASE

INSULATION

ROOF INSULATION WALL INSULATION : N/A

THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS

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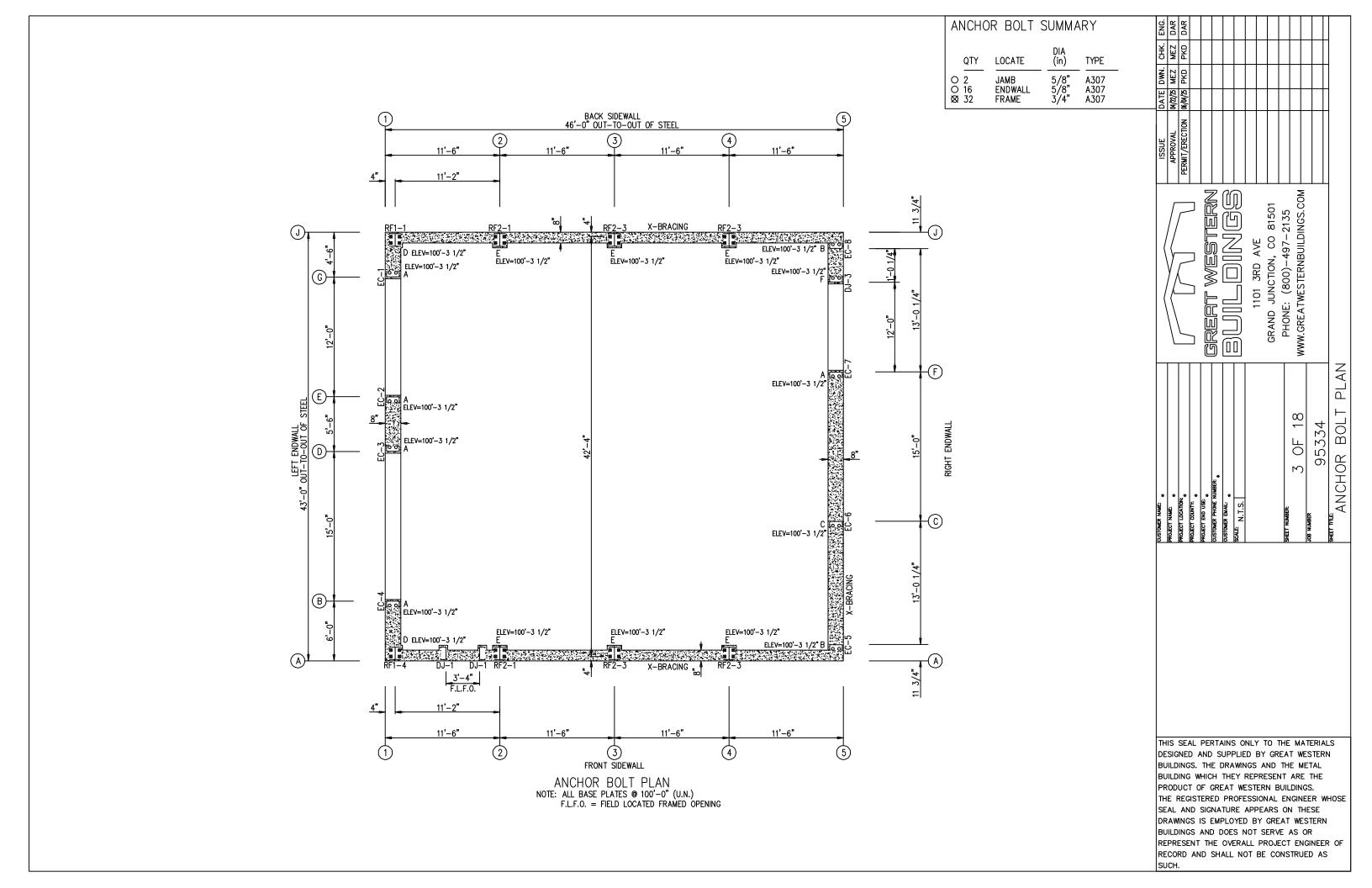
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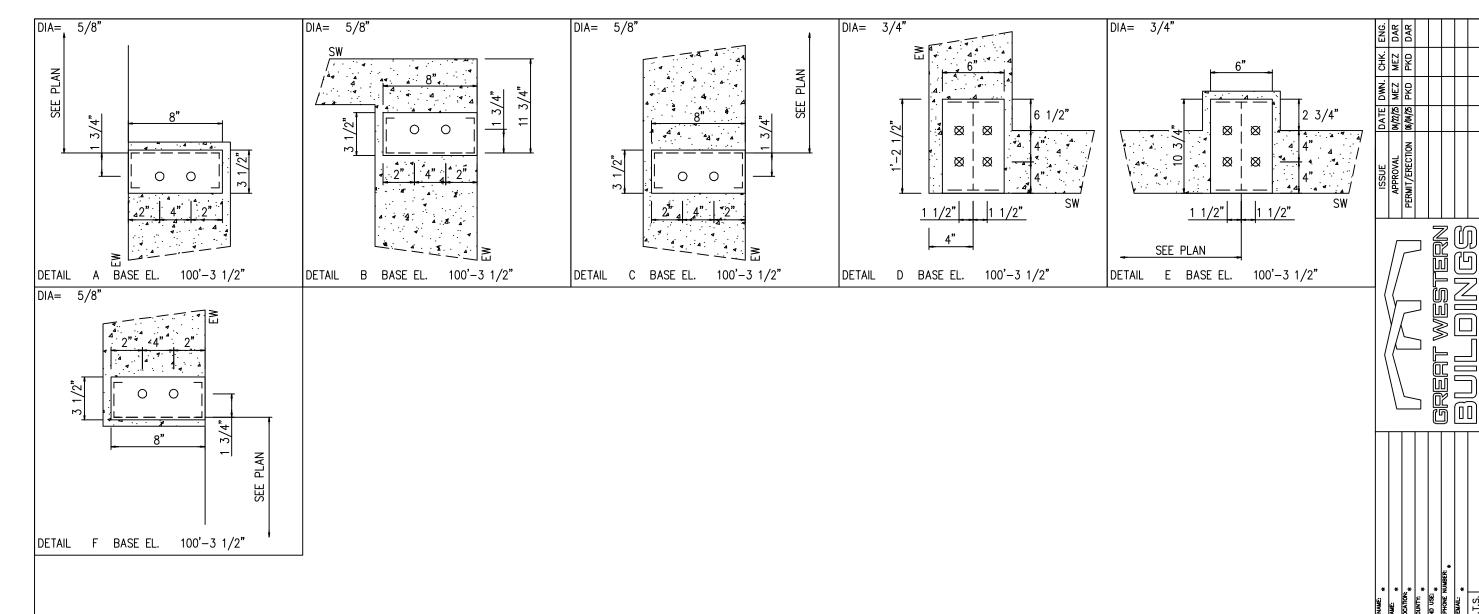
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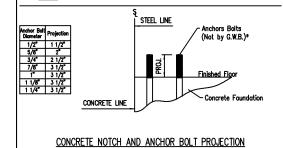
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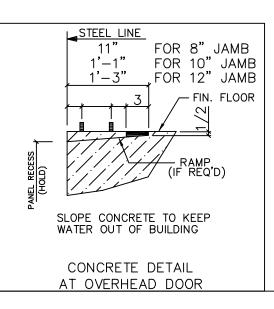
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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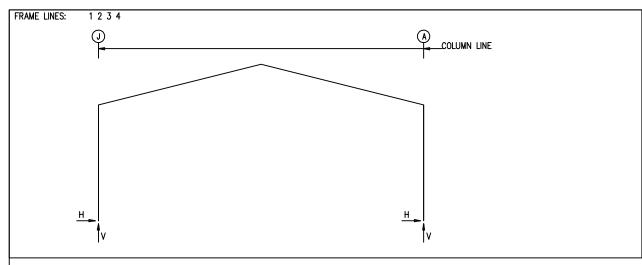
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RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BASE	PLATE	S				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolf QTY	t(in) DIA	Base Width	_Plate(in) Length	Thick	Grout (in)
1	J	1	3.4	9.6	4 2	-0.9 -0.9	-0.6 -1.3	4	0.750	6.000	14.50	0.375	3.5
1	Α	5 1	0.9 -3.4	-0.6 9.6	1 3	-3.4 0.9	9.6 -1.3	4	0.750	6.000	14.50	0.375	3.5

RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BAS	E PLATE	ES .				
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	J	1	7.0	15.6	2	-1.6 0.1	-2.4 -4.0	4	0.750	6.000	10.75	0.375	3.5
2*	Α	3 1	1.6 -7.0	-2.4 15.6	1 7	-7.0 -0.1	15.6 -4.0	4	0.750	6.000	10.75	0.375	3.5
2*	FRAME lin	nes:	2 3 4										

NOTES F	OR REACTIONS	
the for W	ig reactions are based on illowing building data: fidth (ft) ength (ft) ave Height (ft) oof Slope (rise/12) ead Load (psf) ive Load (psf) ive Load (psf) illimate Wind Speed (mph) find Code xposure losed/Open mportance Wind mportance Seismic eismic Coeff (Fa*Ss)	43.00 46.00 15.30/15.30 3.0:12/3.0:12 2.00 55.00 115C-21 C Enclosed 1.00 1.00 C 0.39
ID D	escription	

1	Dead+Collateral+Snow+Slide_Snov
2	0 CD 1 0 CW: 1

1 Dead+Collateral+Snow+Slide_Snow
2 0.6Dead+0.6Wind_Left1
3 0.6Dead+0.6Wind_Right1
4 0.6Dead+0.6Wind_Right1
5 0.6Dead+0.6Wind_Right2
6 0.6Dead+0.6Wind_Right2
7 0.6Dead+0.6Wind_Long1L
7 0.6Dead+0.6Wind_Long2L
8 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
10 Dead+0.6Wind_Right2+0.6Wind_Suction
11 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
13 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
14 Dead+Collateral+E2UNB_SL_L
15 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
16 Dead+Collateral+E2UNB_SL_R
17 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

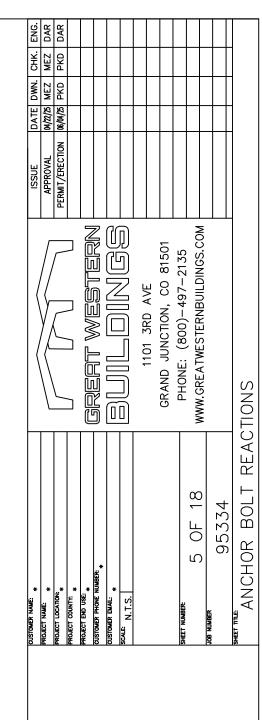
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Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Note
L_EW F_SW R_EW B_SW	1 A 5 J	3,4 A,C 4,3	3.2 0.9 3.2	3.7 1.1 3.7	1.7 0.5 1.7	1.9 0.7 1.9			(h)
(h)Rigid	frame	e at endv	vall						

AN	ICHO	R BOLT S	SUMMA	\RY
	QTY	LOCATE	DIA (in)	TYPE
00 8		JAMB ENDWALL FRAME	5/8" 5/8" 3/4"	A307 A307 A307

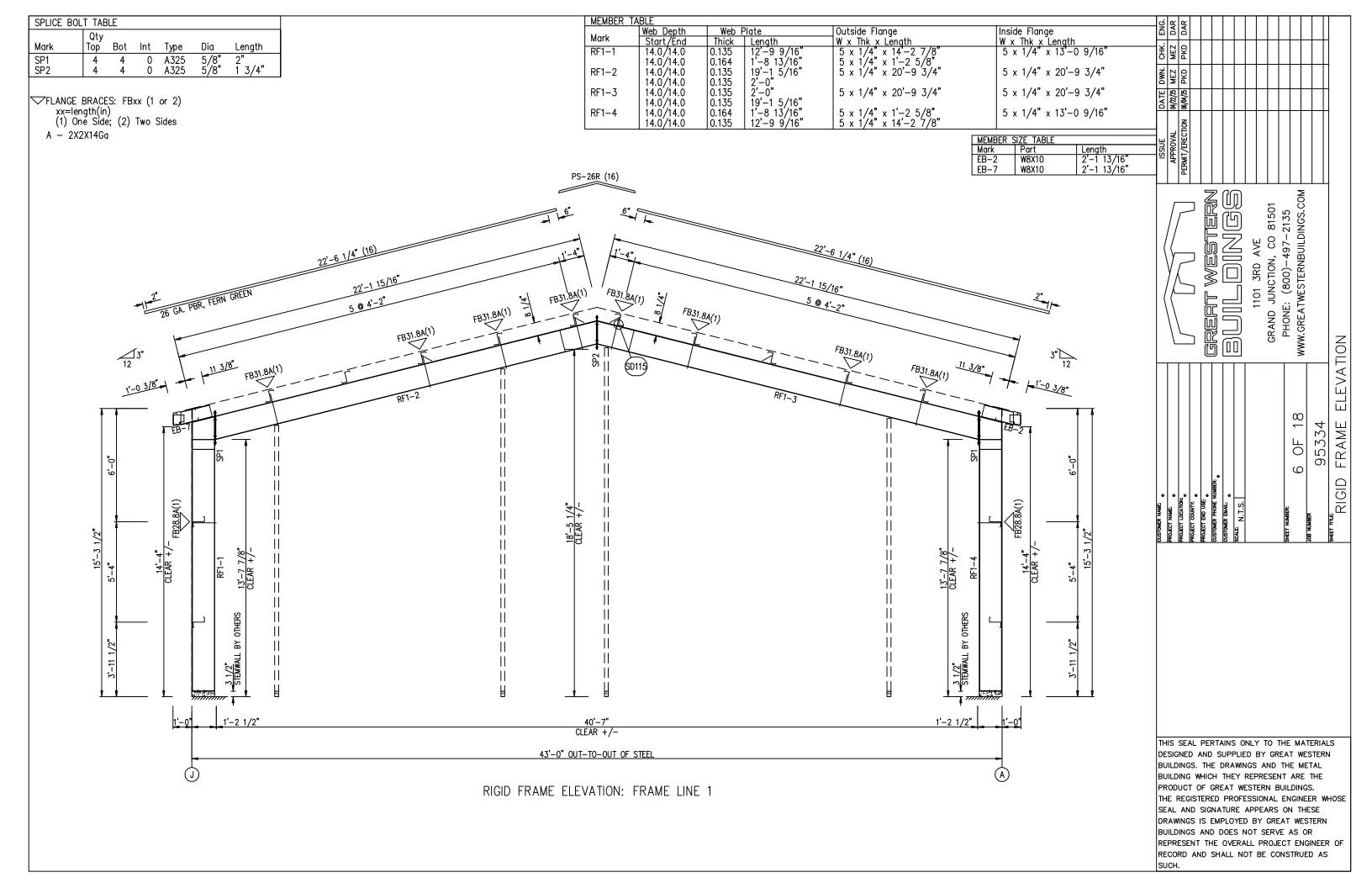
RIGIE	FRAN	ΙE:	BASI	C COLUN	IN REACT	10NS (k))						
FRAME Line 1	Column Line J A	Horz 0.2 -0.2	Dead Vert 0.8 0.8	-—-Colle Horz 0.1 -0.1	ateral- Vert 0.2 0.2	Horz 1.1 –1.1	-Live Vert 3.1 3.1	Horz 3.1 -3.1	-Snow Vert 8.6 8.6	-—-Wind Horz -1.7 -0.3	_Left1- Vert -3.1 -1.9	-Wind_ Horz 0.3 1.7	Right1- Vert -1.9 -3.1
FRAME Line 1	Column Line J A	Wind Horz -1.7 -0.2	_Left2- Vert -1.9 -0.8	-Wind_ Horz 0.2 1.7	Right2- Vert -0.8 -1.9	Wind Horz 0.0 0.2	l_Long1- Vert -2.4 -2.1	Wind Horz -0.2 0.0	d_Long2- Vert -2.1 -2.4	-Seismi Horz -0.3 -0.3	ic_Left Vert -0.2 0.2	Seismid Horz 0.3 0.3	:_Right Vert 0.2 -0.2
FRAME Line 1	Column Line J A	-MIN_S Horz 1.1 -1.1	NOW Vert 3.1 3.1	F1UNB_ Horz 2.4 -2.4	SL_L- Vert 8.0 4.6	F1UNB_ Horz 2.4 -2.4	SL_R- Vert 4.6 8.0						
FRAME Line 2* 2*	Column Line J A	Horz 0.4 -0.4	Dead Vert 1.1 1.1	-—-Colle Horz 0.1 -0.1	ateral- Vert 0.3 0.3	 Horz 2.3 -2.3	-Live Vert 5.2 5.2	Horz 6.4 -6.4	-Snow Vert 14.2 14.2	-—-Wind Horz -3.1 -0.1	_Left1- Vert -5.1 -3.2	-Wind_ Horz 0.1 3.1	Right1- Vert -3.2 -5.1
FRAME Line 2* 2*	Column Line J A	Wind Horz -3.0 -0.2	_Left2- Vert -3.2 -1.3	-Wind_ Horz 0.2 3.0	Right2- Vert -1.3 -3.2	Wind Horz -0.2 0.6	l_Long1- Vert -7.8 -7.2	Wind Horz -0.6 0.2	d_Long2- Vert -7.2 -7.7	-Seismi Horz -0.4 -0.4	ic_Left Vert -0.3 0.3	Seismid Horz 0.4 0.4	:_Right Vert 0.3 -0.3
FRAME Line 2* 2*	Column Line J A	Seismic Horz 0.0 0.0	_Long1 Vert -1.9 -1.9	Seismic Horz 0.0 0.0	:_Long2 Vert 1.9 1.9	-MIN_S Horz 2.3 -2.3	5NOW Vert 5.2 5.2	F2UNB. Horz 5.0 -5.0	_SL_L- Vert 13.3 7.6	F2UNB_ Horz 5.0 -5.0	SL_R- Vert 7.6 13.3		
2*	FRAME lin	ies:	2 3 4	•									

