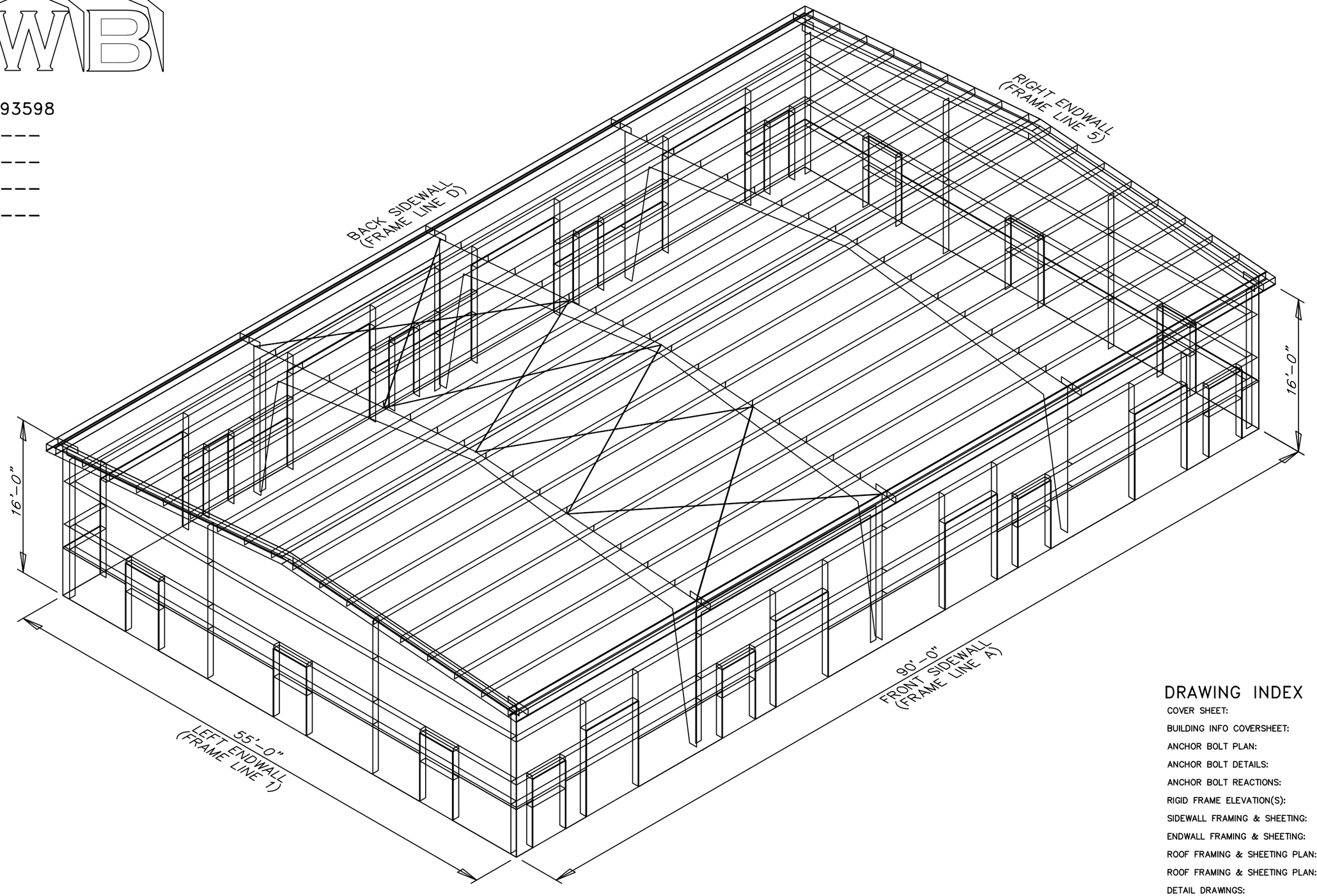
JOB NUMBER: 93598

PROJECT NAME: ---

PROJECT LOCATION: ---

PROJECT LOCATION: ---

PROJECT COUNTY: ---



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GENERAL NOTES

1.1 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3".

1.2 MATERIALS

	ASTM DESIGNATION	MIN. YIELD STRENGTH
HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36 KSI
STEEL PIPES	A500	Fy = 42 KSI
STRUCTURAL TUBING	A500	Fy = 42 KSI
STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55 KSI
COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55 KSI
ROOF & WALL SHEETS	A792/A653	Fy = 50, 80 KSI
CABLE BRACE	A475 – TYPE 1	EXTRA HIGH STRENGTH
ROD BRACE	A36	Fy = 36 KSI
		MIN. TENSILE STRENGTH
MACHINE BOLTS & NUTS	A307	Fu = 60 KSI
HIGH STRENGTH BOLTS (1"ø & LESS)	A325–TYPE 1	Fu = 120 KSI
HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø)	A325–TYPE 1	Fu = 105 KSI
ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)	A36/A307/F1554	Fu = 60 KSI

1.3 PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS:
SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'–1 1/4" A307 EXCEPT :
A) ENDWALL RAFTER SPLICE – 5/8"ø x 0'–1 3/4" A325–N
B) ENDWALL COLUMN TO RAFTER CONNECTION – (SEE WALL ELEVATION)
C) MAIN FRAME CONNECTIONS – SEE CROSS SECTION
D) FLANGE BRACECONNECTIONS – 1/2"ø x 0'–1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325–N UNLESS SPECIFICALLY NOTED OTHERWISE.
HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN–OF–THE–NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325–N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
INSIDE– UNDER ROOF PANELS & BASE OF WALL PANELS
OUTSIDE – BETWEEN ROOF PANELS & RIDGE CAP
– BETWEEN WALL PANELS & EAVE/GABLE TRIM

1.8 ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS M.B.S. SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING SUCH.

2.2 METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.) STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE, METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

2.3 IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

2.4 APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

2.5 ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/ END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE, LATEST EDITION)

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)

2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE.
UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 WARNING IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSIVE EFFECTS ON THE GALVALUME ALLOY COATING WHEN THEY ARE IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN–OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.

2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SRTONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI–A8–2

BUILDING LOADS

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

DESIGN CODE / WIND CODE	: IBC–15
OCCUPANCY / RISK CATEGORY	: II–Normal
ENCLOSURE	: Enclosed
ROOF DEAD LOAD (D) (PSF)	: 2.00
ROOF COLLATERAL LOAD (C) (PSF)	: 3.00
WIND LOAD	
ULTIMATE WIND SPEED, (VULT) (MPH)	: 130.00
WIND EXPOSURE CATEGORY	: C
INTERNAL PRESSURE COEFFICIENT, (GCpi)	: 0.18/–0.18
WALL PANEL DESIGN WIND PRESSURE (PSF)	: 34.16/–37.00
WIND ENCLOSURE CLASSIFICATION	: Enclosed
LIVE LOAD	
PRIMARY FRAMING (PSF)	: 20.00
TRIB. AREA REDUCTION	: No
SECONDARY FRAMING (PSF)	: 20.00
SNOW LOAD	
GROUND SNOW LOAD, (Pg) (PSF)	: 30.00
ROOF SNOW LOAD, (Pf) (PSF)	: 30.00
SNOW EXPOSURE FACTOR, (Ce)	: 1.00
SNOW IMPORTANCE FACTOR, (Is)	: 1.00
THERMAL FACTOR, (Ct)	: 1.00
SEISMIC LOAD	
SEISMIC IMPORTANCE FACTOR, (Ie)	: 1.00
SITE CLASSIFICATION	: D
SPECTRAL RESPONSE ACCELERATION	: Ss = 0.185 :S1 = 0.059
SPECTRAL RESPONSE COEFFICIENTS	: Sds = 0.197 :Sd1 = 0.094
SEISMIC DESIGN CATEGORY	: B
BASIC SEISMIC FORCE RESISTING SYSTEM	: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR RESISTANCE : RIGID FRAMES (OMF) : BRACED FRAMES (OCBF/OMF)
TOTAL DESIGN BASE SHEAR, (V) (KIPS)	: LONGITUDINAL = 2.89 : TRANSVERSE = 2.74
RESPONSE MODIFICATION FACTORS, (R)	: RIGID FRAMES = 3.00 Ω = 2.50 : SW WIND BENT = 3.00 Ω = 2.50
SEISMIC RESPONSE COEFFICIENTS, (Cs)	: RIGID FRAMES = 0.0658 : SW WIND BENT = 0.0658
ANALYSIS PROCEDURE USED	: EQUIVALENT LATERAL FORCE PROCEDURE
OTHER LOADS/REQUIREMENTS	

BUILDING DESCRIPTION:

WIDTH (FT)	: 55.00
LENGTH (FT)	: 90.00
EAVE HEIGHT AT BSW (FT)	: 16.00
EAVE HEIGHT AT FSW (FT)	: 16.00
ROOF SLOPE AT BSW	: 1.0:12
ROOF SLOPE AT FSW	: 1.0:12
BAY SPACING (FT)	: 4 AT 22.50

COVERING AND TRIMS:

ROOF PANELS & TRIMS	
PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: CHARCOAL GRAY
TRIM COLORS	
GABLE/EAVE	: CHARCOAL GRAY
EAVE GUTTER	: CHARCOAL GRAY
WALL PANELS & TRIMS	
PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: ASH GRAY
TRIM COLORS	
CORNER	: CHARCOAL GRAY
FRAMED OPENING	: CHARCOAL GRAY
DOWNSPOUTS	: CHARCOAL GRAY
BASE	: CHARCOAL GRAY
INSULATION	
ROOF INSULATION	: 9 1/2" (R–30) SKYLINER
WALL INSULATION	: 8" (R–25) SKYLINER

WAINSCOT PANELS & TRIMS	
PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: CHARCOAL GRAY
TRIM COLORS	: CHARCOAL GRAY

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

GWB
3033 S. PARKER RD 12 FLOOR
AURORA. CO 80014
PHONE: (800)–497–2135
WWW.GREATWESTERNBUILDINGS.COM

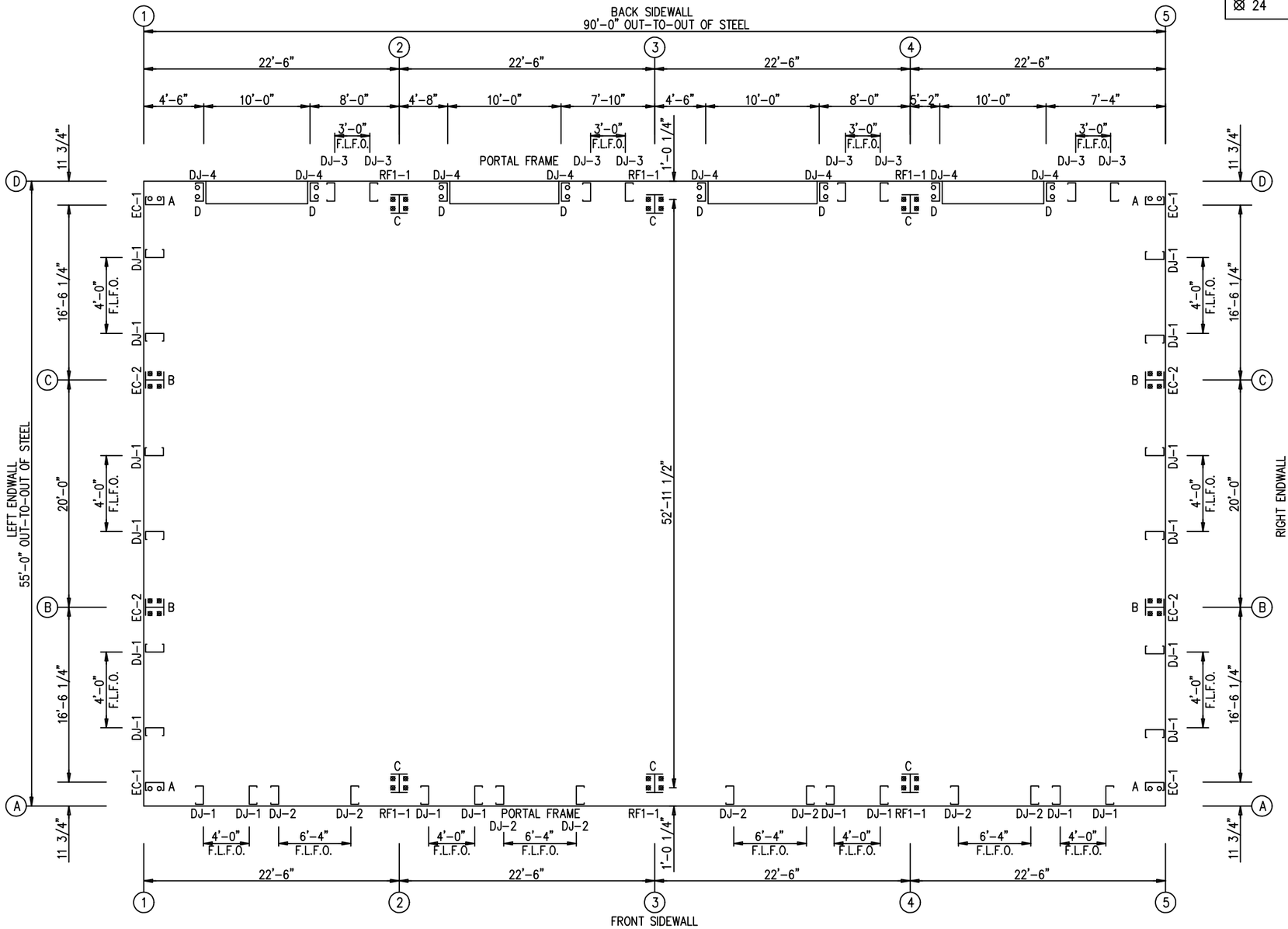
CUSTOMER NAME:
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 2 OF 18
JOB NUMBER 93598
SHEET TITLE: BUILDING INFO COVERSHEET

THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS.
THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

ANCHOR BOLT SUMMARY

	QTY	LOCATE	DIA (in)	TYPE
○	16	JAMB	5/8"	A307
⊠	16	ENDWALL	3/4"	A307
○	8	ENDWALL	5/8"	A307
⊠	24	FRAME	3/4"	A307



ANCHOR BOLT PLAN

NOTE: ALL BASE PLATES @ 100'-0" (U.N.)
F.L.F.O.= FIELD LOCATED FRAME OPENING

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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CUSTOMER NAME:
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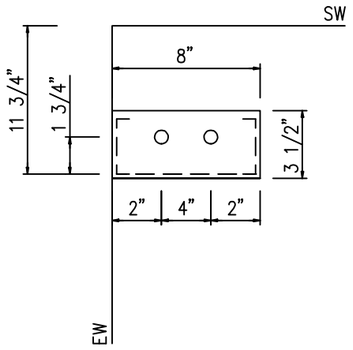
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

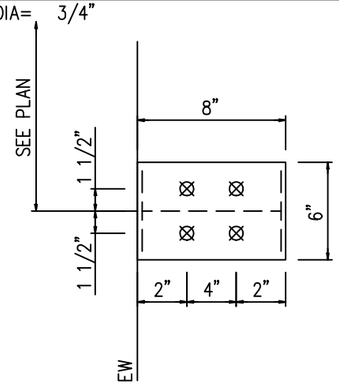
SHEET NUMBER: 3 OF 18
JOB NUMBER: 93598
SHEET TITLE: ANCHOR BOLT PLAN

DIA= 5/8"



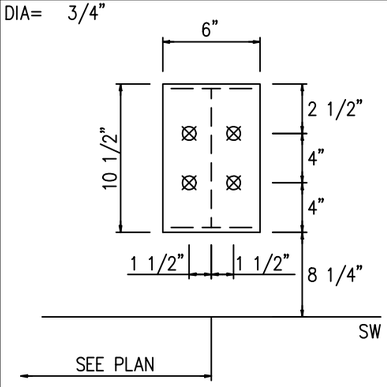
DETAIL A

DIA= 3/4"



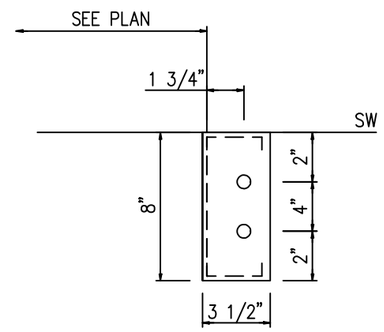
DETAIL B

DIA= 3/4"



DETAIL C

DIA= 5/8"



DETAIL D

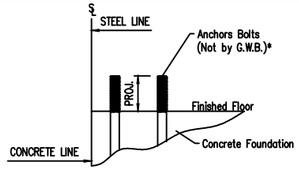
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

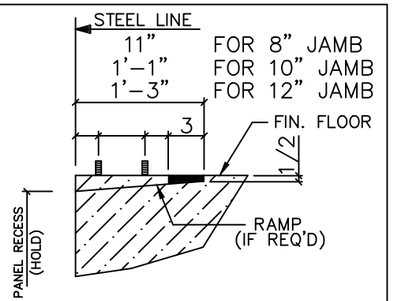
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	2 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/8"	3 1/2"
1 1/4"	3 1/2"



CONCRETE NOTCH AND ANCHOR BOLT PROJECTION



SLOPE CONCRETE TO KEEP WATER OUT OF BUILDING

CONCRETE DETAIL AT OVERHEAD DOOR

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/16/11	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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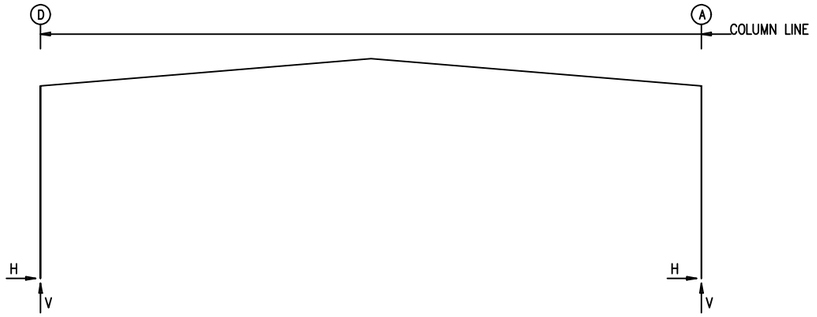
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 4 OF 18
JOB NUMBER 93598
SHEET TITLE: ANCHOR BOLT DETAILS

FRAME LINES: 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) QTY	DIA	Base_Plate(in) Width	Length	Thick	Grout (in)
2*	D	1	14.2	23.4	2	−6.4	−9.2	4	0.750	6.000	10.50	0.500	0.0
					4	−2.4	−10.2						
2*	A	3	6.4	−9.2	1	−14.2	23.4	4	0.750	6.000	10.50	0.500	0.0
		1	−14.2	23.4	5	2.4	−10.2						

2* FRAME lines: 2 3 4

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 55.00
Length (ft)	= 90.00
Eave Height (ft)	= 16.00/16.00
Roof Slope (rise/12)	= 1.0:12/1.0:12
Dead Load (psf)	= 2.00
Collateral Load (psf)	= 3.00
Live Load (psf)	= 20.00
Snow Load (psf)	= 30.00
Ultimate Wind Speed (mph)	= 130.00
Wind Code	= IBC−15
Exposure	= C
Closed/Open	= Enclosed
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= B
Seismic Coeff (Fa*Ss)	= 0.30

ID	Description
1	Dead+Collateral+Snow+Slide_Snow
2	0.6Dead+0.6Wind_Left1
3	0.6Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Long1L
5	0.6Dead+0.6Wind_Long2L
6	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
7	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
8	Dead+Collateral+E1UNB_SL_L
9	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
10	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
11	Dead+Collateral+E1UNB_SL_R
12	Dead+Collateral+E2UNB_SL_L
13	Dead+Collateral+E2UNB_SL_R

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1						53	10	
F_SW	A	2,3	2.9	3.8	0.7	0.9			(b)
R_EW	5						53	10	
B_SW	D	2,3	2.9	3.8	0.7	0.9			(b)

(b)Wind bent in bay, base above finish floor

Reactions for seismic represent shear force, Eh

RIGID FRAME: BASIC COLUMN REACTIONS (k)

FRAME Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	D	1.1	2.2	1.2	2.0	7.9	12.8	11.9	19.2	−11.7	−17.5	−4.5	−11.5
2*	A	−1.1	2.2	−1.2	2.0	−7.9	12.8	−11.9	19.2	4.5	−11.5	11.7	−17.5

FRAME Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	D	−9.3	−10.5	−2.0	−4.4	−5.1	−19.1	−5.6	−16.2	−0.3	−0.2	0.3	0.2
2*	A	2.0	−4.4	9.3	−10.5	5.6	−16.2	5.1	−19.1	−0.3	0.2	0.3	−0.2

FRAME Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	D	0.0	−0.9	7.9	12.8	9.3	18.4	9.3	10.1
2*	A	0.0	−0.9	−7.9	12.8	−9.3	10.1	−9.3	18.4

2* FRAME lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
1	D	0.3	0.3	2.0	3.0	−3.9	−2.0	−2.8	−0.9	−1.8	2.1	−3.4	−2.0
1	C	0.9	0.8	5.1	7.6	−9.1	−5.7	−6.3	−2.9	−4.1	4.5	−8.9	−5.5
1	B	0.9	0.8	5.1	7.6	−5.7	−9.1	−2.9	−6.3	−4.1	4.5	−5.5	−8.9
1	A	0.3	0.3	2.0	3.0	−2.0	−3.9	−0.9	−2.8	−1.8	2.1	−2.0	−3.4

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	Seis Long Vert	MIN_SNOW Horz	MIN_SNOW Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	D	0.0	0.0	0.0	0.0	2.0	0.0	3.1	0.0	0.8
1	C	0.0	0.0	0.0	0.0	5.1	0.0	8.9	0.0	2.9
1	B	0.0	0.0	0.0	0.0	5.1	0.0	2.9	0.0	8.9
1	A	0.0	0.0	0.0	0.0	2.0	0.0	0.8	0.0	3.1

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
5	A	0.3	0.3	2.0	3.0	−3.9	−2.0	−2.8	−0.9	−1.8	2.1	−3.4	−2.0
5	B	0.9	0.8	5.1	7.6	−9.1	−5.7	−6.3	−2.9	−4.1	4.5	−8.9	−5.5
5	C	0.9	0.8	5.1	7.6	−5.7	−9.1	−2.9	−6.3	−4.1	4.5	−5.5	−8.9
5	D	0.3	0.3	2.0	3.0	−2.0	−3.9	−0.9	−2.8	−1.8	2.1	−2.0	−3.4

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	Seis Long Vert	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
5	A	0.0	0.0	0.0	0.0	2.0	0.0	3.1	0.0	0.8
5	B	0.0	0.0	0.0	0.0	5.1	0.0	8.9	0.0	2.9
5	C	0.0	0.0	0.0	0.0	5.1	0.0	2.9	0.0	8.9
5	D	0.0	0.0	0.0	0.0	2.0	0.0	0.8	0.0	3.1

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) QTY	DIA	Base_Plate(in) Width	Length	Thick	Grout (in)
1	D	6	1.3	−2.1	7	−1.1	−1.9	2	0.625	3.500	8.000	0.250	0.0
		8	0.0	3.7	6	1.3	−2.1						
1	C	6	2.7	−4.9	7	−2.5	−4.8	4	0.750	6.000	8.000	0.375	0.0
		8	0.0	10.6	6	2.7	−4.9						
1	B	9	2.7	−4.9	10	−2.5	−4.8	4	0.750	6.000	8.000	0.375	0.0
		11	0.0	10.6	9	2.7	−4.9						
1	A	9	1.3	−2.1	10	−1.1	−1.9	2	0.625	3.500	8.000	0.250	0.0
		11	0.0	3.7	9	1.3	−2.1						
5	A	6	1.3	−2.1	7	−1.1	−1.9	2	0.625	3.500	8.000	0.250	0.0
		12	0.0	3.7	6	1.3	−2.1						
5	B	6	2.7	−4.9	7	−2.5	−4.8	4	0.750	6.000	8.000	0.375	0.0
		12	0.0	10.6	6	2.7	−4.9						
5	C	9	2.7	−4.9	10	−2.5	−4.8	4	0.750	6.000	8.000	0.375	0.0
		13	0.0	10.6	9	2.7	−4.9						
5	D	9	1.3	−2.1	10	−1.1	−1.9	2	0.625	3.500	8.000	0.250	0.0
		13	0.0	3.7	9	1.3	−2.1						

ANCHOR BOLT SUMMARY

QTY	LOCATE	DIA (in)	TYPE
○ 16	JAMB	5/8"	A307
☒ 16	ENDWALL	3/4"	A307
○ 8	ENDWALL	5/8"	A307
☒ 24	FRAME	3/4"	A307

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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SCALE: N.T.S.

SHEET NUMBER: 5 OF 18
JOB NUMBER 93598
SHEET TITLE: ANCHOR BOLT REACTIONS

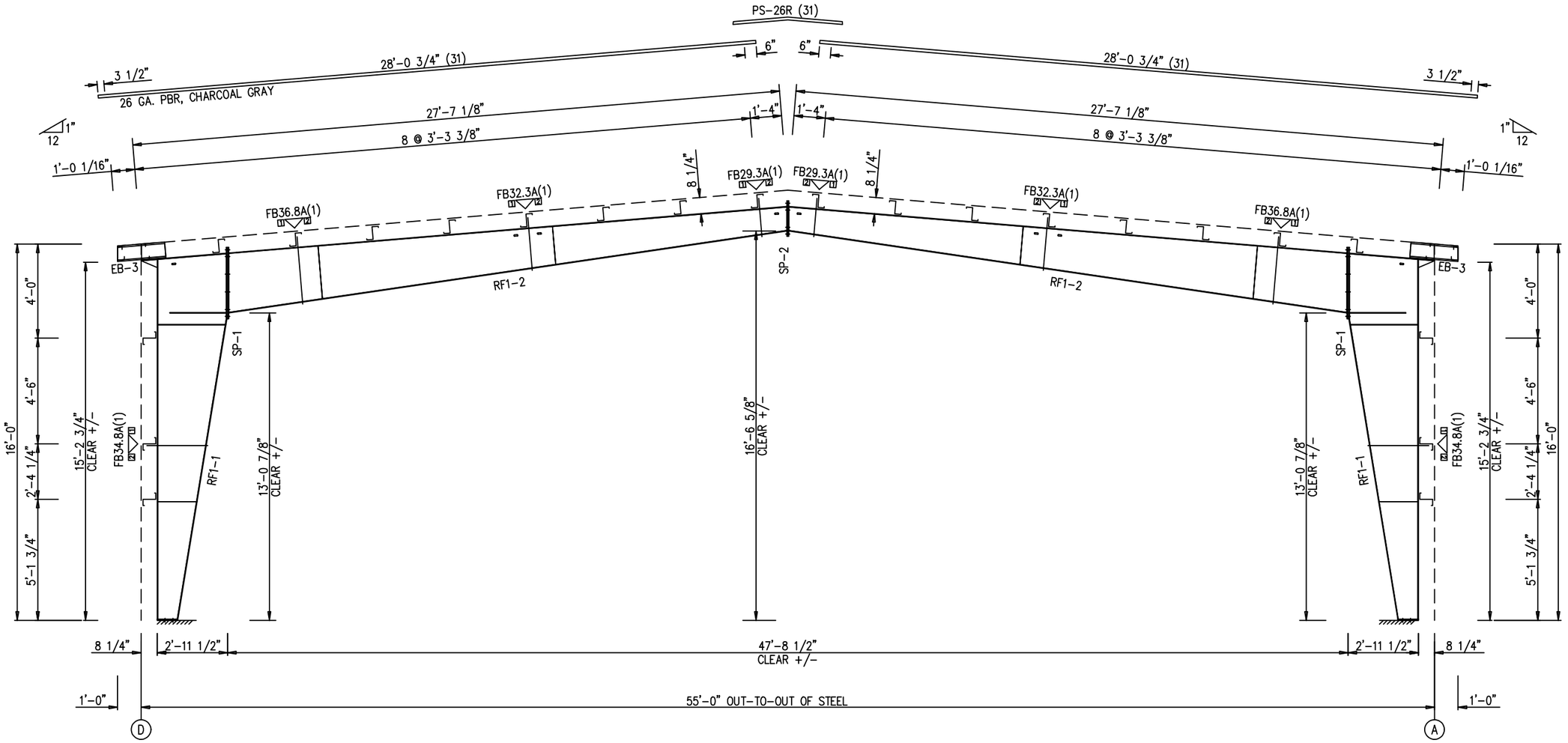
SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	4	4	A325	3/4"	2 1/4"
SP-2	4	4	0	A325	5/8"	2"