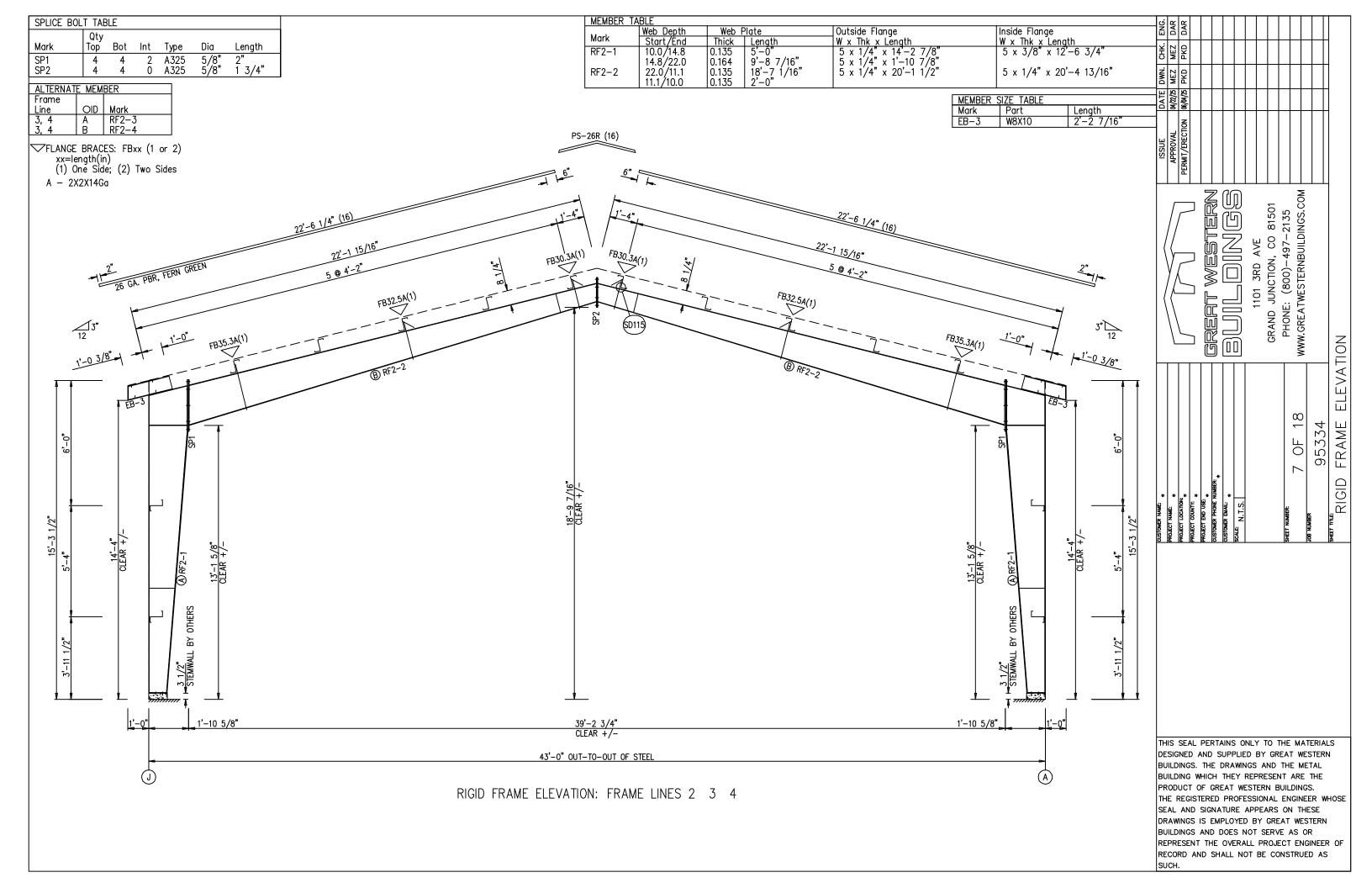
ENDV Frm Line 1 1 1	VALL Col Line G E D B	Dead Vert 0.0 0.1 0.1 0.0	Wind Press Horz -1.2 -1.5 -1.9	BASIC Wind Suct Horz 1.3 1.7 2.1		10NS (k)	,								
Frm Line 5 5 5	Col Line A C F J	Dead Vert 0.2 0.4 0.4 0.2	Collat Vert 0.0 0.1 0.1 0.0	Live Vert 0.9 2.2 2.2 0.9	Snow Vert 2.6 6.1 6.1 2.6	Wind_L Horz -0.9 0.0 0.0	Left1 Vert -2.5 -1.6 -1.6 -1.1	Wind_F Horz 0.0 0.9 0.0 0.0	Right1 Vert 0.3 -3.1 -2.8 -1.3	Wind_Le Horz -0.9 0.0 0.0	eft2 Vert -2.0 -0.9 -0.9 -0.6	Wind_F Horz 0.0 0.9 0.0 0.0	Right2 Vert 0.8 -2.3 -2.1 -0.9	Wind Press Horz -1.0 -2.5 -2.5 -1.0	
Frm Line 5 5 5	Col Line A C F J	Wind Suct Horz 1.2 2.7 2.7 1.2	Wind_ Horz 0.0 0.4 0.0 0.0	Long1 Vert -0.7 -3.2 -1.5 -0.8	Wind_Long2 Horz Vert -0.4 -1.4 0.0 -0.9 0.0 -2.5 0.0 -1.4		s_Left v Vert -0.7 0.7 0.0 0.0	Seis Horz 0.0 0.5 0.0	_Right vert 0.8 -0.8 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	- I Ho O. O. O.	0 0. 0 2. 0 2.	ert 9 2		
Frm Line 5 5 5	Col Line A C F J	E2UNB_S Horz 0.0 0.0 0.0 0.0	L_L- Vert 2.6 7.1 2.4 0.7	0.0 0.0 0.0	R- Vert 0.7 2.4 7.1 2.6										
ENDV	WALL	COLUMI	N:	MAXIM	UM REACTIONS,	ANCHOR E	30LTS, & E	BASE PLA	TES						