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – 2X2X14Ga

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	10.0/19.8	0.164	5'-0"	5 x 1/4" x 15'-3 11/16"	5 x 1/4" x 12'-11 5/16"
	19.8/34.5	0.188	7'-6 5/16"	5 x 1/4" x 3'-7 5/8"	
	34.5/35.0	0.250	3'-0 1/4"		
RF1-2	30.0/27.0	0.250	4'-0 1/4"	5 x 1/4" x 23'-9 11/16"	5 x 1/4" x 23'-11 3/4"
	27.0/19.5	0.188	10'-0"		
	19.5/12.0	0.135	10'-0"		

MEMBER SIZE TABLE		
Mark	Part	Length
EB-3	W8X10	2'-0 3/4"

CONNECTION PLATES	
☐ ID	Mark/Part
1	CL-86
2	CL-85



RIGID FRAME ELEVATION: FRAME LINES 2 3 4

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	15/76/11	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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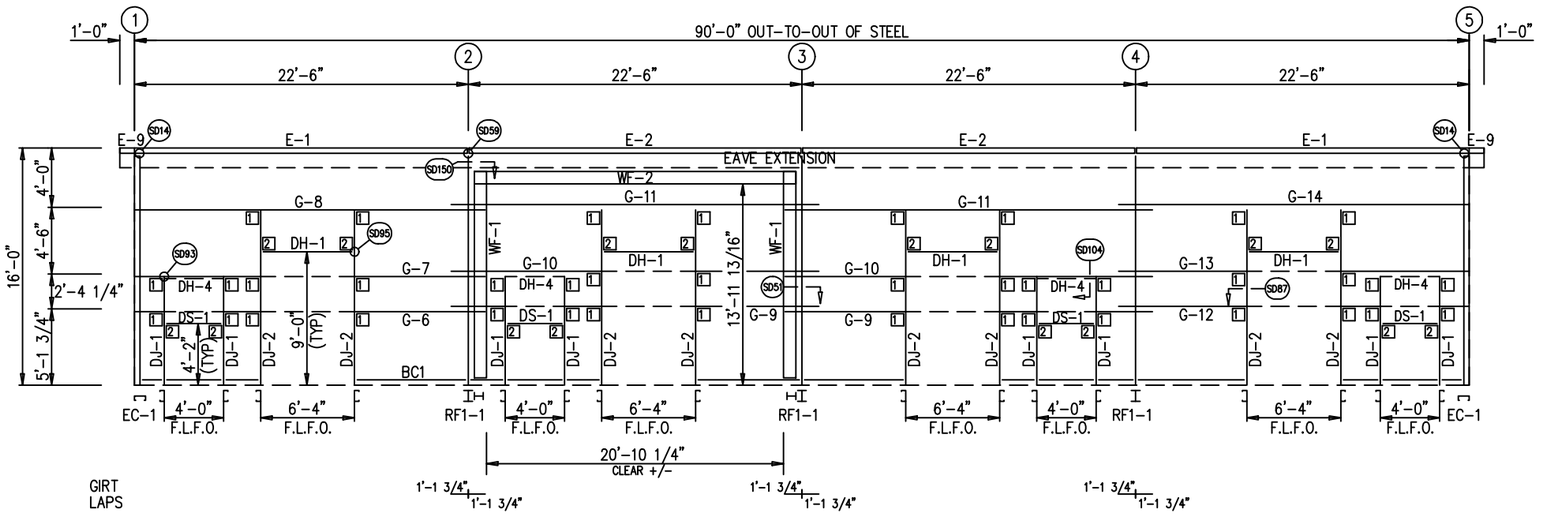
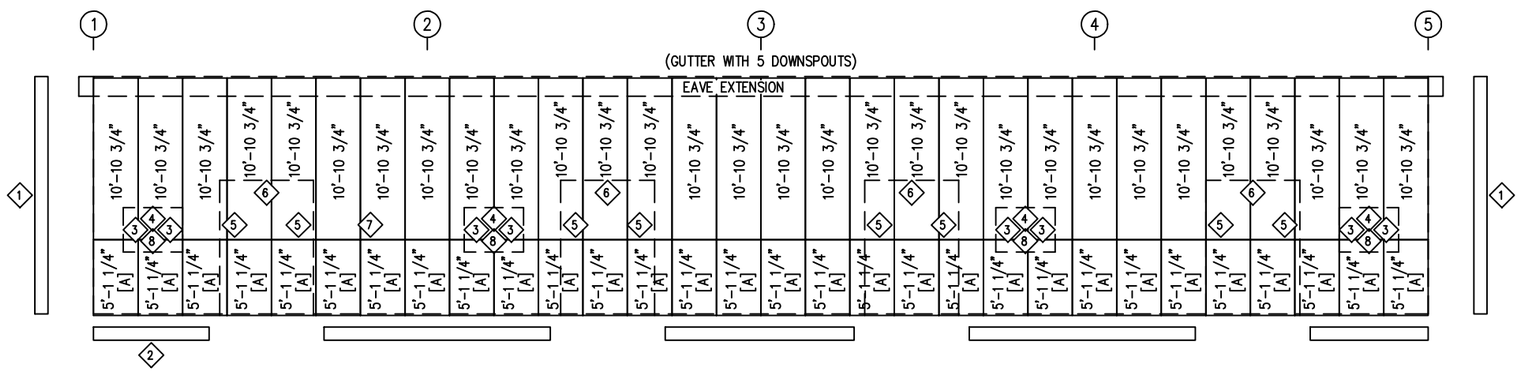
SCALE: N.T.S.

SHEET NUMBER: 6 OF 18
JOB NUMBER 93598
SHEET TITLE: RIGID FRAME ELEVATION



SIDEWALL FRAMING: FRAME LINE A

NOTE: F.L.F.O. = FIELD LOCATED FRAME OPENING



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 GA. PBR – ASH GRAY

[A] PANELS: 26 GA. PBR – CHARCOAL GRAY

TRIM TABLE
FRAME LINE A

◇ID	QUAN	PART	LENGTH	DETAIL
1	2	FL-10	16'-0"	TD40
2	9	FL-60	10'-2"	TD75
3	8	FL-48	3'-4"	TD51
4	4	FL-52	4'-4"	TD52
5	8	FL-48	9'-2"	TD51
6	4	FL-52	6'-8"	TD52
7	9	FL-237	10'-2"	TD199
8	4	FL-50	4'-4"	TD52

BOLT TABLE
FRAME LINE A

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	5/8"	1 3/4"
WF-1 – RF1-1	8	A325	5/8"	1 1/2"

MEMBER TABLE
FRAME LINE A

QUAN	MARK	PART	LENGTH
2	WF-1	W10X12	14'-4"
1	WF-2	W10X22	20'-9 7/16"
8	DJ-1	8x25C16	7'-2"
8	DJ-2	8x25C16	11'-8"
4	DH-1	8x25C16	6'-3 1/2"
4	DH-4	GH-1	4'-0"
4	DS-1	8x25C16	3'-11 1/2"
2	E-1	L08E16-1	21'-5 13/16"
2	E-2	L08E16-1	21'-9 13/16"
2	E-9	L08E16-1	11 11/16"
1	G-6	8X35Z16	23'-7 1/2"
1	G-7	8X25Z16	23'-7 1/2"
1	G-8	8X25Z12	23'-7 1/2"
2	G-9	8X35Z16	24'-9 1/2"
2	G-10	8X25Z16	24'-9 1/2"
2	G-11	8X25Z14	24'-9 1/2"
1	G-12	8X35Z16	23'-7 1/2"
1	G-13	8X25Z16	23'-7 1/2"
1	G-14	8X25Z12	23'-7 1/2"

CONNECTION PLATES
FRAME LINE A

□ID	QUAN	MARK
1	40	CL-103
2	16	CL-100

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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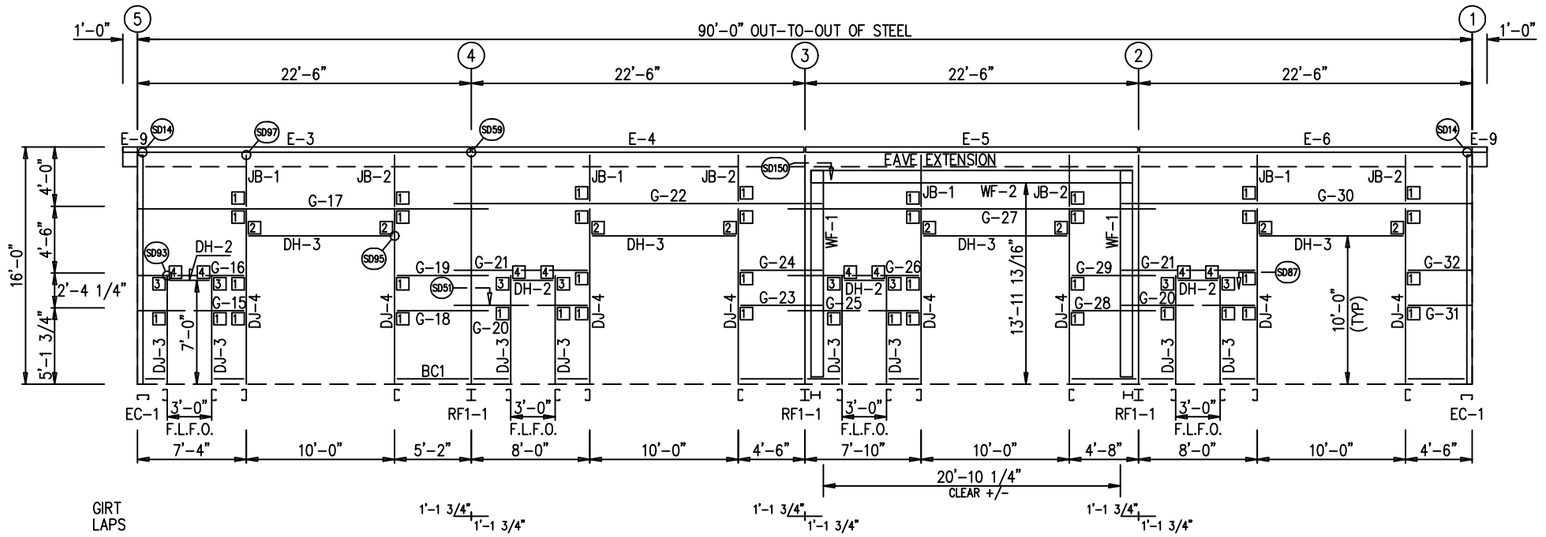
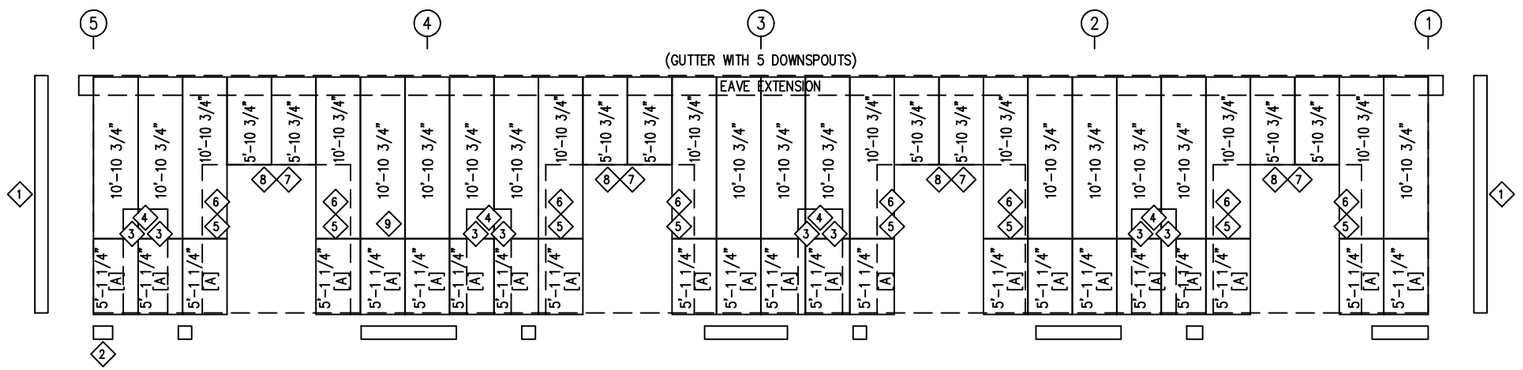
SCALE: N.T.S.
SHEET NUMBER: 7 OF 18
JOB NUMBER 93598
SHEET TITLE: SIDEWALL FRAMING & SHEETING



SIDEWALL FRAMING: FRAME LINE D

NOTE: F.L.F.O. = FIELD LOCATED FRAME OPENING



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 GA. PBR – ASH GRAY

[A] PANELS: 26 GA. PBR – CHARCOAL GRAY

TRIM TABLE
FRAME LINE D

◇ID	QUAN	PART	LENGTH	DETAIL
1	2	FL-10	16'-0"	TD40
2	5	FL-60	10'-2"	TD75
3	8	FL-48	7'-2"	TD51
4	4	FL-52	3'-4"	TD52
5	8	FL-55	10'-3"	TD51
6	8	FL-48	10'-2"	TD51
7	4	FL-55	10'-7"	TD52
8	4	FL-52	10'-4"	TD52
9	5	FL-237	10'-2"	TD199