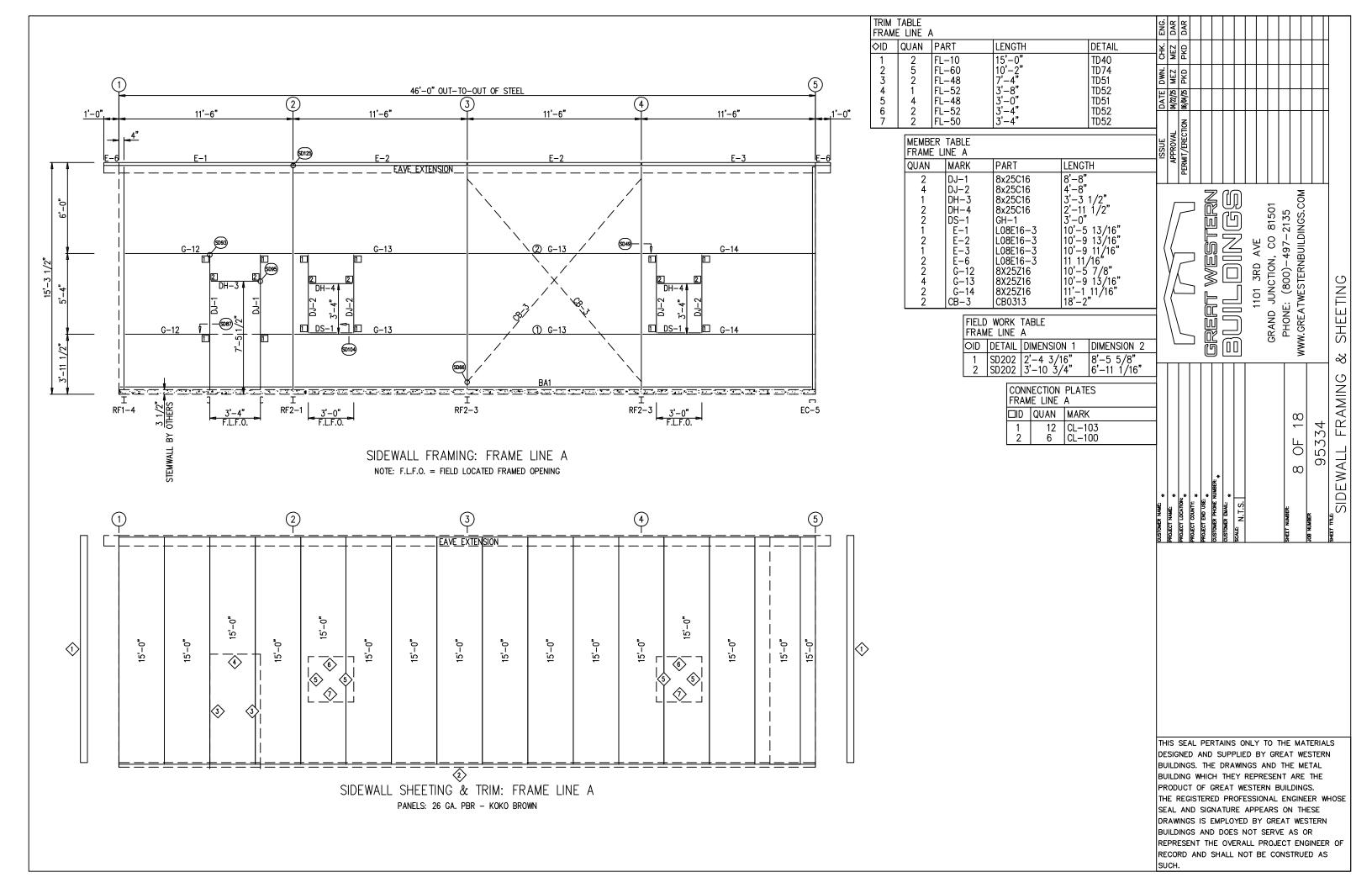
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grou (in
1	G	- — 8 10	0.8	0.0	9	-0.7	0.0	2	0.625	3.500	8.000	0.250	3.
1	Ε	8 10	1.0 1.0	0.0 0.1	9	-0.9	0.0	2	0.625	3.500	8.000	0.250	3.
1	D	8 10	1.3 1.3	0.1 0.1	9	-1.1	0.1	2	0.625	3.500	8.000	0.250	3.
1	В	8 10	1.0 1.0	0.0 0.0	9	-0.9	0.0	2	0.625	3.500	8.000	0.250	3.
5	Α	11 1	0.7 0.0	-1.4 2.8	9 11	-0.6 0.7	−0.7 −1.4	2	0.625	3.500	8.000	0.250	3.
5	С	12 14	1.6 0.0	-1.7 7.6	13 12	−1.5 1.6	−1.7 −1.7	2	0.625	3.500	8.000	0.250	3.
5	F	15 16	1.6 0.0	-1.5 7.6	9 15	-1.5 1.6	−1.3 −1.5	2	0.625	3.500	8.000	0.250	3.
5	J	17 1	0.7 0.0	-0.7 2.8	9 17	-0.6 0.7	-0.7 -0.7	2	0.625	3.500	8.000	0.250	3.

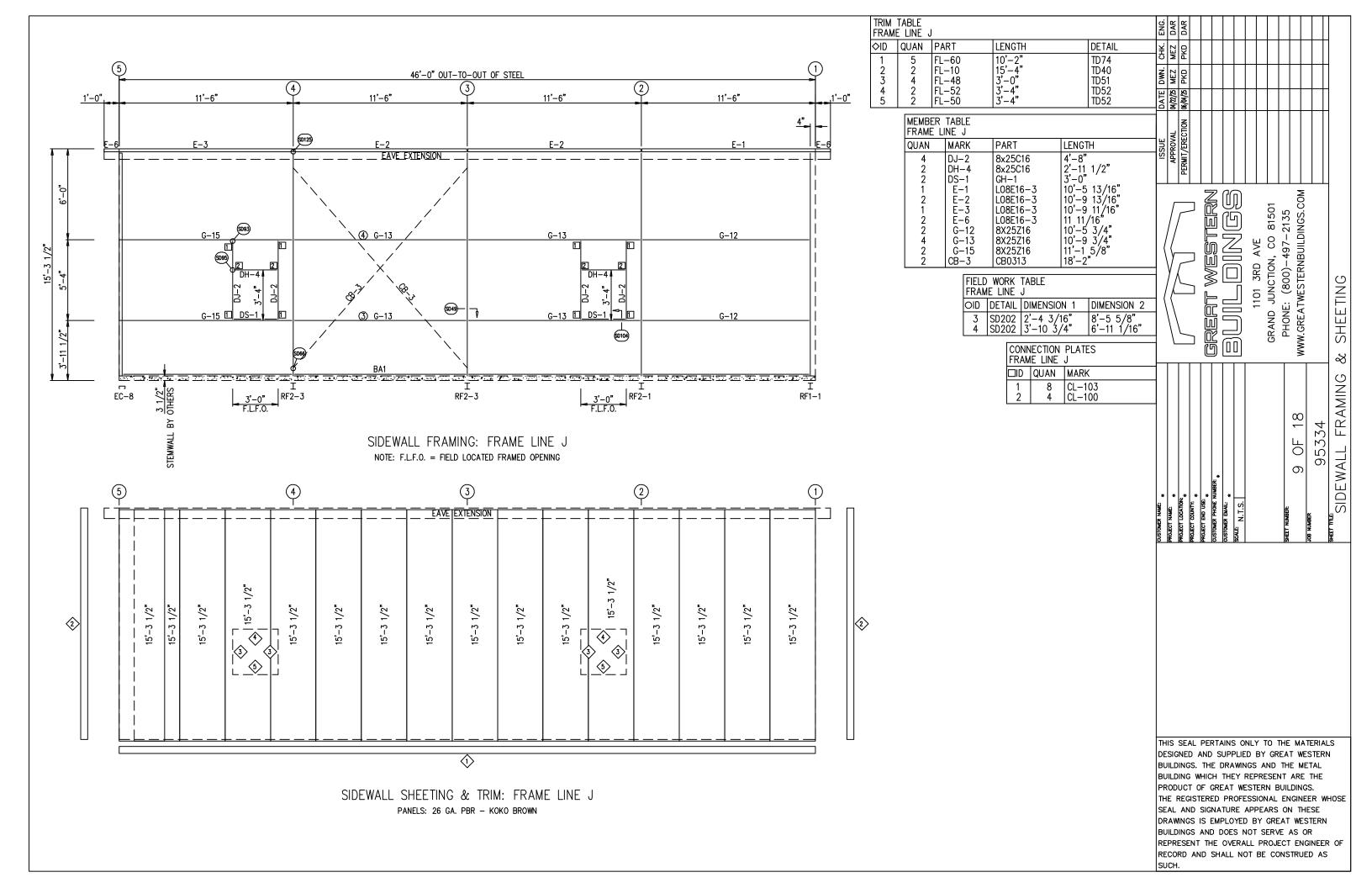


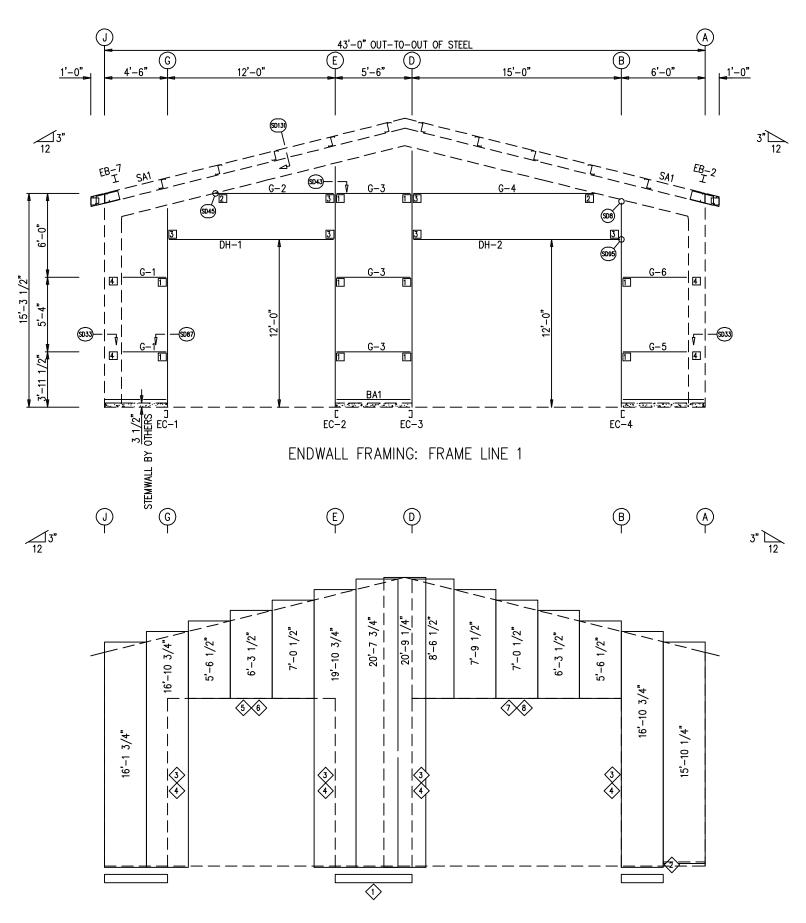
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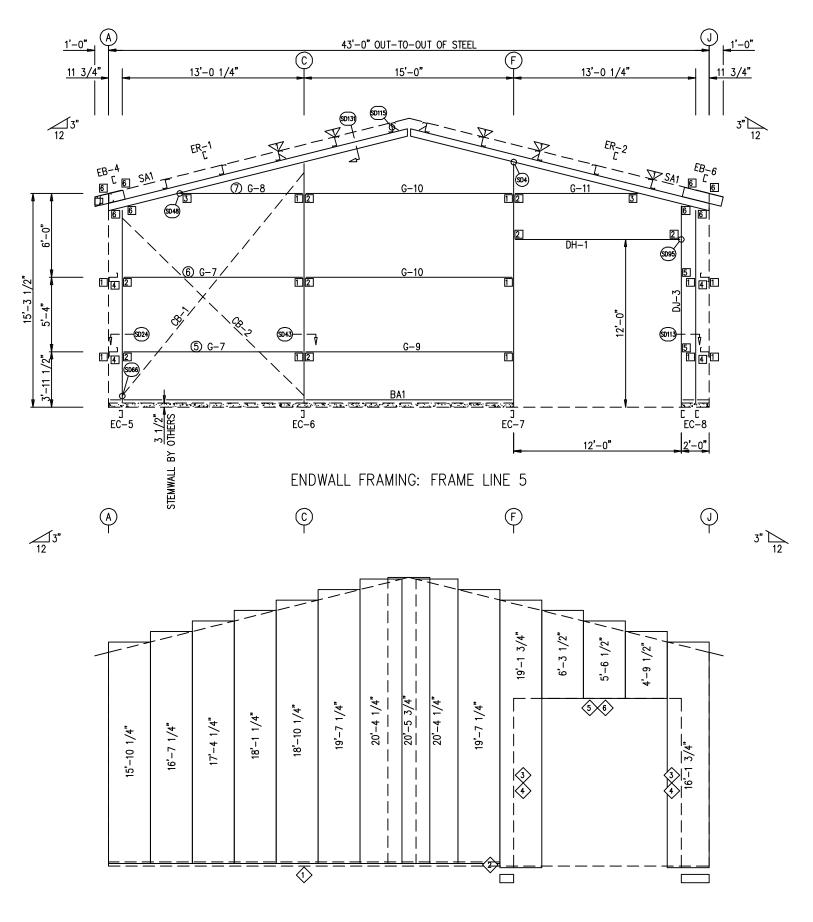