BOLT TABLE
FRAME LINE D

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	5/8"	1 3/4"
WF-1 – RF1-1	8	A325	5/8"	1 1/2"

MEMBER TABLE
FRAME LINE D

QUAN	MARK	PART	LENGTH
2	WF-1	W10X12	14'-4"
1	WF-2	W10X22	20'-9 7/16"
8	DJ-3	8x25C16	7'-2"
8	DJ-4	8x25C16	11'-8"
4	DH-2	8x25C16	2'-11 1/2"
4	DH-3	8x25C16	9'-11 1/2"
1	E-3	L08E16-1	21'-5 13/16"
1	E-4	L08E16-1	21'-9 13/16"
1	E-5	L08E16-1	21'-9 13/16"
1	E-6	L08E16-1	21'-5 13/16"
2	E-9	L08E16-1	11 11/16"
1	G-15	8X35Z16	6'-11 11/16"
1	G-16	8X25Z16	6'-11 11/16"
1	G-17	8X25Z12	23'-7 1/2"
1	G-18	8X35Z16	5'-11 11/16"
1	G-19	8X25Z16	5'-11 11/16"
2	G-20	8X35Z16	8'-9 11/16"
2	G-21	8X25Z16	8'-9 11/16"
1	G-22	8X25Z14	24'-9 1/2"
1	G-23	8X35Z16	5'-3 11/16"
1	G-24	8X25Z16	5'-3 11/16"
1	G-25	8X35Z16	8'-7 11/16"
1	G-26	8X25Z16	8'-7 11/16"
1	G-27	8X25Z14	24'-9 1/2"
1	G-28	8X35Z16	5'-5 11/16"
1	G-29	8X25Z16	5'-5 11/16"
1	G-30	8X25Z12	23'-7 1/2"
1	G-31	8X35Z16	4'-1 11/16"
1	G-32	8X25Z16	4'-1 11/16"
4	JB-1	8x25C16	3'-0 3/8"
4	JB-2	8x25C16	3'-0 3/8"

CONNECTION PLATES
FRAME LINE D

□ID	QUAN	MARK
1	40	CL-103
2	8	CL-100
3	8	GW-500
4	8	CL-48

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	15/16/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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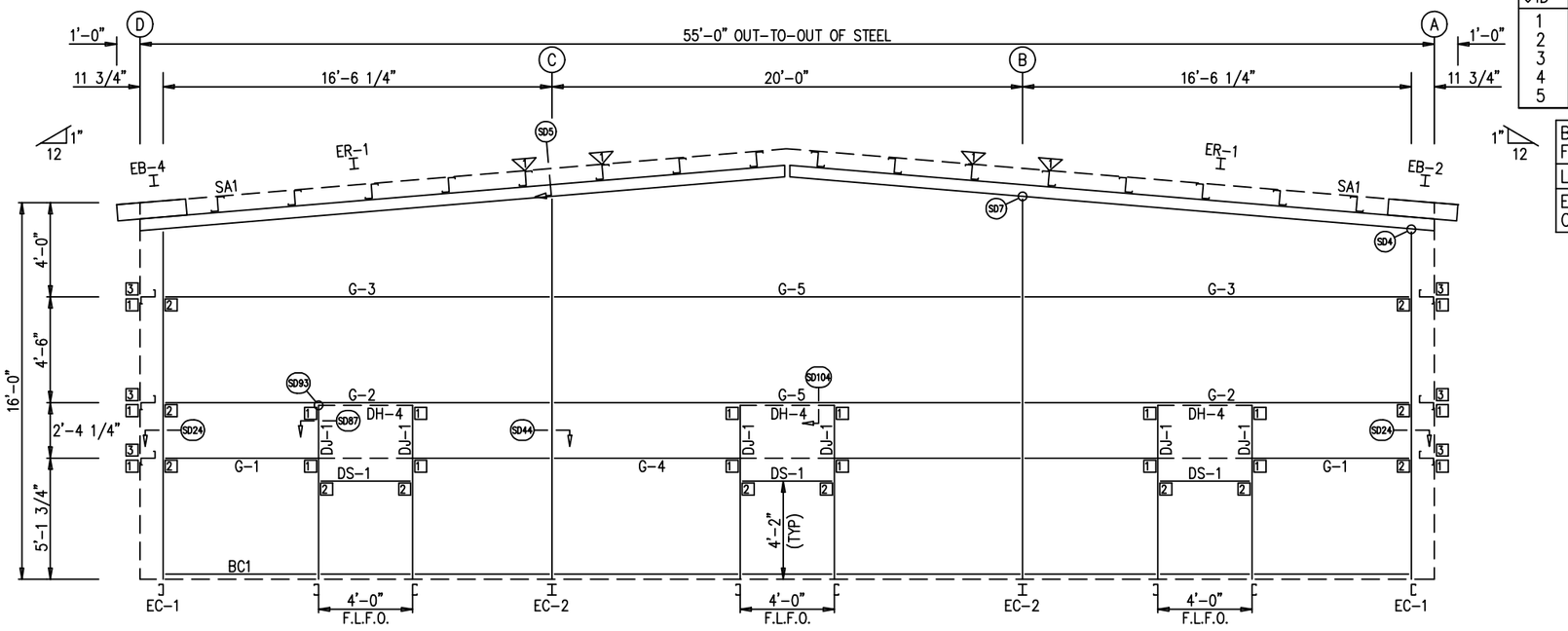
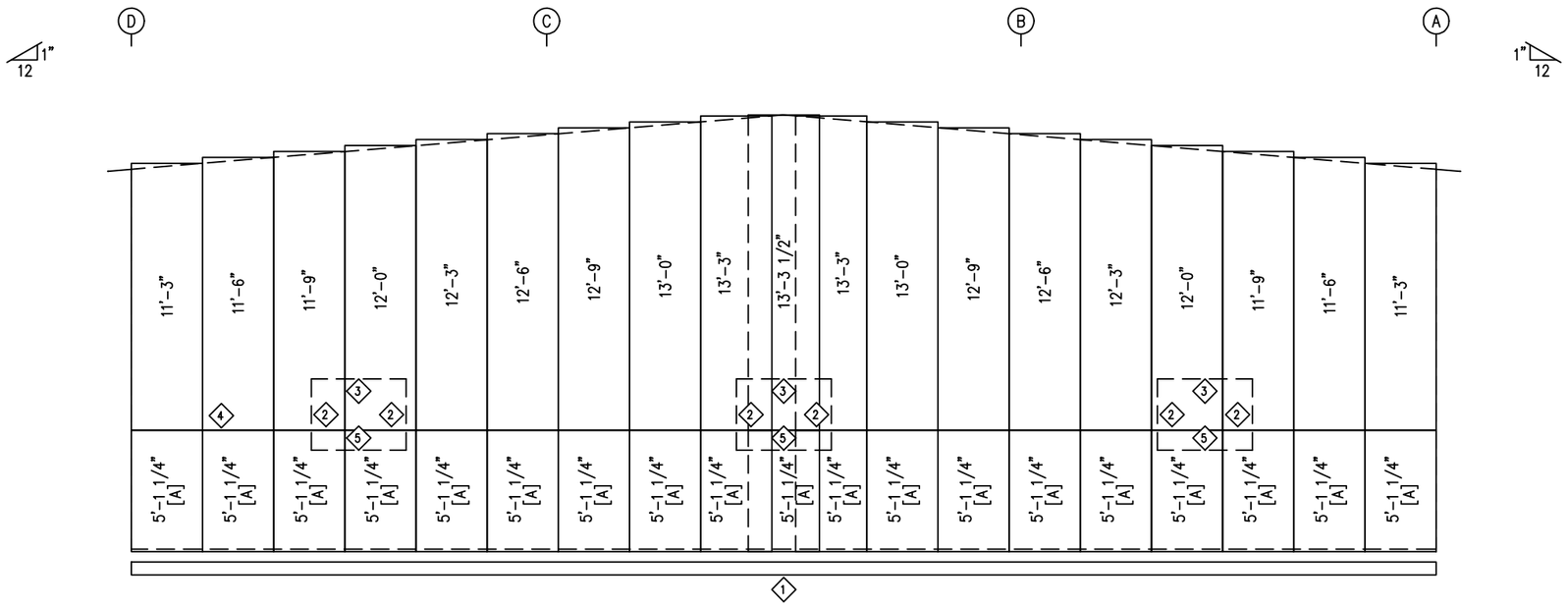
SCALE: N.T.S.

SHEET NUMBER: 8 OF 18
JOB NUMBER: 93598
SHEET TITLE: SIDEWALL FRAMING & SHEETING



ENDWALL FRAMING: FRAME LINE 1

NOTE: F.L.F.O.= FIELD LOCATED FRAME OPENING



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 GA. PBR – ASH GRAY

[A] PANELS: 26 GA. PBR – CHARCOAL GRAY

TRIM TABLE
FRAME LINE 1

◇ID	QUAN	PART	LENGTH	DETAIL
1	6	FL-60	10'-2"	TD75
2	6	FL-48	3'-4"	TD51
3	3	FL-52	4'-4"	TD52
4	6	FL-237	10'-2"	TD199
5	3	FL-50	4'-4"	TD52

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
COLUMNS/RAFTER	2	A325	5/8"	1 1/2"

MEMBER TABLE
FRAME LINE 1

QUAN	MARK	PART	LENGTH
1	EB-2	W8X10	2'-0 3/4"
1	EB-4	W8X10	2'-0 3/4"
2	EC-1	8x25C14	14'-8 1/16"
2	EC-2	W8X10	16'-0 9/16"
2	ER-1	W8X10	27'-6 7/8"
6	DJ-1	8x25C16	7'-2"
3	DH-4	GH-1	4'-0"
3	DS-1	8x25C16	3'-11 1/2"
2	G-1	8X35Z16	16'-1 15/16"
2	G-2	8X25Z14	16'-1 15/16"
2	G-3	8X25Z16	16'-1 15/16"
1	G-4	8X35Z16	19'-3 13/16"
2	G-5	8X25Z12	19'-3 13/16"

CONNECTION PLATES
FRAME LINE 1

□ID	QUAN	MARK
1	18	CL-103
2	12	CL-100
3	6	CL-5

FLANGE BRACE TABLE
FRAME LINE 1

▽ID	QUAN	MARK
1	4	FB28.3

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	15/16/11	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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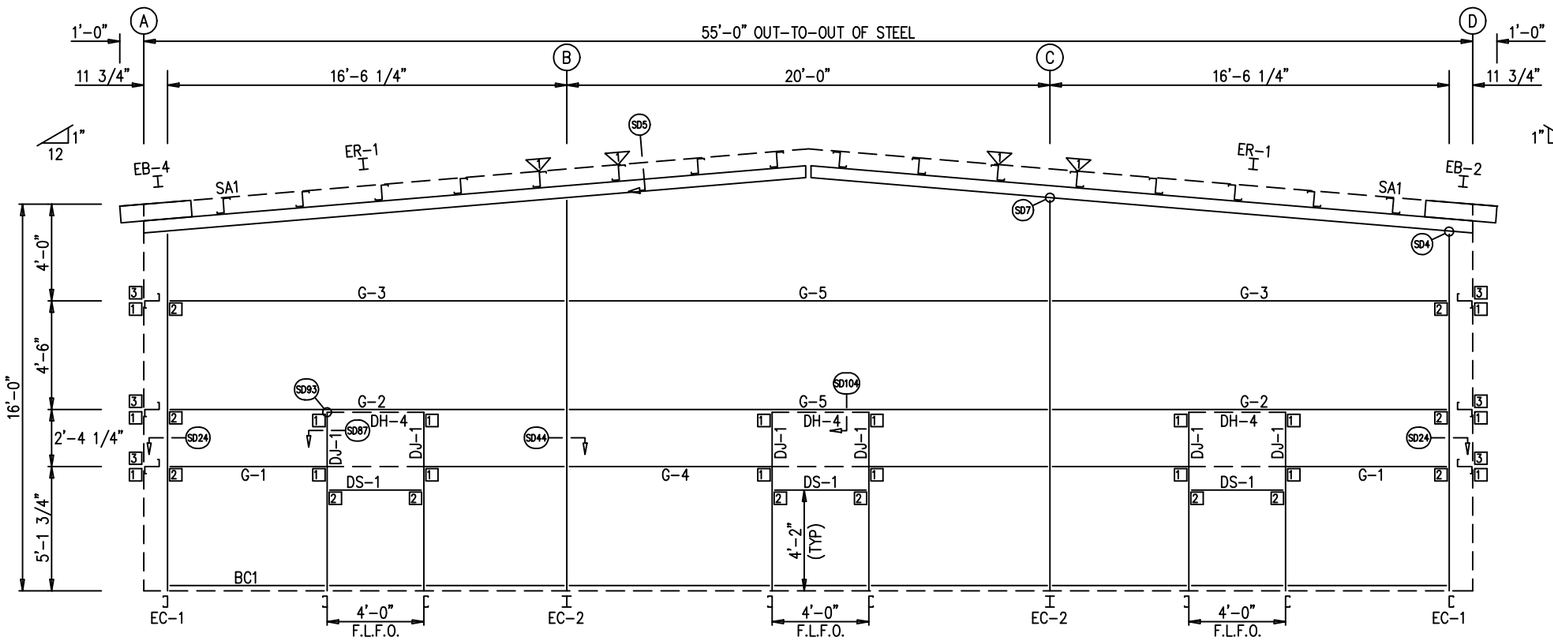
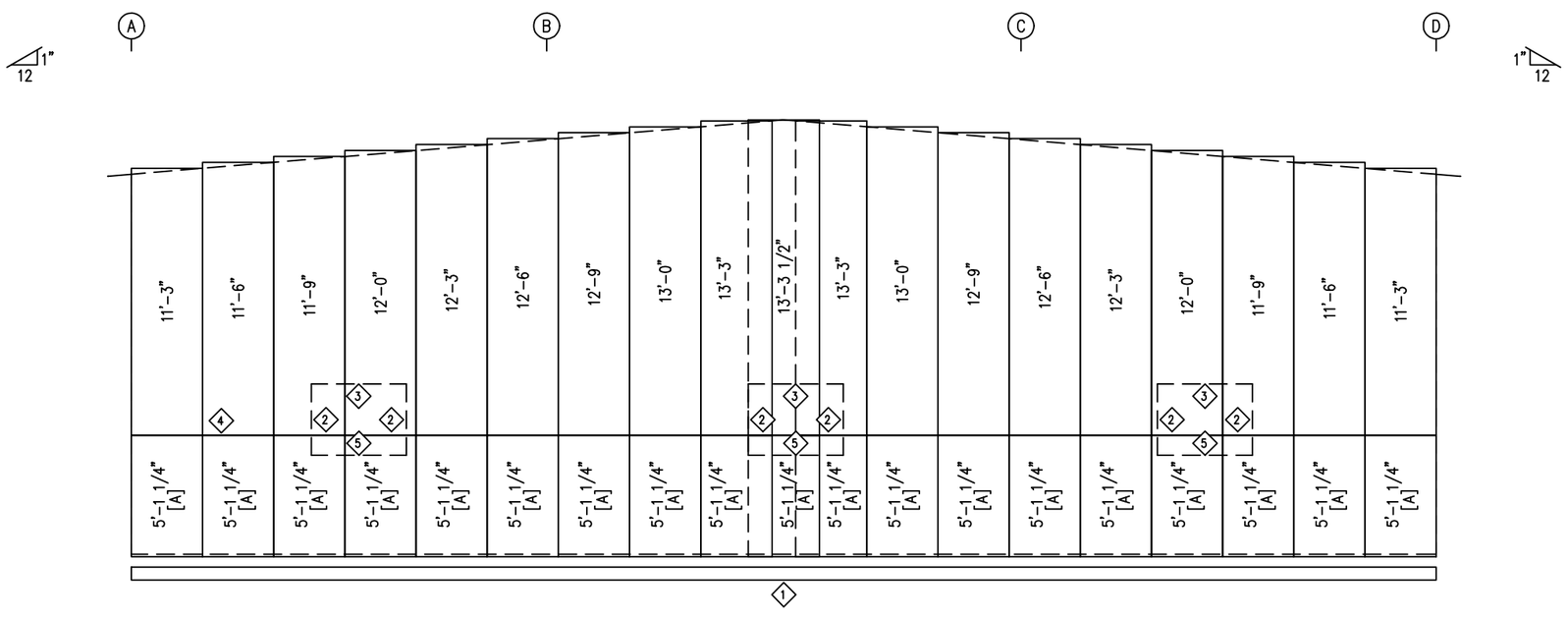
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 9 OF 18
JOB NUMBER: 935980
SHEET TITLE: ENDWALL FRAMING & SHEETING



ENDWALL FRAMING: FRAME LINE 5

NOTE: F.L.F.O.= FIELD LOCATED FRAME OPENING



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 GA. PBR – ASH GRAY

[A] PANELS: 26 GA. PBR – CHARCOAL GRAY

TRIM TABLE
FRAME LINE 5

◇ID	QUAN	PART	LENGTH	DETAIL
1	5	FL-60	10'-2"	TD75
2	6	FL-48	3'-4"	TD51
3	3	FL-52	4'-4"	TD52
4	6	FL-237	10'-2"	TD199
5	3	FL-50	4'-4"	TD52

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 1/2"
COLUMNS/RAFTER	2	A325	5/8"	1 1/2"

MEMBER TABLE
FRAME LINE 5

QUAN	MARK	PART	LENGTH
1	EB-2	W8X10	2'-0 3/4"
1	EB-4	W8X10	2'-0 3/4"
2	EC-1	8x25C14	14'-8 1/16"
2	EC-2	W8X10	16'-0 9/16"
2	ER-1	W8X10	27'-6 7/8"
6	DJ-1	8x25C16	7'-2"
3	DH-4	GH-1	4'-0"
3	DS-1	8x25C16	3'-11 1/2"
2	G-1	8X35Z16	16'-1 15/16"
2	G-2	8X25Z14	16'-1 15/16"
2	G-3	8X25Z16	16'-1 15/16"
1	G-4	8X35Z16	19'-3 13/16"
2	G-5	8X25Z12	19'-3 13/16"

CONNECTION PLATES
FRAME LINE 5

□ID	QUAN	MARK
1	18	CL-103
2	12	CL-100
3	6	CL-5

FLANGE BRACE TABLE
FRAME LINE 5

▽ID	QUAN	MARK
1	4	FB28.3

	ENG.	CHK.	DWN.	DATE
ISSUE	RTS	MEZ	MEZ	15/16/21
APPROVAL	RTS	CAF	AA	06/26/23
PERMIT	RTS	OGR	OGR	10/18/23
ERECTION				

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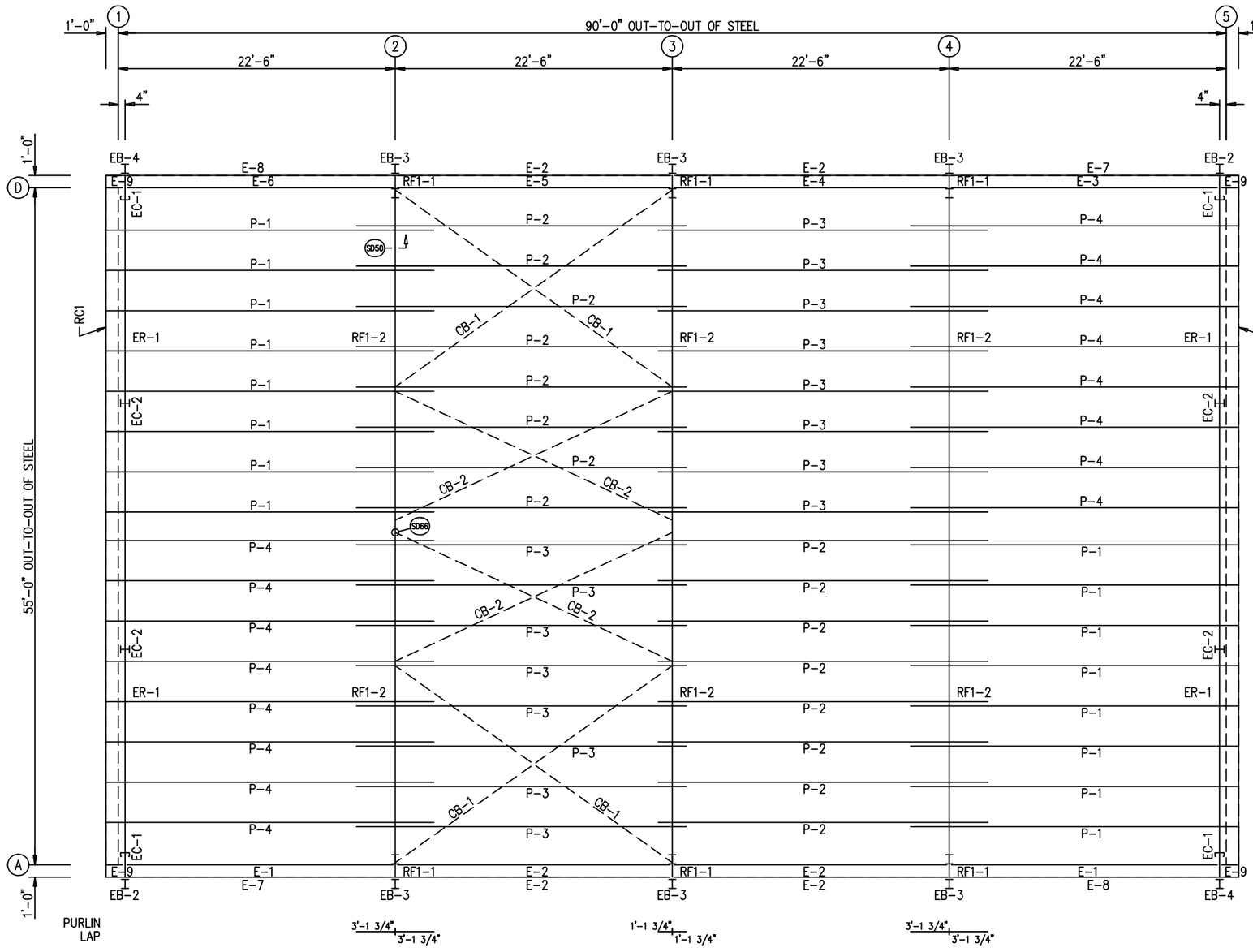
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 10 OF 18
JOB NUMBER 93598
SHEET TITLE: ENDWALL FRAMING & SHEETING



ROOF FRAMING PLAN

BOLT TABLE
ROOF PLAN

LOCATION	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	1 1/2"
EB-3	4	A325	5/8"	1 1/2"
EB-4	4	A325	5/8"	1 1/2"

MEMBER TABLE
ROOF PLAN

QUAN	MARK	PART	LENGTH
2	EB-2	W8X10	2'-0 3/4"
6	EB-3	W8X10	2'-0 3/4"
2	EB-4	W8X10	2'-0 3/4"
16	P-1	8X25Z14	26'-7 1/2"
16	P-2	8X25Z16	26'-9 1/2"
16	P-3	8X25Z16	26'-9 1/2"
16	P-4	8X25Z14	26'-7 1/2"
2	E-1	L08E16-1	21'-5 13/16"
6	E-2	L08E16-1	21'-9 13/16"
1	E-3	L08E16-1	21'-5 13/16"
1	E-4	L08E16-1	21'-9 13/16"
1	E-5	L08E16-1	21'-9 13/16"
1	E-6	L08E16-1	21'-5 13/16"
2	E-7	L08E16-1	23'-1 11/16"
2	E-8	L08E16-1	23'-1 11/16"
4	E-9	L08E16-1	11 11/16"
4	CB-1	CB0250	27'-3 3/4"
4	CB-2	CB0250	25'-2"

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	15/76/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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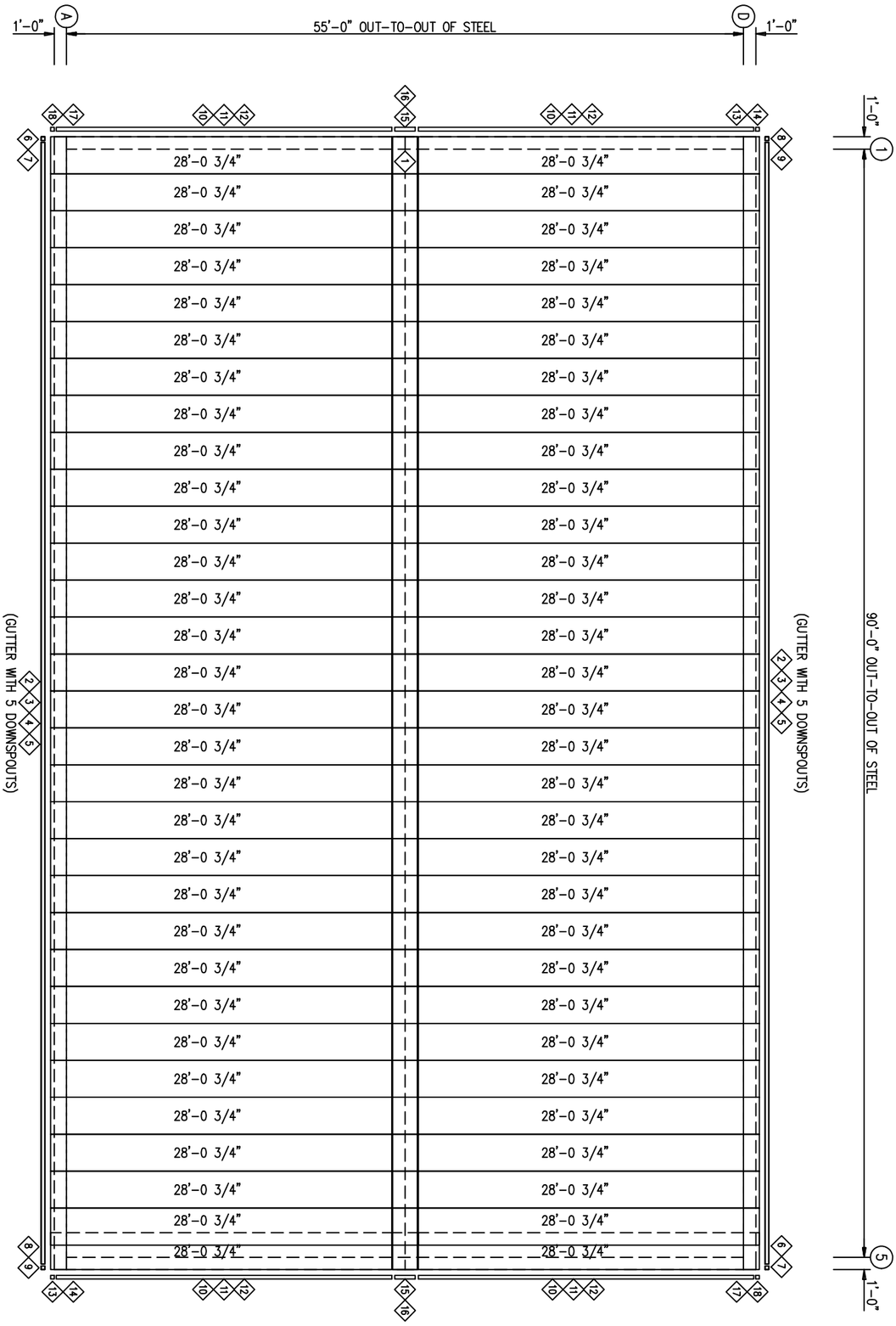
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.
SHEET NUMBER: 11 OF 18
JOB NUMBER 93598
SHEET TITLE: ROOF FRAMING PLAN