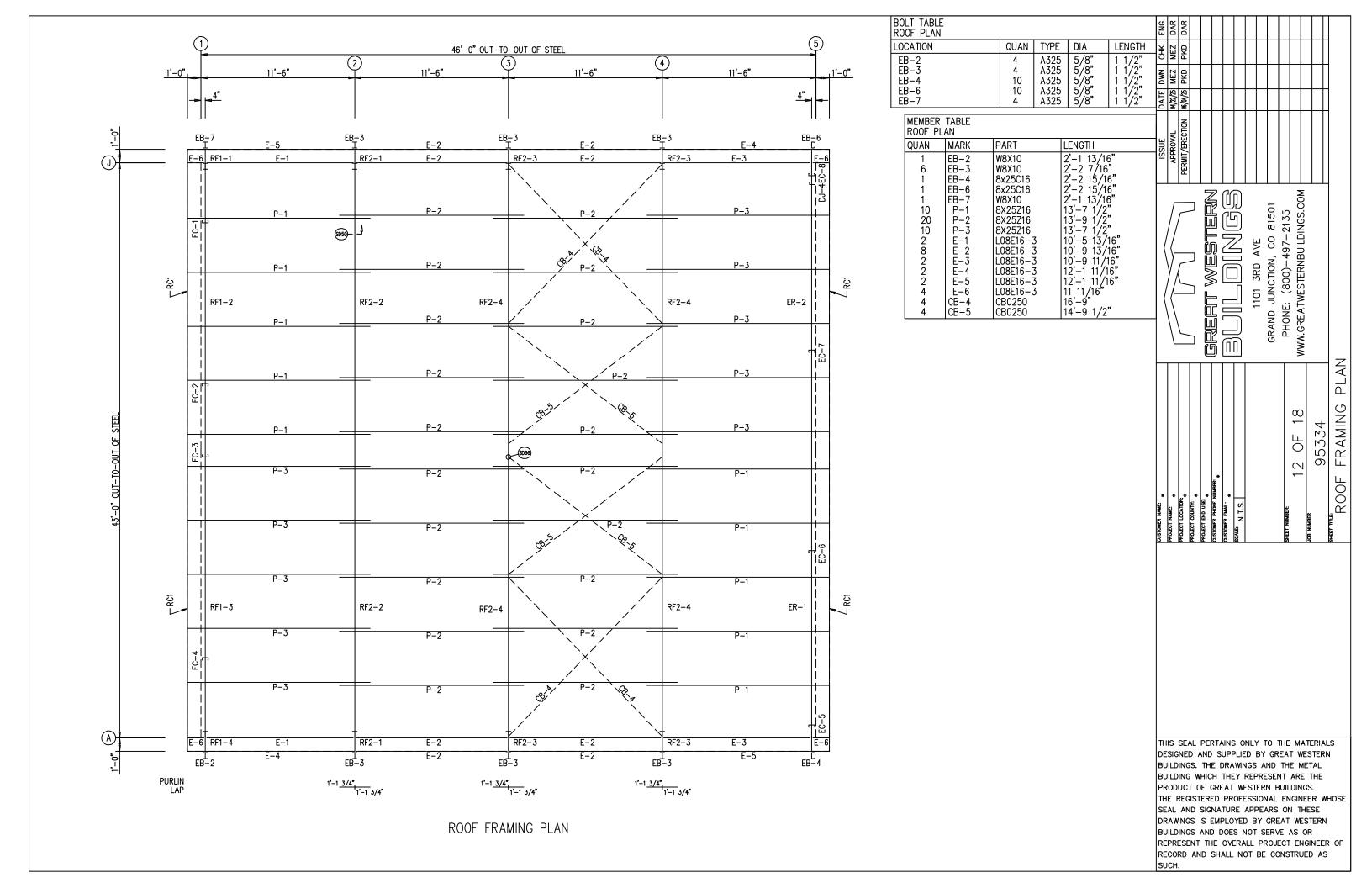
ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - KOKO BROWN