ROOF SHEETING PLAN

PANELS: 26 GA. PBR – CHARCOAL GRAY

TRIM TABLE
ROOF PLAN

ID	QUAN	PART	LENGTH	DETAIL
1	31	PS-26R	3'-0"	TD8
2	16	FL-501	11'-9"	TD20
3	12	FL-502	11'-6"	TD20
4	12	FL-32	12'-2"	TD20
5	16	FL-31	11'-8"	TD20
6	2	FL-32L	8"	TD85
7	2	FL-33L	8"	TD85
8	2	FL-32R	11'-2"	TD13
9	2	FL-33R	8"	TD85
10	12	FL-503	9'-9"	TD24
11	2	FL-504	9'-5"	TD24
12	8	FL-21	9'-5"	TD24
13	2	FL-21L	11'-2"	TD13
14	2	FL-328L	9"	TD85
15	2	FL-23	1'-4"	
16	2	FL-88	1'-2"	TD72
17	2	FL-21R	11'-2"	TD13
18	2	FL-328R	9"	TD85

CUSTOMER NAME:
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

SHEET NUMBER: 12 OF 18
JOB NUMBER: 93598
SHEET TITLE: ROOF SHEETING PLAN

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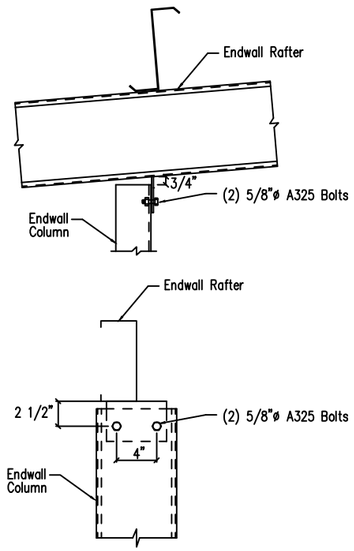
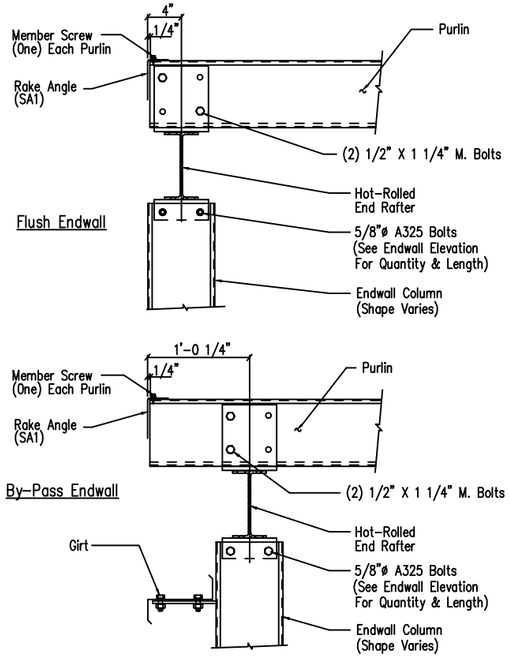
ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

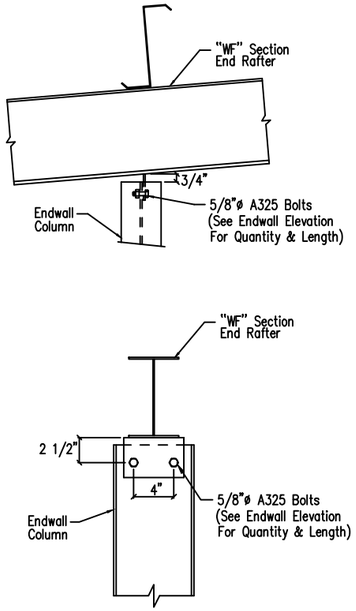


Cold Form Column to Cold Form Rafter — DRAWING NO. SD4

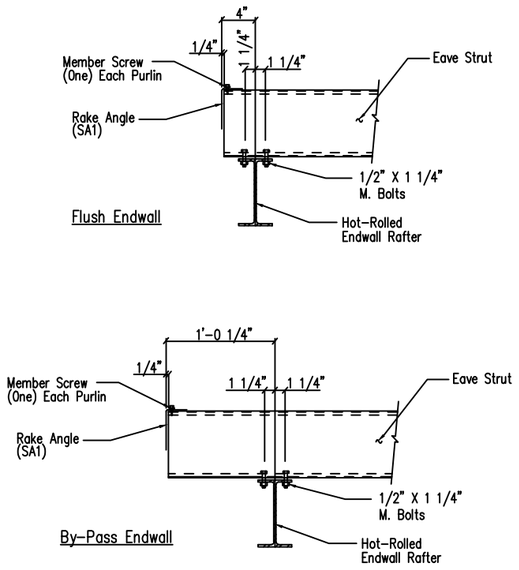
Created On: 2/14/20



Section Thru Rake w/Hot Rolled Rafter — DRAWING NO. SD5



Hot Rolled Column to Hot Rolled Rafter — DRAWING NO. SD7



Eave Strut to Hot Rolled Rafter Connection — DRAWING NO. SD14

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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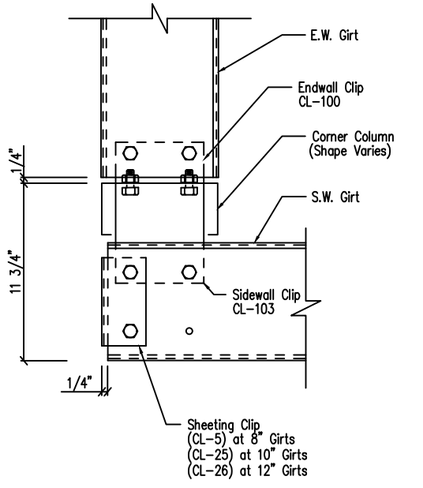
CUSTOMER EMAIL:
SCALE: N.T.S.

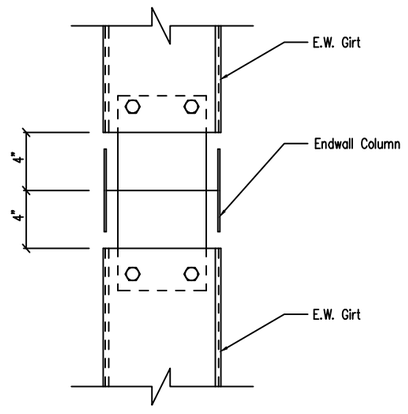
SHEET NUMBER: 13 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS



ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE BELOW THE INTENDED GIRT LINE, THE CLIP(S) IN REFERENCE ARE TOED DOWN. IF ABOVE THE GIRT LINE, THEN TOED UP.

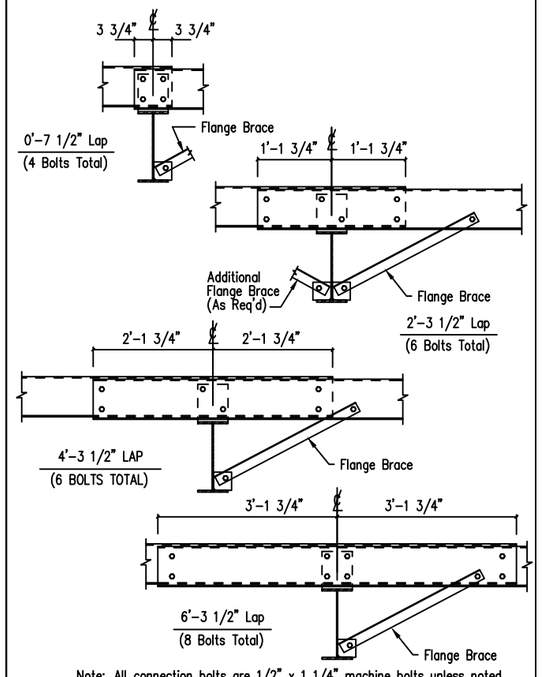
Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Section at "C" Corner Column
Flush Endwall — DRAWING NO. SD24



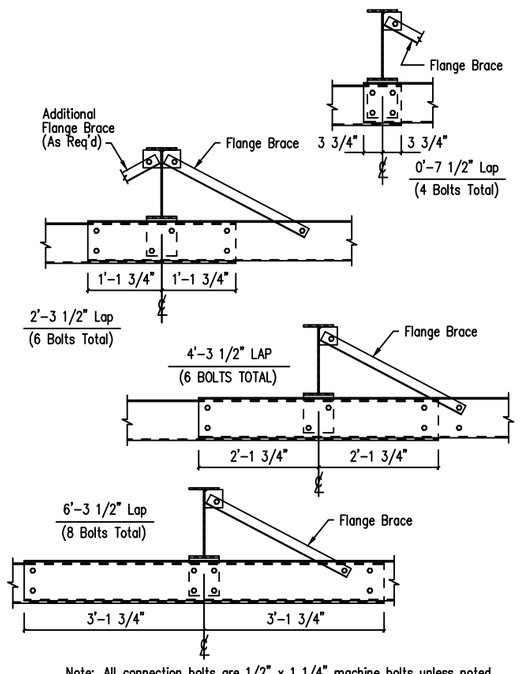
Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Girt to Hot Rolled Endwall Column Connection — DRAWING NO. SD44



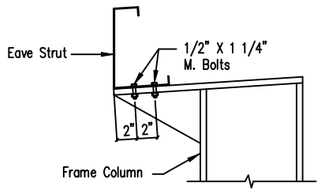
Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

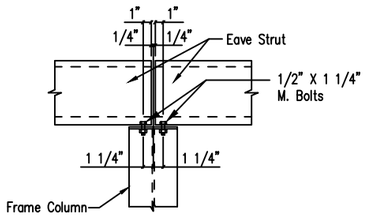
Interior Bay Purlin Framing — DRAWING NO. SD50



Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

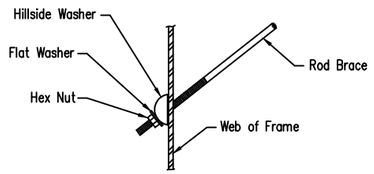
Interior Bay Girt Framing — DRAWING NO. SD51



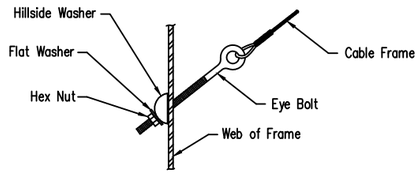


Eave Strut at Interior Column
By-Pass Sidewall

DRAWING NO. SD59



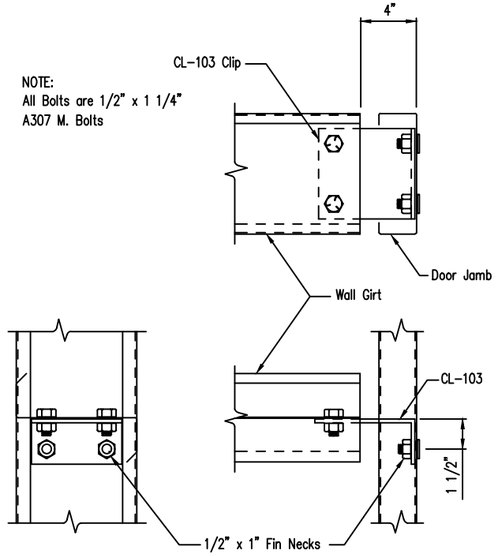
Rod Brace to Frame Detail



Cable Brace to Frame Detail

Cable or Rod Brace to Frame Connection

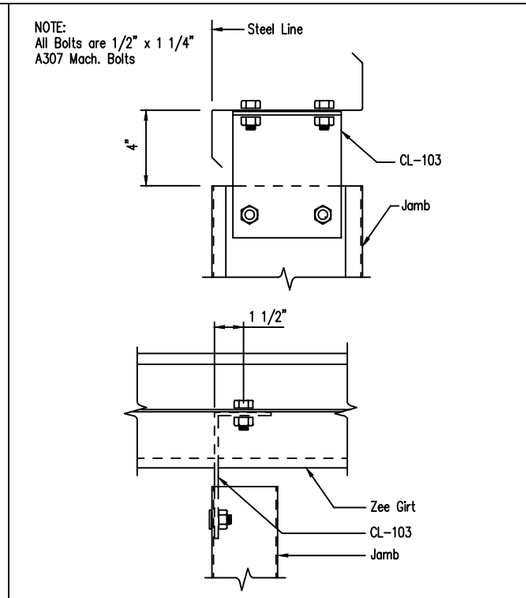
DRAWING NO. SD66



ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE BELOW THE INTENDED GIRT LINE, THE CLIP(S) IN REFERENCE ARE TOED DOWN. IF ABOVE THE GIRT LINE, THEN TOED UP. THE DRAWINGS ABOVE ARE TOED DOWN FOR REFERENCE.

Girt to Jamb (Bolted Clips)

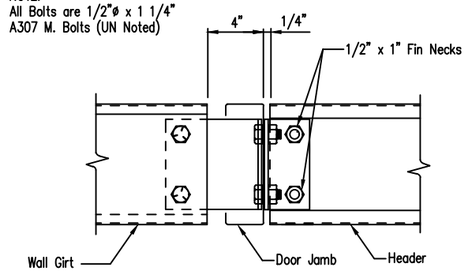
DRAWING NO. SD87

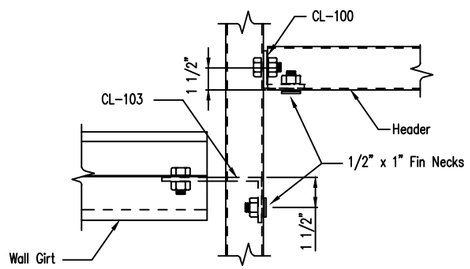


ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE LEFT OF THE INTENDED JAMB LINE, THE CLIP(S) IN REFERENCE ARE TOED LEFT. IF RIGHT OF THE JAMB LINE, THEN TOED RIGHT. THE DRAWINGS ABOVE ARE TOED LEFT FOR REFERENCE.

Jamb to Girt

DRAWING NO. SD93

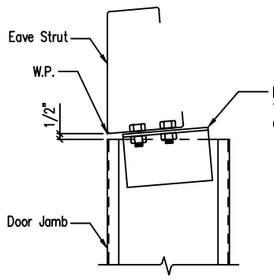




ERECTOR NOTE: IF CLIP BOXES ON BUILDING ELEVATION VIEW(S) ARE BELOW THE INTENDED GIRT LINE, THE CLIP(S) IN REFERENCE ARE TOED DOWN. IF ABOVE THE GIRT LINE, THEN TOED UP. THE DRAWINGS ABOVE ARE TOED DOWN FOR REFERENCE.

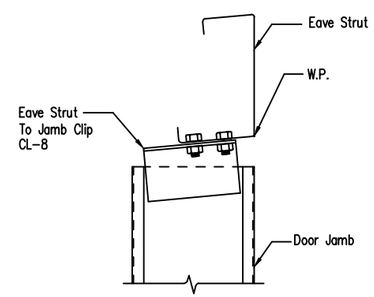
Girt/Header to Jamb

DRAWING NO. SD95



Low Eave Condition

NOTE:
All Bolts are 1/2" x 1 1/4"
A307 Mach. Bolts

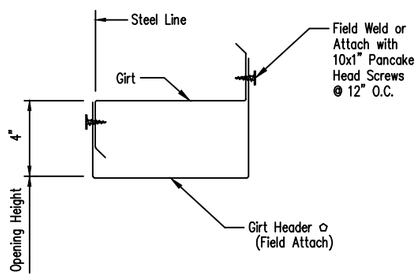


High Eave Condition

Created On: 9/2/11

Jamb To Eave Strut
2:12 Roof Pitch and Higher

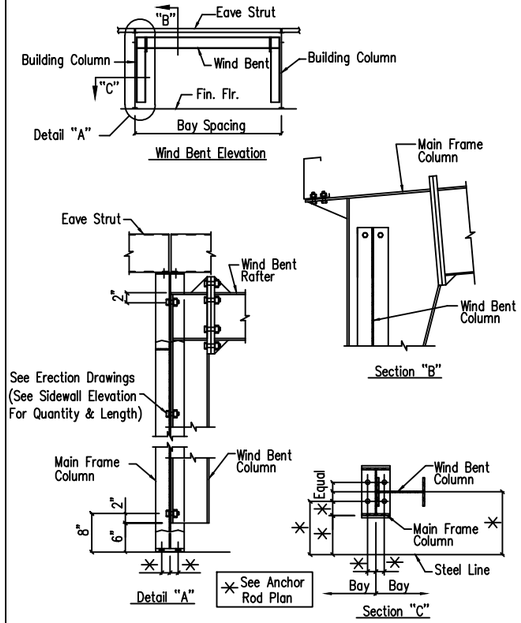
DRAWING NO. SD97



o MEMBER TABLE	
ZEE Size	Piece Mark
8"	GH-1
10"	GH-2
12"	GH-3

Framed Opening w/ Girt Header

DRAWING NO. SD104



Created On: 5/25/12
Revised On: 7/23/14

Flush Wind Bent (Floating)

DRAWING NO. SD150

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	15/16/11	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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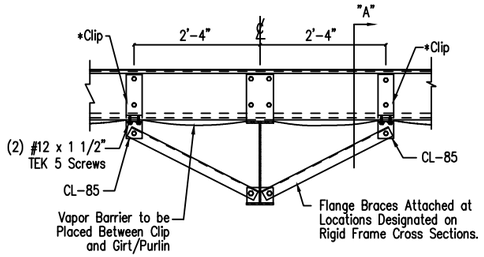
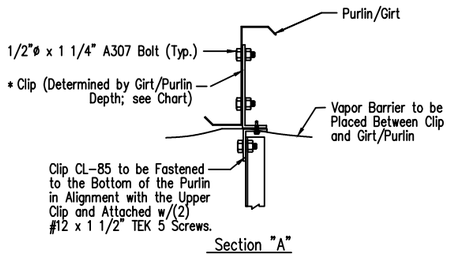
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.
SHEET NUMBER: 14 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS



*Clip Table	
8" Girt/Purlin	CL-86
10" Girt/Purlin	CL-87
12" Girt/Purlin	CL-88



Section "A"

Note: All connection bolts are 1/2" x 1 1/4" machine bolts unless noted.

Interior Girt/Purlin Flange Brace Attachment for Insulation

DRAWING NO. SD161

Revised On : 05/04/18
Created On: 6/28/11

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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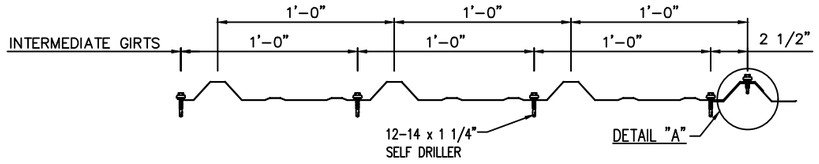
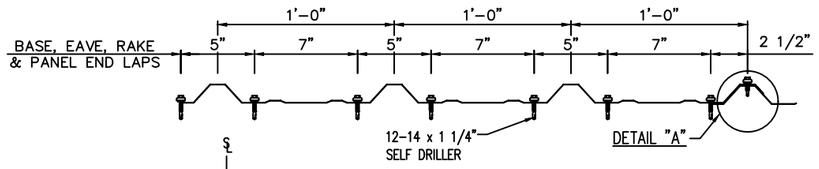
PROJECT NAME:
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.

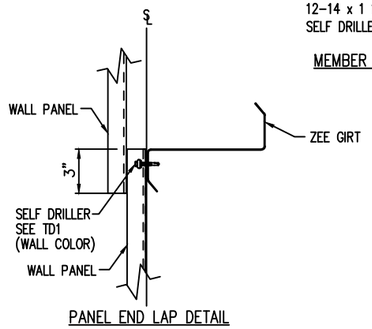
SHEET NUMBER: 15 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS



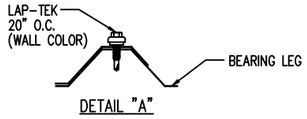
MEMBER SCREW SPACING

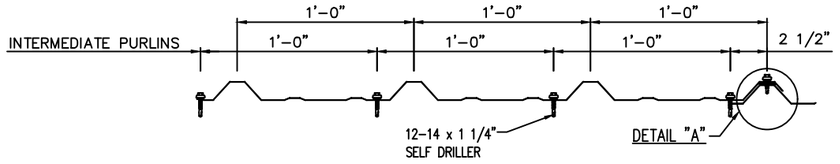


MEMBER SCREW SPACING

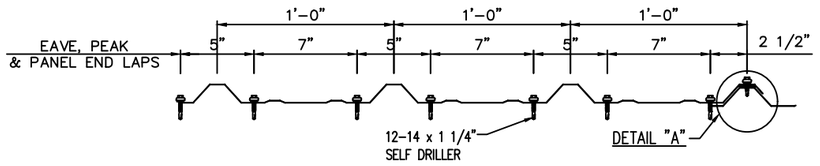


PANEL END LAP DETAIL

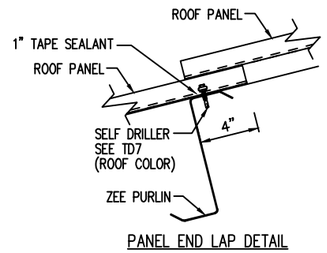




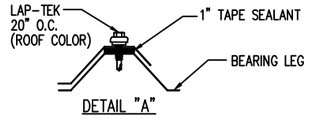
MEMBER SCREW SPACING



MEMBER SCREW SPACING



PANEL END LAP DETAIL



Fastener Location at Wall – PBR

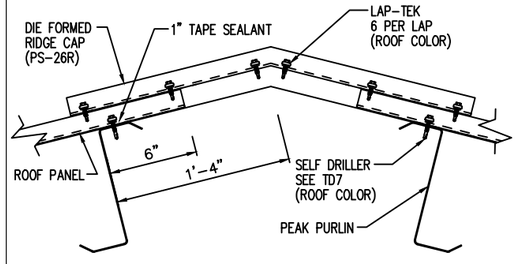
DRAWING NO. TD1

Created On: 08/13/18

Fastener Location at Roof – PBR

DRAWING NO. TD7

Created On: 08/13/18

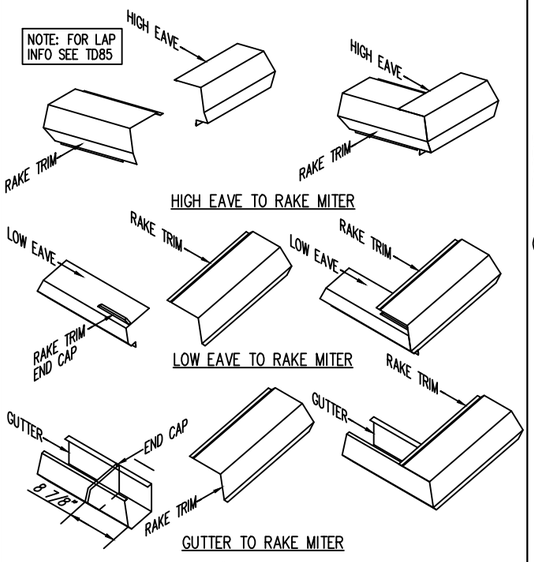


Die Formed Ridge Detail – PBR

Up to a 4:12 Roof Slope

DRAWING NO. TD8

Created On: 08/13/18

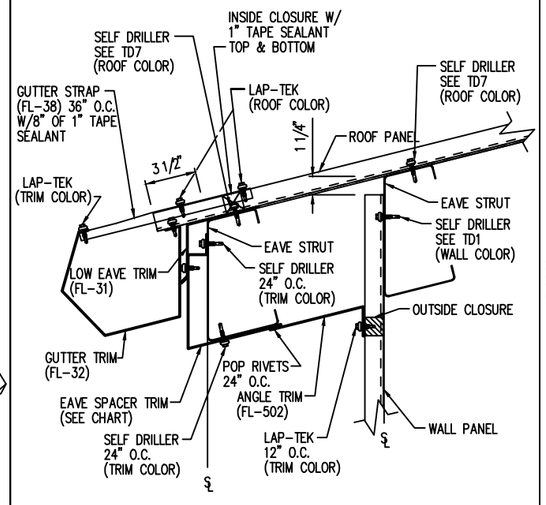


LOCATE THE GUTTER END CAP 8 7/8" AWAY FROM THE FRONT MITER OF THE GUTTER. IF DONE PROPERLY THE END CAP SHOULD LINE UP WITH THE LOW LEG OF THE RAKE TRIM THAT SITS ON TOP OF THE ROOF PANEL. APPLY ONE BEAD OF TUBE SEALANT (NOT BY G.W.B.) BETWEEN THE END CAP SURFACE AND THE GUTTER. ATTACH THE END CAP TO THE GUTTER USING (12) POP RIVETS. CHECK TO MAKE SURE ALL EDGES ARE SEALED WITH TUBE SEALANT (NOT BY G.W.B.). INSERT THE GUTTER INTO THE RAKE TRIM. ALIGN THE MITERED EDGES AND ATTACH TO THE RAKE TRIM WITH (12) POP RIVETS.

Sculptured Trim Detail – PBR

DRAWING NO. TD13

Created On: 08/13/18



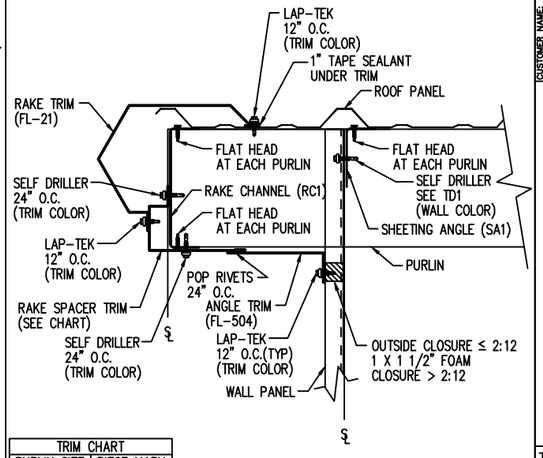
TRIM CHART	
PURLIN SIZE	PIECE MARK
8"	FL-501
10"	FL-501
12"	FL-501

Low Eave Gutter Detail – PBR

Sculptured Gutter – Open Wall – With Soffit

DRAWING NO. TD20

Created On: 08/08/18



TRIM CHART	
PURLIN SIZE	PIECE MARK
8"	FL-503
10"	FL-503
12"	FL-503

NOTE: FIELD NOTCH WALL PANEL AROUND PURLINS AND EAVE STRUTS
NOTE: FIELD SLOPE CUT WALL PANELS AS REQUIRED FOR ROOF SLOPE

Rake Detail – PBR

Sculptured Rake – Soffit Trim

DRAWING NO. TD24

Created On: 08/08/18

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/16/11	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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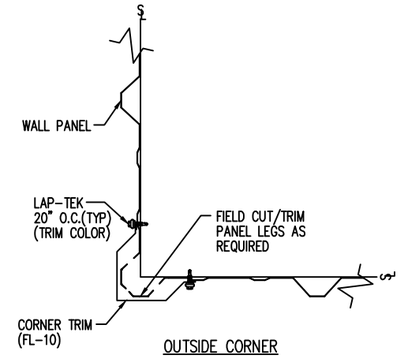
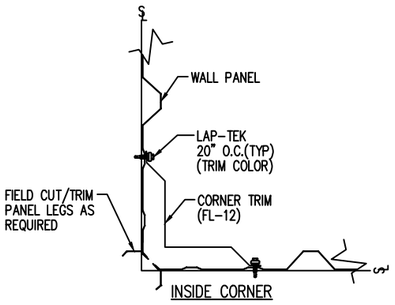
PROJECT LOCATION:
PROJECT COUNTY:
PROJECT END USE:
CUSTOMER PHONE NUMBER:
CUSTOMER EMAIL:
SCALE: N.T.S.
SHEET NUMBER: 16 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS