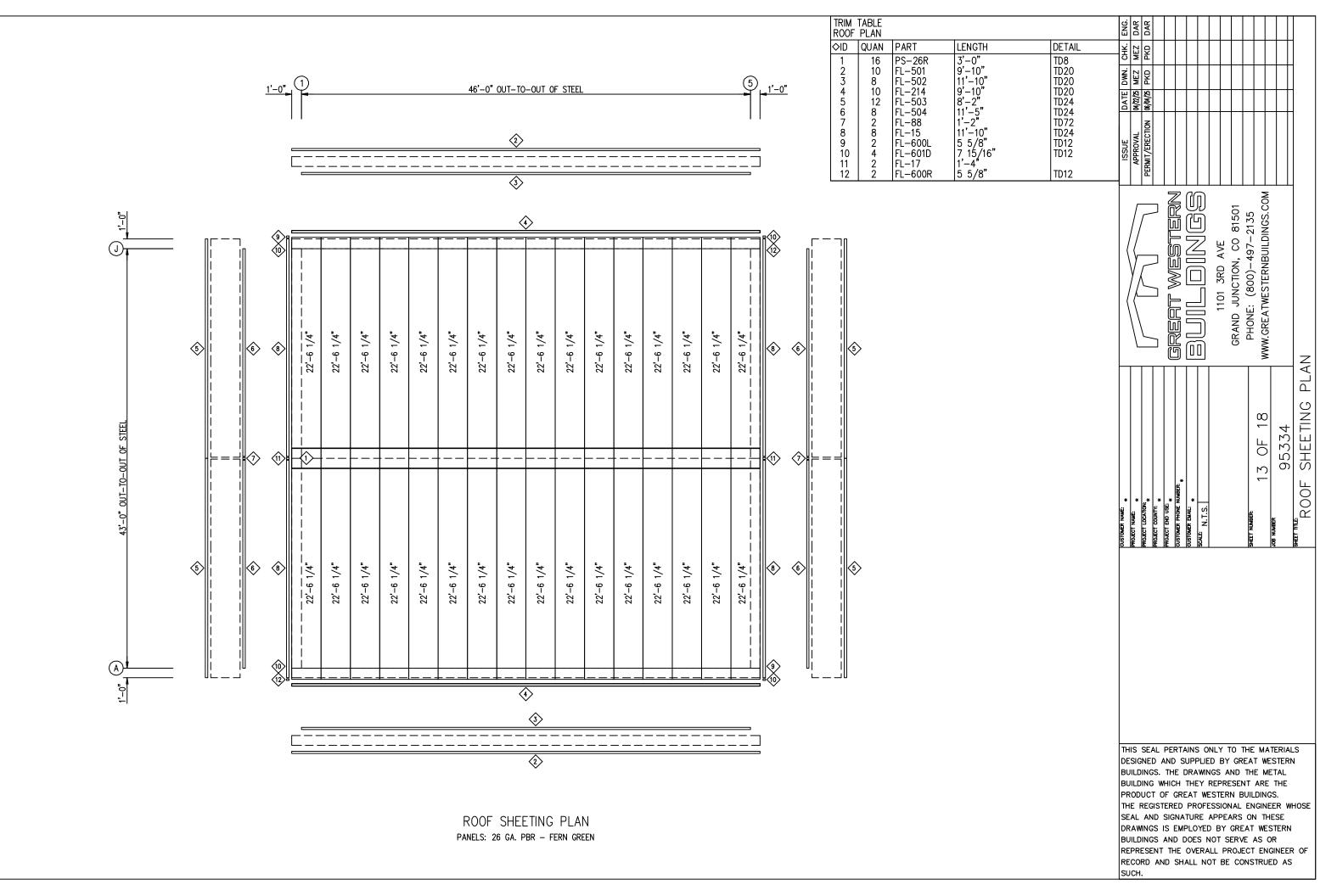
TRIM	TABLE				٩	ENG.	DAR	뜻		1	П		1		П	
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	2	FI -60	10'-2" 1'-2" 11'-11" 12'-2" 12'-7" 12'-4"	TD74 TD36	—		MEZ		_		Н	\perp				
1 2 3 4 5 6 7	1 4	FL-48 FL-55	11'-11"	TD51	7	DAIE DWN.	04/22/25 MEZ	S S								
5	4	FL-48 FL-55	12,-2	TD51 TD51 TD52	S ATE	<u>ا ۲</u>	4/22/22	96/04/25								
6 7	1 1	FL-48 FL-55 FL-52 FL-55	12 – 4 15' – 7"	TD52 TD52												
8	1	FL-52	15'-4"	TD52	<u>'</u>	ISSUE	APPROVAL	ERECT								
E	BOLT TAI	BLE INE 1			<u> 8</u>	3	APPR	PERMIT/ERECTION								
	OCATION		QUAN TY		LENGTH			Н								
L		RAFTER	2 A3	25 5/8"	1 1/2"		_		ם מו		(M)	_	M O		
	FRAM	ER TABLE E LINE 1				,	//		, [81501	PHONE: (800)-497-2135		
	QUAN 1	MARK EB-2	PART W8X10	LENGTH	,"	/	/		Π	_	5	, į	8	7-2		
	1	IEB-7	W8X10	2'-1 13/16)) .,,	\	\sum_{i}		ן נ	תן וור		} {	Ž	- 49 NBU		
		EC-1 EC-2	8x25C16 8x25C14 8x25C12	17'-1 5/16	0,0		//] [2	CTC	300) TFR		<u>ا</u> ی
	1 1	EC-3 EC-4	18725016	18 -2 13/1 14'-5 13/1	6"	(灹	_	J		\Box	1101 3PD AVE	5 N	(B)		
	1 1	DH-1 DH-2	8x25C16 8x25C16 8x25C16 8x25Z16 8x25Z16 8x25Z16 8x25Z16	11'-11 1/2 14'-11 1/2	" "	\	/		() []		=)	GRAND JUNCTION, CO	ONE RFA		SHEETING
	1 3 1	G-1 G-2	8X25Z16 8X25Z16	2'-11 3/16 8'-4 11/16	,, ,,		/	_	ງ ຢູ່		믅) 1	GRA	PH W		빐
	3	G-3 G-4	8X25Z16 8X25Z16	4'-9 13/16 12'-10 11/	6" 16"	_			((<u>'</u> 5]		≶		্থ প্ৰ
	1 1	G-5 G-6	8X25Z16 8X25Z16	2'-1 13/16 2'-1 13/16 14'-1 5/16 18'-2 13/1 14'-5 13/1 11'-11 1/2 2'-11 3/16 8'-4 11/16 4'-9 13/16 12'-10 11/ 4'-5 3/16'	,											
		•	CONNECTION	ON PLATES												FRAMING
			FRAME LIN □ID QUAI	IE 1										2	_	XX
			1 1	0 CL-103										OF	5334	뷥
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					CUSTOMER NAME:		PROJECT NAME:	ROJECT LOCATIVE	**************************************	SUSTOMER PHON		/ <u>?</u> Z		SHEET NUMBER:	MBER	النا
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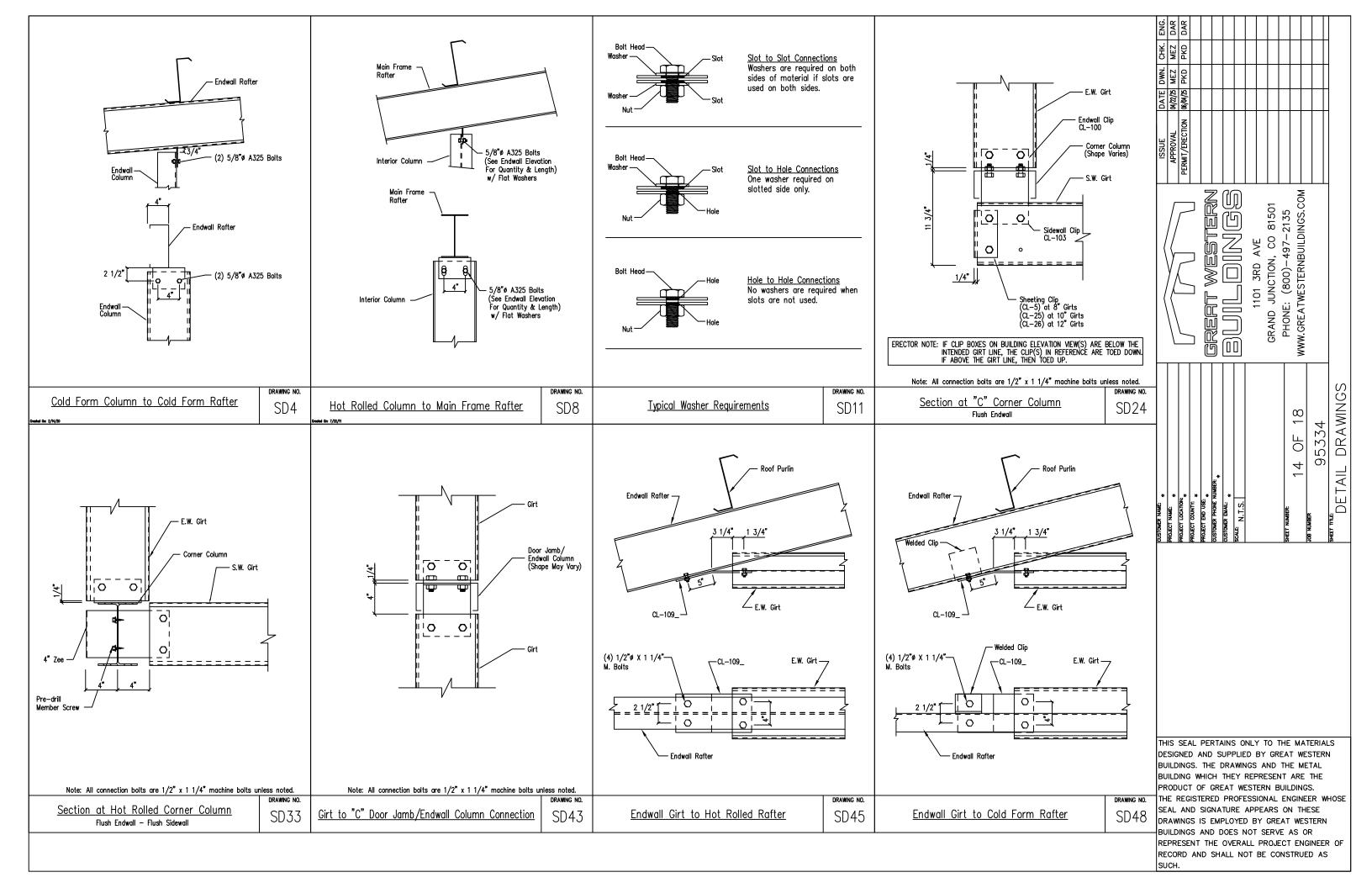


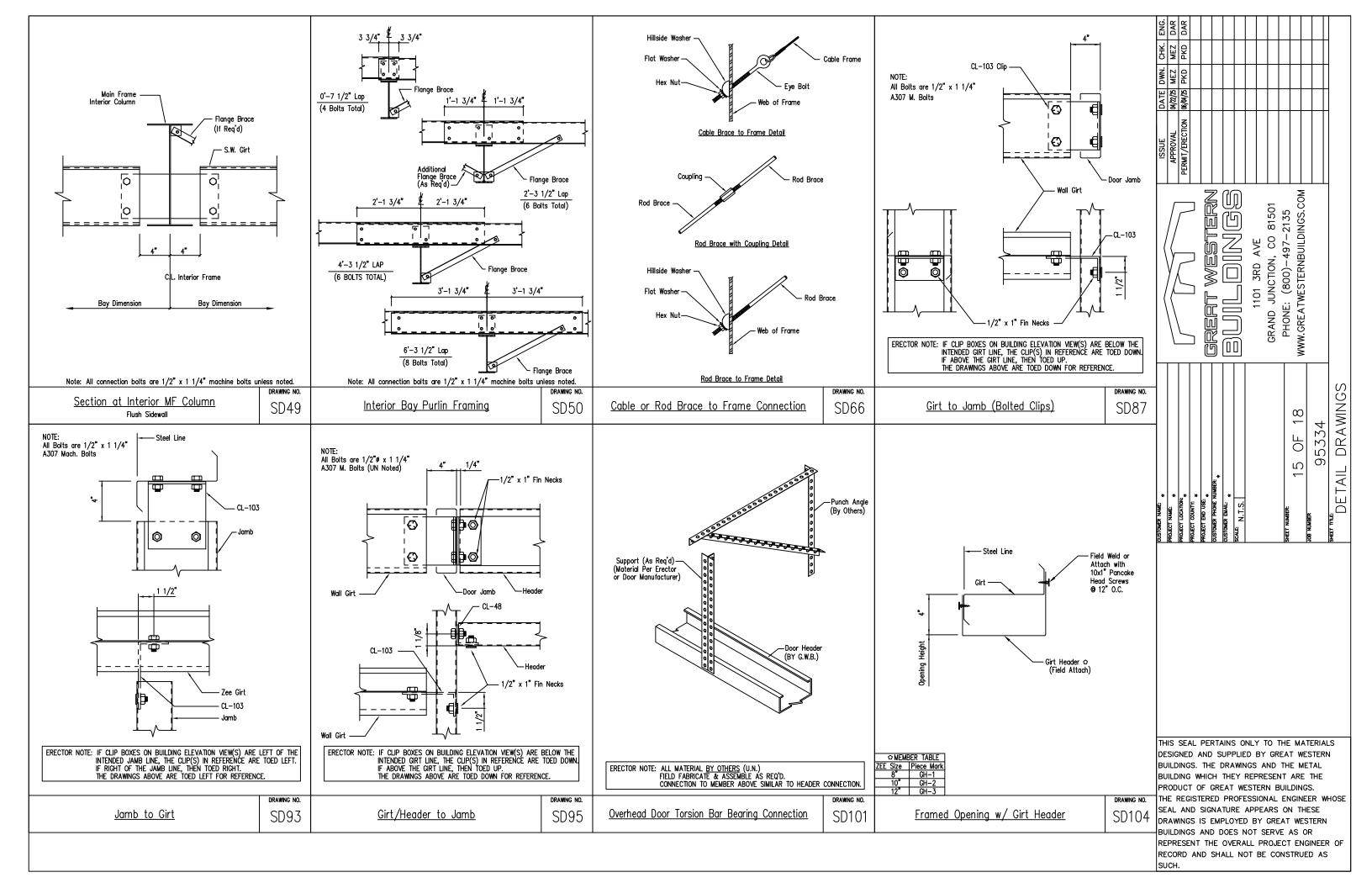
ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 GA. PBR - KOKO BROWN