OUTSIDE CORNER

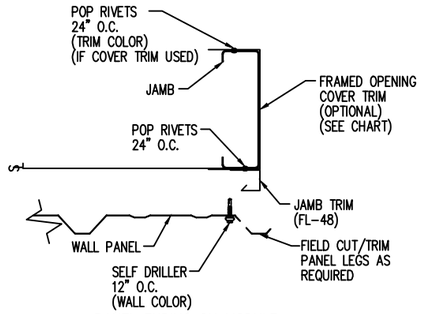


INSIDE CORNER

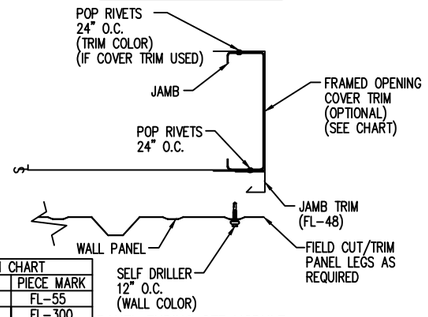
Section at Corner Detail – PBR

DRAWING NO. TD40

Created On: 08/26/18



PANEL ENDING ON MODULE



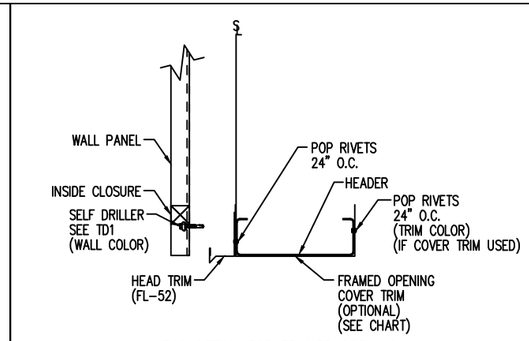
PANEL ENDING OFF MODULE

TRIM CHART	
GIRT SIZE	PIECE MARK
8"	FL-55
10"	FL-300
12"	FL-301

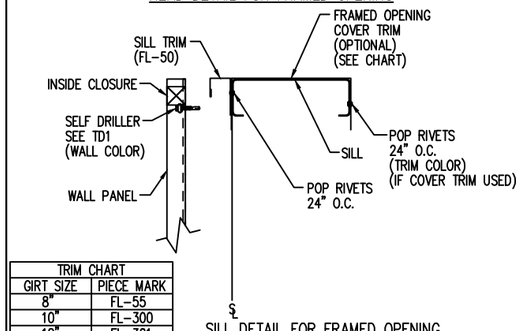
Framed Opening Jamb Detail – PBR

DRAWING NO. TD51

Created On: 08/26/18



HEAD DETAIL FOR FRAMED OPENING



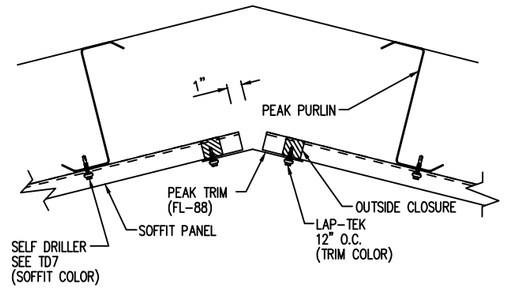
SILL DETAIL FOR FRAMED OPENING

TRIM CHART	
GIRT SIZE	PIECE MARK
8"	FL-55
10"	FL-300
12"	FL-301

Framed Opening Head and Sill Details – PBR

DRAWING NO. TD52

Created On: 08/26/18

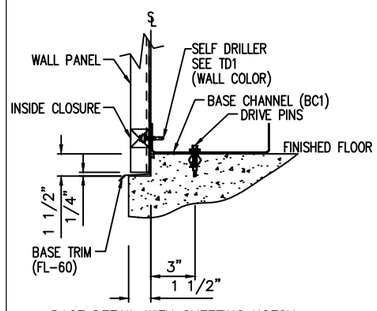


Flat Ridge Detail – PBR

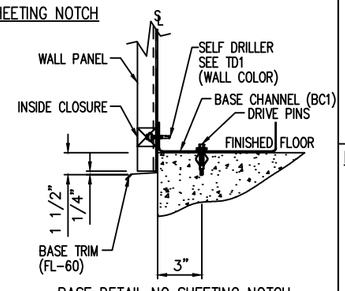
With Soffit

DRAWING NO. TD72

Created On: 08/17/18



BASE DETAIL WITH SHEETING NOTCH



BASE DETAIL NO SHEETING NOTCH

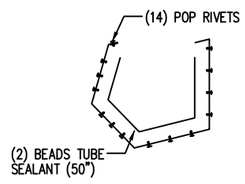
NOTE: IF INSULATION IS REQUIRED INSTALL TRIM FIRST.
NOTE: A MINIMUM OF 1/4" SPACE SHOULD BE ALLOWED FROM THE SHEET END TO ANY SURFACE
NOTE: USE POWER DRIVEN FASTENERS SUCH AS RAMSET (OR EQUAL). 0.140ø STRAIGHT SHANK W 1 1/4" PENETRATION AND MIN. ALLOWABLE SHEAR OF 220# (2'-0" O.C.)(NOT BY GWB)

Base Channel w/Trim Details

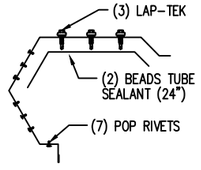
DRAWING NO. TD75

Created On: 08/26/18

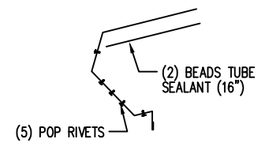
EAVE GUTTER



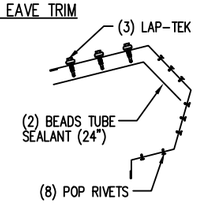
RAKE TRIM



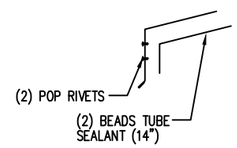
LOW EAVE TRIM



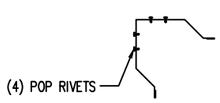
HIGH EAVE TRIM



LOW EAVE TRIM



CORNER TRIM



NOTE: TRIM PROFILES MAY VARY; 2" MIN. LAP UNLESS NOTED
NOTE: TUBE SEALANT (NOT BY G.W.B.)

Trim Laps – PBR Sculptured

DRAWING NO. TD85

Created On: 08/26/18

ISSUE	DATE	DWN.	CHK.	ENG.
APPROVAL	05/26/21	MEZ	MEZ	RTS
PERMIT	06/26/23	AA	CAF	RTS
ERECTION	10/18/23	OGR	OGR	RTS

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PROJECT END USE:
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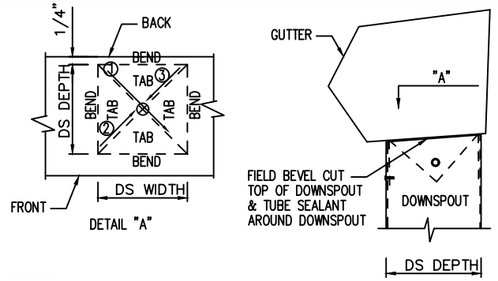
SCALE: N.T.S.

SHEET NUMBER: 17 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS



STEP 1 REFER TO THE BUILDING ERECTIO DRAWINGS FOR THE LOCATION AN SPACING OF THE DOWNSPOUTS.

STEP 2 LOCATE ALL DOWNSPOUTS OVER A MAJOR PANEL RIB IF POSSIBLE.

STEP 3 MAKE A CARBOARD TEMPLATE OF THE DOWNSPOUT SHAPE. PLACE THE TEMPLATE ON THE BOTTOM OF THE GUTTER AND TRACE THE OUTLINE. REMOVE THE TEMPLATE AND DRAW A LINE FROM CORNER TO CORNER, FORMING AN "X" PATTERN.

STEP 4 DRILL A HOLE AT THE CENTER OF THE "X". USING TIN SNIPS, CUT ALONG THE LINES OF THE "X" ONLY. DO NOT CUT ALONG THE OUTSIDE LINES OF THE DOWNSPOUT SQUARE.

STEP 5 BEND EACH TRIANGULAR TAB DOWN TOWARD THE GROUND, 90° TO THE BOTTOM OF THE GUTTER.

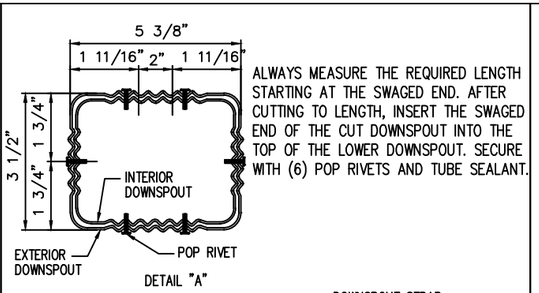
STEP 6 POSITION THE TOP OF THE DOWNSPOUT UNDER THE GUTTER. MAKE SURE ALL FOUR GUTTER TABS ARE ON THE INSIDE OF THE DOWNSPOUT.

STEP 7 INSTALL POP RIVETS THROUGH THE DOWNSPOUT INTO THE GUTTER TAB. ONLY THE TWO SIDES AND THE FRONT OF THE DOWNSPOUT WILL RECEIVE POP RIVETS.

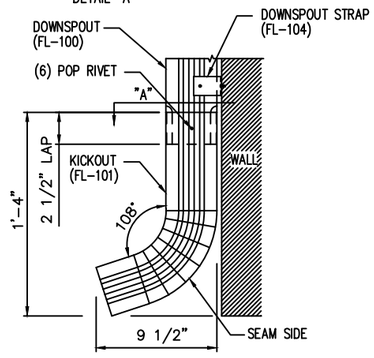
Downspout to Gutter Attachment Detail

DRAWING NO. TD95

Revised On: 8/17/17
Created On: 3/2/12



ALWAYS MEASURE THE REQUIRED LENGTH STARTING AT THE SWAGED END. AFTER CUTTING TO LENGTH, INSERT THE SWAGED END OF THE CUT DOWNSPOUT INTO THE TOP OF THE LOWER DOWNSPOUT. SECURE WITH (6) POP RIVETS AND TUBE SEALANT.



DOWNSPOUT KICKOUT DETAIL

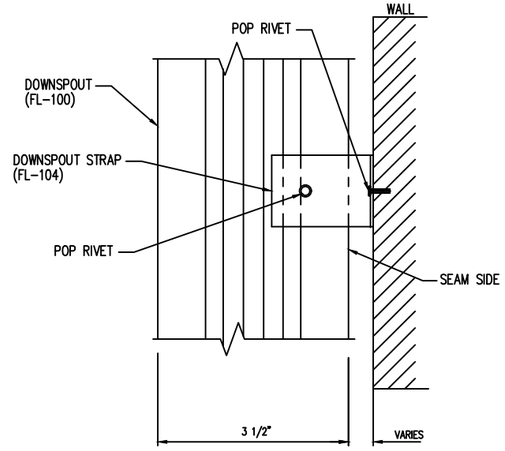
NOTE: TUBE SEALANT (NOT BY G.A.B.)

Downspout Kickout and Splice Detail

3 1/2" x 5 3/8" Roll-Form

DRAWING NO. TD96

Created On: 3/2/12



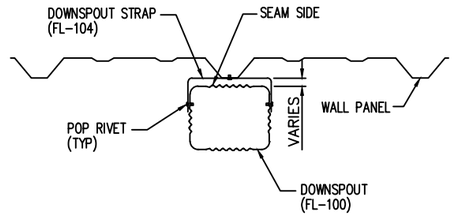
Downspout Strap Attachment Detail

3 1/2" x 5 3/8" Roll-Form

DRAWING NO. TD97

Created On: 3/2/12 Revised on: 08/14/18

D.S. Strap Quantity	
Eave Height	Quantity
10'-0"	2
12'-0"	3
14'-0"	3
16'-0"	4
20'-0"	4
25'-0"	5

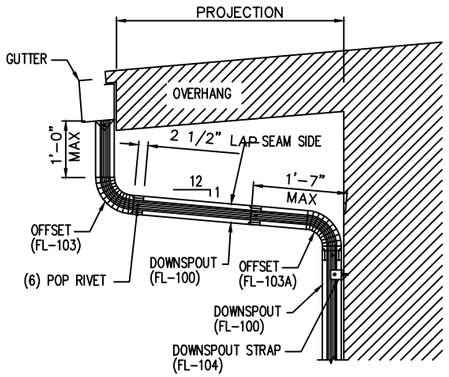


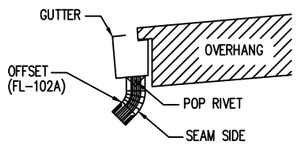
Downspout Strap Attachment Detail – PBR

3 1/2" x 5 3/8" Roll-Form

DRAWING NO. TD98

Created On: 3/2/12 Revised on: 08/14/18



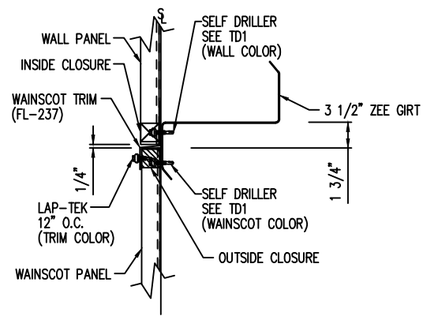


NOTE: REFERENCE DETAIL TD96 FOR SPLICE DETAIL

Eave Canopy Downspout Detail

DRAWING NO. TD99

Created On: 3/2/12



Wainscot with Panel Break – PBR

DRAWING NO. TD199

Created On: 03/02/20

ISSUE	ENG.
APPROVAL	CHK. RTS
PERMIT	DWN. MEZ CAF RTS
ERECTION	MEZ AA OGR OGR
DATE	15/16/21 06/26/23 10/18/23

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SCALE: N.T.S.

SHEET NUMBER: 18 OF 18
JOB NUMBER 93598
SHEET TITLE: DETAIL DRAWINGS