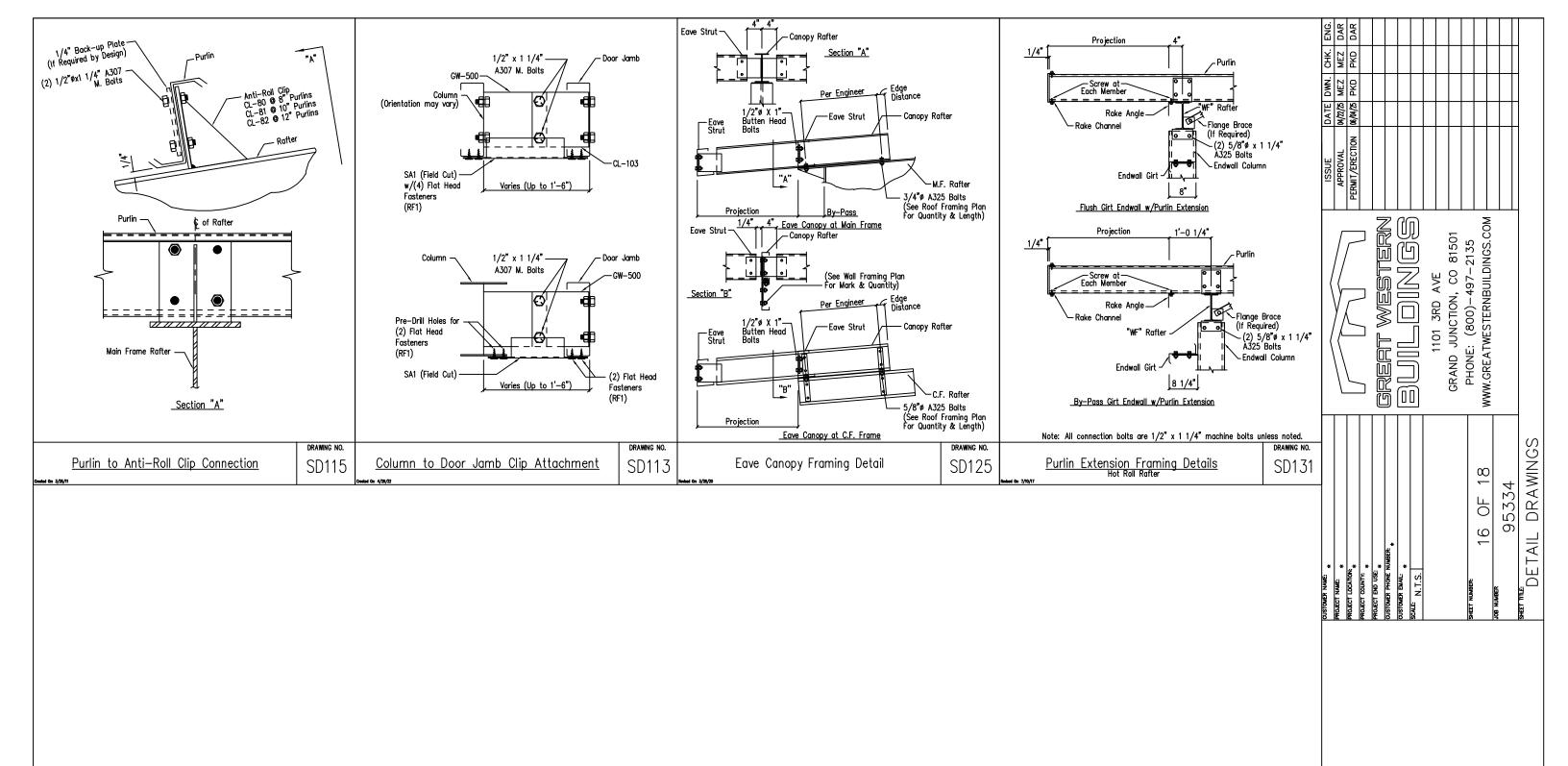
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2	4	FL-60 FL-48 FL-55	10'-2" 1'-2" 11'-11"		TD:	36	\vdash	MEZ	_		1		H				
2 3 4 5	2	FL-48	11 -11 12'-2" 12'-7" 12'-4"		TD:	51	DATE D	04/22/25 M	- F		+	+	\vdash			++	
6	1	FL-55 FL-52	12'-4"		TD:	52 52	Ā				\dashv	+	\vdash			+	
BOLT TABLE FRAME LINE 5								NAL.	PERMIT/ERECTION								
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JAMBS/RÁFTER 2 A325 5/8" 1 1/2" MEMBER TABLE								Γ	\supset			Ŋ		5	PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM		
		LINE 5 MARK	PART	lı.	.ENGTH		┨,	//		į] [<u>P</u>		81501	PHONE: (800)-497-2135.		
	1	EB-4	8x25C16	2	ENGIH 2'-2 15/ 2'-2 15/ 3'-9 7/ 7'-0 1/ 7'-0 1/ 22'-1 11, 2'-7 7/ 2'-7 5/ 4'-7 5/ 4'-7 5/ 8'-8 1/	16"	$\dagger \langle$	_	_		= c) (\mathbb{Z}	AVE	GRAND JUNCTION, CO	197– 3UILD		
	1 1	EB-6 EC-5	8x25C16 8x25C16	1	2-2 15/ 3-9 7/	16" 16"	\	1/	لـــا		֓֞֟֟֝֟֝֟֝֟֓֓֓֟֟֝֓֓֟֟֟֝֟֓֟֟֟֝֟֟֟ ֓֞	\equiv	3RD AVE	lon,	0)-4 ERNB		
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	1	ER-1 ER-2	8X35C12 8X35C12		22'-1 11, 22'-1 11,	/16" /16"		$\left\langle \cdot \right\rangle$					1	5	ONE: EATV		SHEETING
	1	DJ-3 DH-1	8x25C16	1	4'-0 1/ 1'-11 1/	2" ′2"		//	$\overline{}$			╣		GRAN	PHC W.GR		뵜
	2	G-7 G-8	8X25Z16 8X25Z16	$\frac{1}{7}$	2'-7 7/ 7'-9 5/8	8" 3"	L	_			<u>J</u>				 ✓		8
	1 2 1	G-9 G-10	8X25Z16 8X25Z16	1	4'-7 5/ 4'-7 5/	8" 8"											
	1	G-11 CB-1	8X25Z16 CB0250	8	3'-1 1/2 21'-1 1/:	" 4"											
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